tjs architects

BIDDING ADDENDUM

To: All Bidders
From: Tyler Smith

ADDENDUM#: 01
Date: 8.27.26
Project: Bank of Utah Spanish Fork
Project #: 26002

Summary:

Clarifications and bidder question responses.

Description:

General

- See attached Geotech report
- Omit bid bond
- Please see drawings for items that are FOIC (furnished by Owner, Installed by Contactor)
- Building is not sprinklered

A501

- cleaned up overlapping details

A601

- Clarification to aluminum frame

M001

- revised sheet index

Supporting Documents:

Geotech report, A501, A601, M001

REPORT
GEOTECHNICAL STUDY
PROPOSED BANK OF UTAH – SPANISH FORK
1351 NORTH CANYON CREEK PARKWAY
SPANISH FORK, UTAH (40.1276°, -111.6396°)

Submitted To:

Bank of Utah
2605 Washington Boulevard
Ogden, Utah 84401

Submitted By:

GSH Geotechnical, Inc.
473 West 4800 South
Salt Lake City, Utah 84123

June 30, 2026

Job No. 4314-001-26

June 30, 2026
Job No. 4314-001-26

Ms. Menah Strong
Bank of Utah
2605 Washington Boulevard
Ogden, Utah 84401

Mr. Strong:

Re: Report
Geotechnical Study
Bank of Utah – Spanish Fork
1351 North Canyon Creek Parkway
Spanish Fork, Utah (40.1276°, -111.6396°)

1. INTRODUCTION

1.1 GENERAL

This report presents the results of our geotechnical study performed at the site of the Bank of Utah – Spanish Fork to be located at 1351 North Canyon Creek Parkway in Spanish Fork, Utah (40.1276°, -111.6396°). The general location of the site with respect to existing roadways, as of 2026, is presented on Figure 1, Vicinity Map. A more detailed layout of the site showing proposed facilities, existing roadways, and the borings drilled in conjunction with this study is presented on Figure 2, Site Plan.

1.2 OBJECTIVES AND SCOPE

The objectives and scope of the study were planned in discussions between Ms. Menah Strong of Bank of Utah and Mr. Robert Gifford of GSH Geotechnical, Inc. (GSH).

In general, the objectives of this study were to:

1. Define and evaluate the subsurface soil and groundwater conditions across the site.

2. Provide appropriate foundation, earthwork, pavement, and geoseismic recommendations to be utilized in the design and construction of the proposed facilities.

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In accomplishing these objectives, our scope has included the following:

1. A field program consisting of the exploration, logging, and sampling of 5 borings, and a stormwater percolation test.

2. A laboratory testing program.

3. An office program consisting of the correlation of available data, engineering analysis, and the preparation of this summary report.

1.3 AUTHORIZATION

A Professional Services Agreement (PSA) No. 26-0502.rev1 was provided to Ms. Menah C. Strong of Bank of Utah dated June 1, 2026. Authorization was provided by returning a signed copy of the PSA on the same date.

1.4 PROFESSIONAL STATEMENTS

Supporting data upon which our recommendations are based are presented in subsequent sections of this report. Recommendations presented herein are governed by the physical properties of the soils encountered in the exploration borings, projected groundwater conditions, and the layout and design data discussed in Section 2, Proposed Construction. If subsurface conditions other than those described in this report are encountered and/or if design and layout changes are implemented, GSH must be informed so that our recommendations can be reviewed and amended, if necessary.

Our professional services have been performed, our findings developed, and our recommendations prepared in accordance with generally accepted engineering principles and practices in this area at this time.

2. PROPOSED CONSTRUCTION

The project is to consist of the construction of a credit union structure with a footprint of approximately 3,000 to 5,000 square feet and associated pavements. The structure is anticipated to be 1- to 2-stories, constructed slab-on-grade, and supported upon conventional spread and continuous wall footings.

Maximum real column and wall loads are anticipated to be up to 80 kips and 3 to 5 kips per lineal foot, respectively. Real loads are defined as the total of all dead plus frequently applied (reduced) live loads.

Paved parking areas and drive lanes are planned around the structure. Projected traffic in these areas is anticipated to consist of a light volume of automobiles and light trucks, occasional medium-weight trucks, occasional traffic of an AASHTO HS20-44 Firetruck, and no heavyweight trucks.

Site development will require some earthwork in the form of minor cutting and filling. At this time, we anticipate that maximum site grading cuts and fills, excluding utilities, will be on the order of 1 to 3 feet.

3. SITE INVESTIGATIONS

3.1 GENERAL

Subsurface conditions in unexplored locations or at other times may vary from those encountered at specific boring locations. If such variations are noted during construction or if project development plans are changed, GSH must review the changes and amend our recommendations, if necessary.

Boring locations were established by estimating distances and angles from site landmarks. If increased accuracy is desired by the client, we recommend that the boring locations and elevations be surveyed.

3.2 FIELD PROGRAM

To define and evaluate the subsurface soil and groundwater conditions across the site, 5 borings were completed within the accessible areas. These borings were completed to depths ranging from 5.5 to 46.5 feet with a truck-mounted drill rig equipped with hollow-stem augers. The approximate locations of the borings are presented on Figure 2. Additionally, a stormwater percolation test to determine the percolation rate was performed in Boring B-5 at a depth of 5.0 feet.

The field portion of our study was under the direct control and continual supervision of an experienced member of our geotechnical staff. During the course of the drilling operations, a continuous log of the subsurface conditions encountered was maintained. In addition, samples of the typical soils encountered were obtained for subsequent laboratory testing and examination. The soils were classified in the field based upon visual and textural examination. These classifications were supplemented by subsequent inspection and testing in our laboratory. Graphical representation of the subsurface conditions encountered is presented on Figures 3A through 3E, Boring Logs. Soils were classified in accordance with the nomenclature described on Figure 4, Key to Boring Log (USCS).

A 3.25-inch outside diameter, 2.42-inch inside diameter (Dames & Moore) and a 2.0-inch outside diameter, 1.38-inch inside diameter drive sampler (SPT) were utilized at select locations and depths. The blow counts recorded on the boring logs were those required to drive the sampler 12 inches with a 140-pound hammer dropping 30 inches.

Following completion of exploration operations, 1.25-inch diameter slotted PVC pipe was installed in Borings B-1 and B-2 to provide a means of monitoring the groundwater fluctuations. The borings were backfilled with auger cuttings.

3.3 LABORATORY TESTING

3.3.1 General

To provide data necessary for our engineering analysis, a laboratory testing program was performed. This program included moisture, density, partial gradation, Atterberg limits, consolidation, and chemical tests. The following paragraphs describe the tests and summarize the test data.

3.3.2 Moisture and Density Tests

To provide index parameters and to correlate other test data, moisture and density tests were performed on selected samples. The results of these tests are presented on the boring logs, Figures 3A through 3E.

3.3.3 Partial Gradation Tests

To aid in classifying the granular soils, partial gradation tests were performed. Results of the tests are tabulated below and presented on the boring logs, Figures 3A through 3E:

Boring No.	Depth (feet)	Percent Passing No. 200 Sieve	Moisture Content Percent	Soil Classification
B-5	5.0	72.6	20.0	CL

3.3.4 Atterberg Limits Test

To aid in classifying the soils, an Atterberg limits test was performed on a sample of the fine-grained cohesive soils. Results of the test are tabulated below and presented on the boring logs, Figures 3A through 3E:

Boring No.	Depth (feet)	Liquid Limit (percent)	Plastic Limit (percent)	Plasticity Index (percent)	Soil Classification
B-1	5.0	36	19	17	CL
B-1	20.0	41	23	18	CL
B-1	35.0	48	27	21	CL

3.3.5 Consolidation Tests

To provide data necessary for our settlement analysis, consolidation testing was performed on 2 representative samples of the natural fine-grained clay soils encountered at the site. The results of these tests indicate that the samples tested were moderately over-consolidated and will exhibit

moderate strength and compressibility characteristics under the anticipated loading. Detailed results of the tests are maintained within our files and can be transmitted to you, upon your request.

3.3.6 Chemical Tests

To determine if the site soils will react detrimentally with concrete, chemical tests were performed on a representative sample of the near-surface soil encountered at the site. The results of the chemical tests are tabulated below:

Boring No.	Depth (feet)	Soil Classification	pH	Total Water-Soluble Sulfate (mg/kg-dry)
B-2	2.5	CL	9.4	68

4. SITE CONDITIONS

4.1 SURFACE

The site is located at 1351 North Canyon Creek Parkway in Spanish Fork, Utah (40.1276°, -111.6396°). The site is currently vacant/undeveloped land. Review of historic aerial imagery indicates that the site was previously utilized as a staging ground for nearby construction activities and that some non-engineered fills have been placed at the site. The topography of the site is relatively flat, grading down to the north with a total relief of less than 3 feet. Site vegetation consists of various weeds and brush/grass.

The site is bounded to the north by existing paved parking areas along with Market Place Drive followed by vacant/undeveloped agricultural land; to the east by an unnamed paved road and Market Place Drive followed by vacant/undeveloped agricultural land; to the south by an unnamed paved road followed by commercial structures; and to the west by paved parking areas followed by commercial structures and vacant/undeveloped agricultural land.

4.2 SUBSURFACE SOIL

The following paragraphs provide generalized descriptions of the subsurface profiles and soil conditions encountered within the borings conducted during this study. As previously noted, soil conditions may vary in unexplored locations.

The borings were completed to depths ranging from 5.5 to 46.5 feet. The soil conditions encountered in each of the borings, to the depths completed, were generally similar across the boring locations.

- Although not encountered in any of the borings, asphalt and/or concrete was observed on the ground surface of the existing parking lot located to the north and west of the site.

- Non-engineered fill soils were encountered in each boring, to depths ranging from 2.0 to 2.5 feet beneath the existing ground surface. The non-engineered fill soils primarily consisted of sand with varying silt and gravel content.

- Natural soils were encountered below the non-engineered fill in each boring. The natural soils consisted primarily of clay with varying silt, sand, and gravel content.

The natural clay soils were soft to stiff, moist to saturated, light, gray, light brown, and brown in color, and moderately over-consolidated. The natural clay soils are anticipated to exhibit moderate strength and compressibility characteristics under the anticipated loading.

For a more descriptive interpretation of subsurface conditions, please refer to Figures 3A through 3E, Boring Logs. The lines designating the interface between soil types on the boring logs generally represent approximate boundaries. In situ, the transition between soil types may be gradual.

4.3 GROUNDWATER

On June 29, 2026 (17 days following drilling), groundwater was measured within the PVC pipes installed as tabulated below:

Boring No.	Groundwater Depth (feet)
	June 29, 2026
B-1	2.8
B-2	4.0

Groundwater levels vary with changes in season and rainfall, construction activity, irrigation, snow melt, surface water run-off, and other site-specific factors.

5. DISCUSSIONS AND RECOMMENDATIONS

5.1 SUMMARY OF FINDINGS

The proposed structures may be supported upon conventional spread and continuous wall foundations supported upon suitable natural soils and/or structural fill extending to suitable natural soils.

The most significant geotechnical aspects at the site are:

1. The existing non-engineered fills across much of the site.

2. The potential to encounter additional non-engineered fill at the site.

3. The relatively shallow depth to groundwater.

Prior to proceeding with construction, removal of any existing debris, surface vegetation, root systems, topsoil, non-engineered fill, and any deleterious materials from beneath an area extending out at least 5 feet from the perimeter of the proposed structure footprints and 3 feet beyond pavements and exterior flatwork areas will be required. All existing utility locations should be reviewed to assess their impact on the proposed construction and abandoned and/or relocated as appropriate.

Due to the developed nature of this site and the surrounding area, additional non-engineered fills may exist in unexplored areas of the site. Based on our experience, non-engineered fills are frequently erratic in composition and consistency. All surficial loose/disturbed soils and non-engineered fills must be removed below all footings, floor slabs, and rigid pavements. The in situ, non-engineered fills may remain below flexible pavements if free of any deleterious materials, of limited thickness, and if properly prepared, as discussed later in this report. Even with proper preparation, pavements established overlying non-engineered fills may encounter some long-term movements unless the non-engineered fills are completely removed.

On-site non-engineered fill soils encountered were primarily granular. On-site granular soils, including existing non-engineered fills, may be re-utilized as structural site grading fill if they meet the criteria for such, as stated later in this report.

Groundwater was measured as shallow as 2.8 feet below the ground surface. GSH recommends placing floor slabs no closer than 4 feet from the highest groundwater elevation. Site grading fill may be utilized to raise the overall grade to achieve the required separation between the floor slab and the highest groundwater elevation.

Proof rolling of the natural clay subgrade must not be completed if cuts extend to within 1 foot of the groundwater surface. In areas where cuts are to extend to within 1 foot of the groundwater surface, extensive stabilization must be anticipated.

To reduce disturbance of the natural soils during excavation, it is recommended that low-impact, track-mounted equipment with smooth edge buckets/blades be utilized.

Detailed discussions pertaining to earthwork, foundations, pavements, and the geoseismic setting of the site are presented in the following sections.

5.2 EARTHWORK

5.2.1 Site Preparation

Initial site preparation will consist of the removal of any existing debris, non-engineered fills, surface vegetation, root systems, topsoil, and any deleterious materials from beneath an area extending out at least 5 feet from the perimeter of the proposed structure footprint and 3 feet beyond pavements and exterior flatwork areas. All existing utility locations should be reviewed to assess their impact on the proposed construction and abandoned and/or relocated as appropriate.

In situ, non-engineered fills may remain below flexible pavements if free of debris and deleterious materials, less than 3 feet in thickness, and if properly prepared. Proper preparation below pavements will consist of the scarification of the upper 12 inches below asphalt concrete (flexible pavement), followed by moisture preparation and re-compaction to the requirements of structural fill. Even with proper preparation, pavements established overlying non-engineered fills may encounter some long-term movements unless the non-engineered fills are completely removed.

It must be noted that from a handling and compaction standpoint, soils containing high amounts of fines (silts and clays) are inherently more difficult to rework and are very sensitive to changes in moisture content, requiring very close moisture control during placement and compaction. This will be very difficult, if not impossible, during wet and cold periods of the year. Additionally, the on-site soils are likely above optimum moisture content for compacting at present and would require some drying prior to re-compacting.

Subsequent to stripping and prior to the placement of floor slabs, foundations, structural site grading fills, exterior flatwork, and pavements, the exposed subgrade must be proof rolled by passing moderate-weight rubber tire-mounted construction equipment over the surface at least twice. If excessively soft or otherwise unsuitable soils are encountered, they must be completely removed. If removal depth required is greater than 2 feet below footings, GSH must be notified to provide further recommendations.

Subgrade preparation as described must be completed prior to placing overlying structural site grading fills.

Due to the relatively high groundwater, site grading cuts should be kept to a minimum. Cuts extending to within 1 foot of the groundwater elevation will likely disturb the natural clay soils and proof rolling must not be completed. Extensive stabilization must be anticipated in areas where cuts are to extend to within 1 foot of the groundwater surface.

To reduce disturbance of the natural soils during excavation, it is recommended that low-impact, track-mounted equipment with smooth edge buckets/blades be utilized.

GSH must be notified prior to the placement of structural site grading fills, floor slabs, footings, and pavements to verify that all loose/disturbed soils and non-engineered fills have been

completely removed and/or properly prepared. For more information refer to Section 5.11 of this report.

5.2.2 Temporary Excavations

Temporary excavations up to 8 feet deep in fine-grained cohesive soils, above or below the water table, may be constructed with sideslopes no steeper than one-half horizontal to one vertical (0.5H:1.0V). Excavations deeper than 8 feet are not anticipated at the site.

For granular (cohesionless) soils, construction excavations above the water table, not exceeding 4 feet, shall be no steeper than one-half horizontal to one vertical (0.5H:1.0V). For excavations up to 8 feet, in granular soils and above the water table, the slopes shall be no steeper than one horizontal to one vertical (1H:1V). Excavations encountering saturated cohesionless soils will be very difficult and will require very flat sideslopes and/or shoring, bracing, and dewatering.

To reduce disturbance of the natural soils during excavation, it is recommended that low-impact, track-mounted equipment with smooth edge buckets/blades be utilized.

The static groundwater table was encountered as shallow as 2.8 feet below the existing surface and may be shallower with seasonal fluctuations. Consideration for dewatering of utility trenches, excavations for the removal of non-engineered fill, and other excavations below this level should be incorporated into the design and bidding process.

All excavations must be inspected periodically by qualified personnel. If any signs of instability or excessive sloughing are noted, immediate remedial action must be initiated.

5.2.3 Structural Fill

Structural fill is defined as all fill which will ultimately be subjected to structural loadings, such as imposed by footings, floor slabs, pavements, etc. Structural fill will be required as backfill over foundations and utilities, as site grading fill, and as replacement fill below footings. All structural fill must be free of surface vegetation, root systems, rubbish, topsoil, frozen soil, and other deleterious materials.

Structural site grading fill is defined as structural fill placed over relatively large open areas to raise the overall grade. For structural site grading fill, the maximum particle size shall not exceed 4 inches; although, occasional larger particles, not exceeding 8 inches in diameter, may be incorporated if placed randomly in a manner such that “honeycombing” does not occur, and the desired degree of compaction can be achieved. The maximum particle size within structural fill placed within confined areas shall be restricted to 2 inches.

On-site soils, including existing non-engineered fills, may be re-utilized as structural site grading fill if they do not contain construction debris or deleterious material and meet the requirements of

structural fill. Fine-grained soils will require very close moisture control and may be very difficult, if not impossible, to properly place and compact during wet and cold periods of the year.

Imported structural fill below foundations and floor slabs shall consist of a well graded sand and gravel mixture with less than 30 percent retained on the three-quarter-inch sieve and less than 20 percent passing the No. 200 Sieve (clays and silts).

To stabilize soft subgrade conditions or where structural fill is required to be placed closer than 2.0 feet above the water table at the time of construction, a mixture of coarse angular gravels and cobbles and/or 1.5- to 2.0-inch gravel (stabilizing fill) shall be utilized. It may also help to utilize a stabilization geosynthetic, such as Mirafi RS380i/RS580i or a geogrid such as Tensar NX750/NX850, placed on the natural ground if 1.5- to 2.0-inch gravel is used as stabilizing fill.

GSH must be notified prior to the placement of structural site grading fills, floor slabs, footings, and pavements to verify that all loose/disturbed soils and non-engineered fills have been completely removed and/or properly prepared. For more information refer to Section 5.11 of this report.

5.2.4 Fill Placement and Compaction

All structural fill shall be placed in lifts not exceeding 8 inches in loose thickness. Structural fills shall be compacted in accordance with the percent of the maximum dry density as determined by the AASHTO[1] T180 (ASTM[2] D1557) compaction criteria in accordance with the following table:

Location	Total Fill Thickness (feet)	Minimum Percentage of Maximum Dry Density
Beneath an area extending at least 5 feet beyond the perimeter of the structure	0 to 5	95
Site grading fills outside area defined above	0 to 5	90
Utility trenches within structural areas	--	96
Road base	--	96

If structural fills greater than 5 feet thick are anticipated at the site, GSH **must** be contacted for additional recommendations.

1 American Association of State Highway and Transportation Officials
2 American Society for Testing and Materials

Subsequent to stripping and prior to the placement of structural site grading fill, the subgrade shall be prepared as discussed in Section 5.2.1, Site Preparation, of this report. In confined areas, subgrade preparation shall consist of the removal of all loose or disturbed soils.

To stabilize soft subgrade conditions or where structural fill is required to be placed closer than 2.0 feet above the water table at the time of construction, a mixture of coarse angular gravels and cobbles and/or 1.5- to 2.0-inch gravel (stabilizing fill) shall be utilized. It may also help to utilize a stabilization geosynthetic, such as Mirafi RS380i/RS580i or a geogrid such as Tensar NX750/NX850, placed on the natural ground if 1.5- to 2.0-inch gravel is used as stabilizing fill. If a differing material such as structural fill with fines is to be placed directly on the gravel, to eliminate the potential of fines migration into the gravel fill.

Non-structural fill may be placed in lifts not exceeding 12 inches in loose thickness and compacted by passing construction, spreading, or hauling equipment over the surface at least twice.

GSH must be notified prior to the placement of structural site grading fills, floor slabs, footings, and pavements to verify that all loose/disturbed soils and non-engineered fills have been completely removed and/or properly prepared. For more information refer to Section 5.11 of this report.

5.2.5 Utility Trenches

All utility trench backfill material below structurally loaded facilities (footings, floor slabs, flatwork, pavements, etc.) shall be placed at the same density requirements established for structural fill. If the surface of the backfill becomes disturbed during the course of construction, the backfill shall be proof rolled and/or properly compacted prior to the construction of any exterior flatwork over a backfilled trench. Proof rolling shall be performed by passing moderately loaded rubber tire-mounted construction equipment uniformly over the surface at least twice. If excessively loose or soft areas are encountered during proof rolling, they shall be removed to a maximum depth of 2 feet below design finish grade and replaced with structural fill.

Many utility companies and City-County governments are now requiring that Type A-1a or A-1b (AASHTO Designation – granular soils with limited fines) soils be used as backfill over utilities. These organizations are also requiring that in public roadways, the backfill over major utilities be compacted over the full depth of fill to at least 96 percent of the maximum dry density as determined by the AASHTO T180 (ASTM D1557) method of compaction. GSH recommends that as the major utilities continue onto the site that these compaction specifications are followed.

Fine-grained soils, such as silts and clays, are not recommended for utility trench backfill in structural areas.

The static groundwater table was encountered as shallow as 2.8 feet below the existing surface and may be shallower with seasonal fluctuations. Dewatering of utility trenches and other excavations below this level should be anticipated.

To reduce disturbance of the natural soils during excavation, it is recommended that low-impact, track-mounted equipment with smooth edge buckets/blades be utilized.

5.3 GROUNDWATER

On June 29, 2026 (17 days following drilling), groundwater was measured within the PVC pipes installed as tabulated below:

Boring No.	Groundwater Depth (feet)
	June 29, 2026
B-1	2.8
B-2	4.0

Groundwater was measured as shallow as 2.8 feet below the ground surface. GSH recommends placing floor slabs no closer than 4 feet from the highest groundwater elevation. Site grading fill may be utilized to raise the overall grade to achieve the required separation between the floor slab and the highest groundwater elevation.

The groundwater measurements presented are conditions at the time of the field exploration and may not be representative of other times or locations. Groundwater levels may vary seasonally and with precipitation, as well as other factors including irrigation. Evaluation of these factors is beyond the scope of this study. Groundwater levels may, therefore, be at shallower or deeper depths than those measured during this study, including during construction and over the life of the structure.

The extent and nature of any dewatering required during construction will be dependent on the actual groundwater conditions prevalent at the time of construction and the effectiveness of construction drainage to prevent run-off into open excavations.

5.3.1 Stormwater Percolation Test

A stormwater percolation test was performed at a depth of approximately 5 feet in the representative natural clay soils at Boring B-5. The field percolation rate was 45 minutes per inch. This design percolation rate should be considered typical for the soils at the site.

5.4 SPREAD AND CONTINUOUS WALL FOUNDATIONS

5.4.1 Design Data

The results of our analysis indicate that the proposed structures may be supported upon conventional spread and continuous wall foundations established upon suitable natural soils and/or

structural fill extending to suitable natural soils. Under no circumstances shall foundations be established over non-engineered fills, loose or disturbed soils, topsoil, surface vegetation, root systems, rubbish, construction debris, other deleterious materials, frozen soils, or within ponded water. For design, the following parameters are provided:

Minimum Recommended Depth of Embedment for Frost Protection	- 30 inches
Minimum Recommended Depth of Embedment for Non-frost Conditions	- 15 inches
Recommended Minimum Width for Continuous Wall Footings	- 18 inches
Minimum Recommended Width for Isolated Spread Footings	- 24 inches
Recommended Net Bearing Capacity for Real Load Conditions	- 1,500 pounds per square foot
Bearing Capacity Increase for Seismic Loading	- 50 percent

The term “net bearing capacity” refers to the allowable pressure imposed by the portion of the structure located above lowest adjacent final grade. Therefore, the weight of the footing and backfill to lowest adjacent final grade need not be considered. Real loads are defined as the total of all dead plus frequently applied live loads. Total load includes all dead and live loads, including seismic and wind.

5.4.2 Installation

Under no circumstances shall the footings be installed upon non-engineered fills, loose or disturbed soils, topsoil, surface vegetation, root systems, rubbish, construction debris, or other deleterious materials. If unsuitable soils are encountered, they must be removed and replaced with compacted granular fill. If granular soils become loose or disturbed, they must be recompacted prior to pouring the concrete.

The width of structural replacement fill below footings shall be equal to the width of the footing plus one foot for each foot of fill thickness.

GSH must be notified prior to the placement of structural site grading fills, floor slabs, footings, and pavements to verify that all loose/disturbed soils and non-engineered fills have been

completely removed and/or properly prepared. For more information refer to Section 5.11 of this report.

5.4.3 Settlements

Based on column loadings, soil bearing capacities, and the foundation recommendations as discussed above, we expect primary total settlement beneath individual foundations to be less than one inch.

The amount of differential settlement is difficult to predict because the subsurface and foundation loading conditions can vary considerably across the site. However, we anticipate differential settlement between adjacent foundations could vary from 0.5 to 0.75 inch. The final deflected shape of the structure will be dependent on actual foundation locations and loading.

5.5 LATERAL RESISTANCE

Lateral loads imposed upon foundations due to wind or seismic forces may be resisted by the development of passive earth pressures and friction between the base of the footings and the supporting soils. In determining frictional resistance, a coefficient of friction of 0.35 may be utilized for the footing interface with in situ natural clay soils and 0.40 for footing interface with natural granular soils or granular structural fill. Passive resistance provided by properly placed and compacted granular structural fill above the water table may be considered equivalent to a fluid with a density of 300 pounds per cubic foot. Below the water table, this granular soil shall be considered equivalent to a fluid with a density of 150 pounds per cubic foot.

A combination of passive earth resistance and friction may be utilized provided that the friction component of the total is divided by 1.5.

5.6 LATERAL PRESSURES

Parameters, as presented within this section, are for backfills which will consist of drained soil placed and compacted in accordance with the recommendations presented herein.

The lateral pressures imposed upon subgrade facilities will, therefore, be basically dependent upon the relative rigidity and movement of the backfilled structure. For active walls, such as retaining walls which can move outward (away from the backfill), drained backfill may be considered equivalent to a fluid with a density of 40 pounds per cubic foot in computing lateral pressures. For more rigid subgrade walls that are not more than 10 inches thick, granular backfill may be considered equivalent to a fluid with a density of 50 pounds per cubic foot. For very rigid non-yielding walls, granular backfill shall be considered equivalent to a fluid with a density of at least 60 pounds per cubic foot. The above values assume that the surface of the soils slope behind the wall is horizontal and that the granular fill within 3 feet of the wall will be compacted with hand-operated compacting equipment.

For seismic loading of below-grade walls, the uniform lateral pressures below, in pounds per square foot (psf), shall be added based on wall depth and wall case:

IBC 2021

Uniform Lateral Pressures			
Wall Height (Feet)	**Active Pressure Case (psf)**	**Moderately Yielding Case (psf)**	**At Rest/Non-Yielding Case (psf)**
4	45	78	112
6	68	118	168
8	90	157	224
10	113	196	280

IBC 2024

Uniform Lateral Pressures			
Wall Height (Feet)	**Active Pressure Case (psf)**	**Moderately Yielding Case (psf)**	**At Rest/Non-Yielding Case (psf)**
4	41	74	107
6	62	111	160
8	83	148	213
10	103	185	266

5.7 FLOOR SLABS

Floor slabs may be established upon suitable natural subgrade soils or structural fill extending to suitable natural soils. Under no circumstances shall floor slabs be established directly over non-engineered fills, loose or disturbed soils, sod, rubbish, construction debris, other deleterious materials, frozen soils, or within ponded water.

Groundwater was measured as shallow as 2.8 feet below the ground surface. GSH recommends placing floor slabs no closer than 4 feet from the highest groundwater elevation. Site grading fill may be utilized to raise the overall grade to achieve the required separation between the floor slab and the highest groundwater elevation.

To facilitate curing of the concrete and to provide a capillary moisture break, it is recommended that floor slabs be directly underlain by at least 4 inches of "free-draining" fill, such as "pea" gravel or three-quarters to one inch minus clean gap-graded gravel.

If a vapor barrier is proposed to be utilized, GSH must be contacted for additional recommendations.

Settlement of lightly loaded floor slabs designed according to previous recommendations (average uniform pressure of 200 pounds per square foot or less) is anticipated to be less than one-quarter of an inch.

GSH must be notified prior to the placement of structural site grading fills, floor slabs, footings, and pavements to verify that all loose/disturbed soils and non-engineered fills have been completely removed and/or properly prepared. For more information refer to Section 5.11 of this report.

5.8 PAVEMENTS

The natural clay soils and non-engineered fills will exhibit poor pavement support characteristics when saturated. All pavement areas must be prepared as previously discussed (see Section 5.2.1, Site Preparation). Under no circumstances shall pavements be established over unprepared non-engineered fills, loose or disturbed soils, topsoil, surface vegetation, root systems, rubbish, construction debris, other deleterious materials, frozen soils, or within ponded water. Even with proper preparation, pavements established overlying non-engineered fills may encounter some long-term movements unless the non-engineered fills are completely removed. With an estimated California Bearing Ratio (CBR) of 5 percent of the subgrade soils, a design life of 20 years and the estimated projected traffic as discussed in Section 2, Proposed Construction, the following pavement sections are recommended:

Parking Areas

(Light Volume of Automobiles and Light Trucks,
Occasional Medium-Weight Trucks,
Occasional AASHTO HS20-44 Firetruck,
and No Heavyweight Trucks)
[5 equivalent 18-kip axle loads per day]

Flexible Pavements:
(Asphalt Concrete)

3.0 inches	Asphalt concrete
8.0 inches	Aggregate base (roadbase)
Over	Properly prepared fills, natural subgrade soils, and/or structural site grading fill extending to properly prepared fills and/or natural subgrade soils

Rigid Pavements:
(Non-reinforced Concrete)

5.0 inches	Portland cement concrete (non-reinforced)
5.0 inches	Aggregate base (roadbase)
Over	Properly prepared natural subgrade soils and/or structural site grading fill extending to properly prepared natural subgrade soils

Drive Lanes

(Light Volume of Automobiles and Light Trucks,
Occasional Medium-Weight Trucks,
Occasional AASHTO HS20-44 Firetruck,
and No Heavyweight Trucks)
[10 equivalent 18-kip axle loads per day]

Flexible Pavements:
(Asphalt Concrete)

3.5 inches	Asphalt concrete
9.0 inches	Aggregate base (roadbase)
Over	Properly prepared fills, natural subgrade soils, and/or structural site grading fill extending to properly prepared fills and/or natural subgrade soils

Rigid Pavements:
(Non-reinforced Concrete)

6.0 inches	Portland cement concrete (non-reinforced)
6.0 inches	Aggregate base (roadbase)

Over	Properly prepared natural subgrade soils and/or structural site grading fill extending to properly prepared natural subgrade soils

*GSH must verify that the subgrade in the indicated areas has a CBR of 5 percent or greater. GSH also recommends that the anticipated traffic counts be confirmed.

For dumpster pads, we recommend a pavement section consisting of 8.0 inches of Portland cement concrete, 12.0 inches of aggregate base, over properly prepared natural subgrade or site grading structural fills. Dumpster pads shall not be constructed overlying non-engineered fills under any circumstances.

The above rigid pavement sections are for non-reinforced Portland cement concrete. Concrete shall be designed in accordance with the American Concrete Institute (ACI) and joint details shall conform to the Portland Cement Association (PCA) guidelines. The concrete shall have a minimum 28-day unconfined compressive strength of 4,000 pounds per square inch and contain 6 percent ±1 percent air-entrainment.

The road base must conform to applicable sections of the current Utah Department of Transportation (UDOT) Standard Specifications. All asphalt material and paving operations shall meet applicable specifications of the Asphalt Institute and UDOT. A GSH technician shall observe placement and perform density testing of the road base material and asphalt. Please note that the 3rd column of Table 2 of Section 02721 "Job Mix Gradation Tolerance" does not apply to road base utilized on this project and all road base must meet the requirements of the 2nd column of Table 2 of Section 02721 "Job Mix Gradation Target Blend" without any additional tolerances. The aggregate gradation limits presented on Table 2 of Section 02721, Untreated Base Course (UTBC) (road base) of UDOT 2025 Standard Specification for Road and Bridge Construction are tabulated below:

Gradation Limits	
Sieve Size	**Job Mix Gradation Target Blend**
1 1/2 inch	100
1 inch	90-100
3/4 inch	70-85
1/2 inch	65-80
3/8 inch	55-75
No. 4	40-65
No. 16	25-40
No. 200	7-11

Percent passing based on total aggregate (dry weight), and fine and coarse aggregate having approximately the same bulk specific gravities.

Please note that the recommended pavement section is based on estimated post-construction traffic loading. If the pavement is to be constructed and utilized by construction traffic, the above pavement section may prove insufficient for heavy truck traffic, such as concrete trucks or tractor-trailers used for construction delivery. Unexpected distress, reduced pavement life, and/or premature failure of the pavement section could result if subjected to heavy construction traffic and the owner should be made aware of this risk. If the estimated traffic loading stated herein is not correct, GSH must review actual pavement loading conditions to determine if revisions to these recommendations are warranted.

GSH must be notified prior to the placement of structural site grading fills, floor slabs, footings, and pavements to verify that all loose/disturbed soils and non-engineered fills have been completely removed and/or properly prepared. For more information refer to Section 5.11 of this report.

5.9 CEMENT TYPES

The laboratory tests indicate that the natural soils tested contain a negligible amount of water-soluble sulfates. Based on our test results, concrete in contact with the on-site soil will have a low potential for sulfate reaction (ACI 318, Table 4.3.1). Therefore, all concrete which will be in contact with the site soils may be prepared using Type I or IA cement.

5.10 GEOSEISMIC SETTING

5.10.1 General

Utah municipalities have adopted the International Building Code (IBC) 2021 and will adopt IBC 2024 in July of 2026. The IBC 2021 code refers to ASCE 7-16, and the IBC 2024 Code refers to ASCE 7-22, Minimum Design Loads and Associated Criteria for Buildings and Other Structures (ASCE 7-16, 7-22) determines the seismic hazard for a site based upon mapping of bedrock accelerations prepared by the United States Geologic Survey (USGS) and the soil site class. The USGS values are presented on maps incorporated into the IBC code and are also available based on latitude and longitude coordinates (grid points).

5.10.2 Faulting

Based on our review of available literature, no active faults pass through or immediately adjacent to the site. The nearest active mapped fault consists of the Provo Section of the Wasatch Fault, located about 2.4 miles to the northeast of the site.

5.10.3 Site Class

For dynamic structural analysis, the Site Class D – Default Soil Profile as defined in Chapter 20 of ASCE 7-16 (per Section 1613.3.2, Site Class Definitions, of IBC 2021) or Site Class Default as defined in Chapter 20 of ASCE 7-22 can be utilized. If a measured site class is desired based on

the project structural engineer's evaluation and recommendations, additional testing and analysis can be completed by GSH to determine the measured site class. Please contact GSH for additional information.

5.10.4 Ground Motions

IBC 2021

The IBC 2021 code is based on USGS mapping, which provides values of short and long period accelerations for average bedrock values for the Western United States and must be corrected for local soil conditions. The following table summarizes the peak ground and short and long period accelerations for the MCE event and incorporates the appropriate soil amplification factor for a Site Class D – Default* Soil Profile. Based on the site latitude and longitude (40.1276 degrees north and 111.6396 degrees west, respectively) and Risk Category II, the values for this site are tabulated below:

Spectral Acceleration Value, T	Bedrock Boundary [mapped values] (% g)	Site Coefficient	Site Class D - Default* [adjusted for site class effects] (% g)	Design Values** (% g)
0.2 Seconds (Short Period Acceleration)	S_S = 145.8	F_a = 1.200	S_{MS} = 175.0	S_{DS} = 116.6
1.0 Second (Long Period Acceleration)	S_1 = 53.5	F_v = 1.765	S_{M1} = 94.4	S_{D1} = 62.9

* If a measured site class in accordance with IBC 2021/ASCE 7-16 is beneficial based on the project structural engineer's review, please contact GSH for additional options for obtaining this measured site class.

**IBC 2021/ASCE 7-16 may require a site-specific study based on the project structural engineer's evaluation and recommendations. If needed, GSH can provide additional information and analysis including a complete site-specific study in accordance with chapter 21 of ASCE 7-16.

IBC 2024

The IBC 2024 code is based on USGS mapping, which provides values of short and long period accelerations for average bedrock values for the Western United States and must be corrected for local soil conditions. The following table summarizes the peak ground and short and long period accelerations for the MCE event and incorporates the appropriate soil amplification factor for a Site Class Default* Soil Profile. Based on the site latitude and longitude (40.1276 degrees north

and 111.6396 degrees west, respectively) and Risk Category II, the values for this site are tabulated below:

Spectral Acceleration Value, T	Bedrock Boundary [mapped values] (g)	Site Class Default* [adjusted for site class effects] (g)	Design Values (g)
0.2 Seconds (Short Period Acceleration)	S_S = 1.60	S_{MS} = 1.67	S_{DS} = 1.11
1.0 Second (Long Period Acceleration)	S_1 = 0.58	S_{M1} = 1.20	S_{D1} = 0.80

* If a measured site class in accordance with IBC 2024/ASCE 7-22 is beneficial based on the project structural engineer's review, please contact GSH for additional options for obtaining this measured site class.

5.10.5 Liquefaction

The site is located in an area that has been identified by the Utah Geological Survey (UGS) as being a "high" liquefaction potential zone. Liquefaction is defined as the condition when saturated, loose, granular soils lose their support capabilities because of excessive pore water pressure, which develops during a seismic event. Clayey soils, even if saturated, will generally not liquefy during a major seismic event.

Due to the clayey nature of the soils, liquefaction is not anticipated to occur within the soils encountered at this site.

5.11 SITE VISITS

GSH Geotechnical must be contacted to verify that all topsoil/disturbed soils and any other unsuitable soils have been removed, non-engineered fills have been removed and/or properly prepared, and that suitable soils have been encountered prior to placing site grading fills, footings, slabs, and pavements. GSH recommends that a pre-construction meeting be held with the general contractor, the earthwork contractor, and a member of the GSH Geotechnical engineering team. **To inquire about or schedule site visits/excavation observations, please contact dispatch@gshgeotech.com**

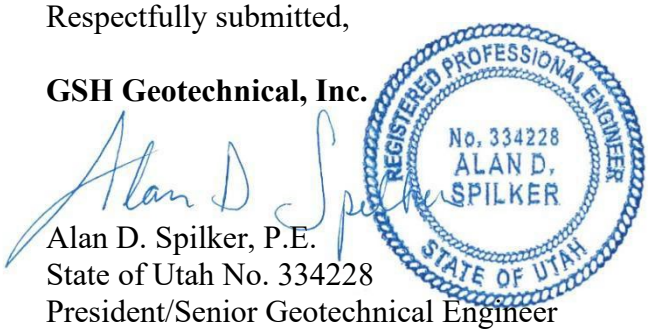
Additionally, GSH Materials Testing and Inspection or another testing agency must observe fill placement and verify in-place moisture content and density of fill materials placed at the site. **To inquire about or to schedule materials testing and other inspections, please contact dispatch@gshmti.com.**

6. CLOSURE

If you have any questions or would like to discuss these items further, please feel free to contact us at (801) 685-9190.

Respectfully submitted,

GSH Geotechnical, Inc.



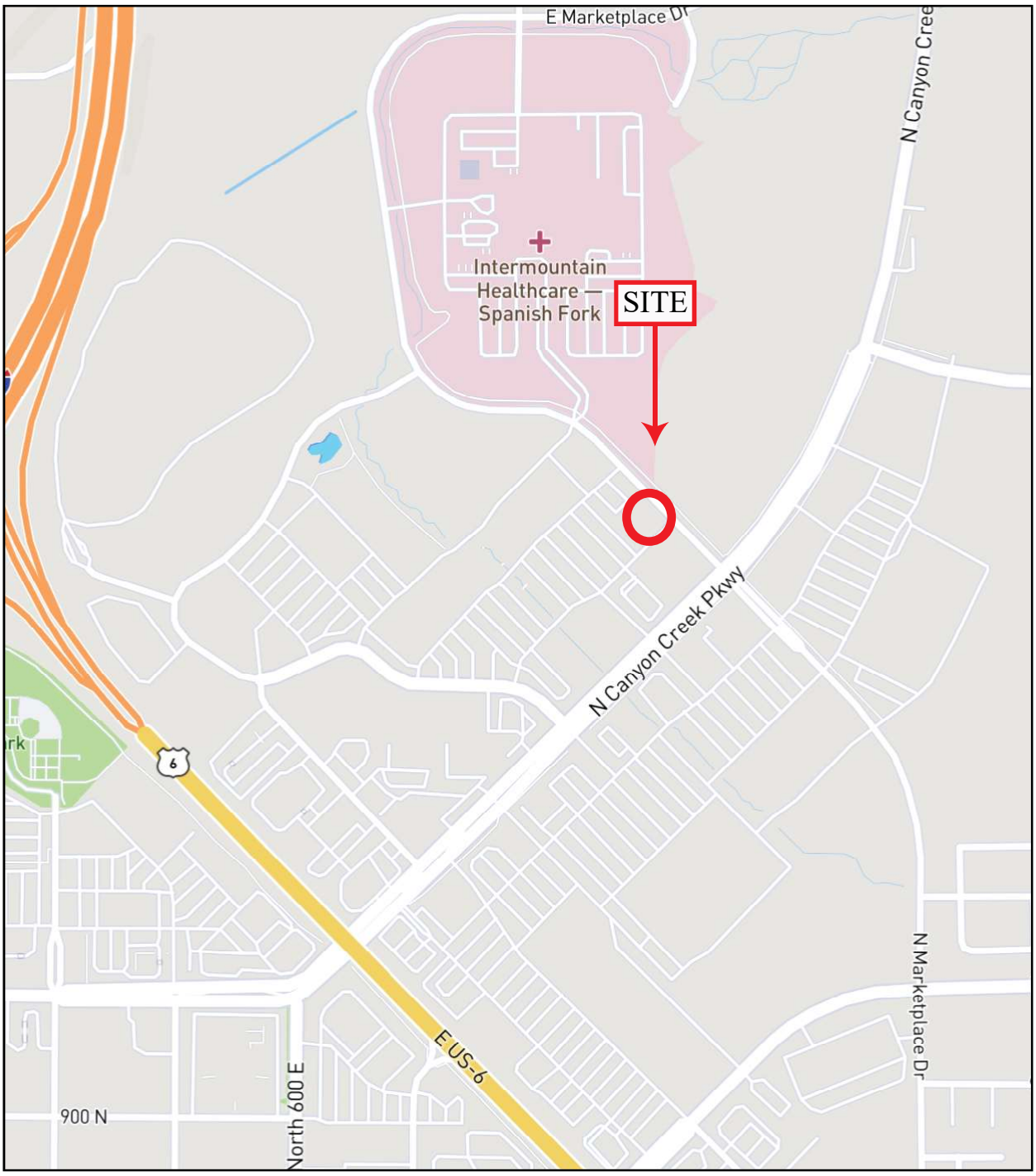
Alan D. Spilker, P.E.
State of Utah No. 334228
President/Senior Geotechnical Engineer

ADS:jmt

Encl. Figure 1, Vicinity Map
Figure 2, Site Plan
Figures 3A through 3E, Boring Logs
Figure 4, Key to Boring Log (USCS)

Addressee (email)

BANK OF UTAH
JOB NO. 4314-001-26



0.05 0 0 0 0 0 0.05 0.1 0.15 0.2 MI 0.25

100 0 0 100 200 300 M 400

REFERENCE:
ALL TRAILS - NATIONAL GEOGRAPHIC TERRAIN
DATED 2026

FIGURE 1
VICINITY MAP

GSH

BANK OF UTAH
JOB NO. 4314-001-26

FIGURE 2
SITE PLAN

REFERENCE:
ADAPTED FROM DRAWING ENTITLED
“BANK OF UTAH - IMPROVEMENT PLANS - COVER/INDEX SHEET” BY REEVE & ASSOCATES, INC., DATED 3/2026

GSH

BORING LOG

Page: 1 of 2

BORING: B-1

CLIENT: Bank of Utah | PROJECT NUMBER: 4314-001-26

PROJECT: Bank of Utah | DATE STARTED: 6/12/26 | DATE FINISHED: 6/12/26

LOCATION: 1351 North Canyon Creek Parkway, Spanish Fork, Utah (40.1276°, -111.6396°) | GSH FIELD REP.: BB

DRILLING METHOD/EQUIPMENT: 4-1/4" ID Hollow-Stem Auger | HAMMER: Automatic | WEIGHT: 140 lbs | DROP: 30"

GROUNDWATER DEPTH: 2.8' (6/29/26) | ELEVATION: ---

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
		Ground Surface	0								
	SM FILL	SILTY FINE TO MEDIUM SAND, FILL with fine gravel; brown									slightly moist dense
▼	CL	SILTY CLAY light brown		3							saturated soft
			5	2					36	17	
		grades light brown/brown	10	3							
		grades with trace fine sand; light brown/brown	15	2							
			20	7					41	18	medium stiff
			25								

See Subsurface Conditions section in the report for additional information.

FIGURE 3A

GSH

BORING LOG

Page: 2 of 2

BORING: B-1

CLIENT: Bank of Utah | PROJECT NUMBER: 4314-001-26

PROJECT: Bank of Utah | DATE STARTED: 6/12/26 | DATE FINISHED: 6/12/26

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
			25								
		grades with layers of fine sand up to 1/8" thick; brown		12							stiff
			30	11							
		grades trace fine sand	35	11					48	21	
		grades brown/light gray	40	12							
			45	7							medium stiff
		End of Exploration at 46.5'. Installed 1.25" diameter slotted PVC pipe to 46.5'.	50								

See Subsurface Conditions section in the report for additional information.

FIGURE 3A
(continued)

BORING LOG

Page: 1 of 1

BORING: B-2

CLIENT: Bank of Utah | PROJECT NUMBER: 4314-001-26

PROJECT: Bank of Utah | DATE STARTED: 6/12/26 | DATE FINISHED: 6/12/26

LOCATION: 1351 North Canyon Creek Parkway, Spanish Fork, Utah (40.1276°, -111.6396°) | GSH FIELD REP.: BB

DRILLING METHOD/EQUIPMENT: 4-1/4" ID Hollow-Stem Auger | HAMMER: Automatic | WEIGHT: 140 lbs | DROP: 30"

GROUNDWATER DEPTH: 4.0' (6/29/26) | ELEVATION: ---

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
		Ground Surface	0								
	SM FILL	SILTY FINE TO MEDIUM SAND, FILL with fine gravel; brown									slightly moist dense
	CL	SILTY CLAY with fine sand; brown									moist medium stiff
▼				7							saturated
			5	4							
		grades with trace fine sand; brown									
			10	6							
		grades with trace fine to medium sand									
			15	7		31.4	89				
		End of Exploration at 16.5'. Installed 1.25" diameter slotted PVC pipe to 16.5'.									
			20								
			25								

See Subsurface Conditions section in the report for additional information.

FIGURE 3B

BORING LOG

Page: 1 of 1

BORING: B-3

CLIENT: Bank of Utah | PROJECT NUMBER: 4314-001-26

PROJECT: Bank of Utah | DATE STARTED: 6/12/26 | DATE FINISHED: 6/12/26

LOCATION: 1351 North Canyon Creek Parkway, Spanish Fork, Utah (40.1276°, -111.6396°) | GSH FIELD REP.: BB

DRILLING METHOD/EQUIPMENT: 4-1/4" ID Hollow-Stem Auger | HAMMER: Automatic | WEIGHT: 140 lbs | DROP: 30"

GROUNDWATER DEPTH: Not Encountered (6/12/26) | ELEVATION: ---

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
		Ground Surface	0								
	SM FILL	SILTY FINE TO MEDIUM SAND, FILL with fine gravel; brown									slightly moist dense
	CL	SILTY CLAY with some fine sand; brown	5								moist medium stiff
		End of Exploration at 5.5'. No groundwater encountered at time of drilling.	10								
			15								
			20								
			25								

See Subsurface Conditions section in the report for additional information.

FIGURE 3C

BORING LOG

Page: 1 of 1

BORING: B-4

CLIENT: Bank of Utah | PROJECT NUMBER: 4314-001-26

PROJECT: Bank of Utah | DATE STARTED: 6/12/26 | DATE FINISHED: 6/12/26

LOCATION: 1351 North Canyon Creek Parkway, Spanish Fork, Utah (40.1276°, -111.6396°) | GSH FIELD REP.: BB

DRILLING METHOD/EQUIPMENT: 4-1/4" ID Hollow-Stem Auger | HAMMER: Automatic | WEIGHT: 140 lbs | DROP: 30"

GROUNDWATER DEPTH: Not Encountered (6/12/26) | ELEVATION: ---

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
		Ground Surface	0								
	SM FILL	SILTY FINE TO MEDIUM SAND, FILL with fine gravel; brown									slightly moist dense
	CL	FINE TO COARSE SANDY CLAY with trace fine gravel; brown									moist medium stiff
			5								
		End of Exploration at 5.5'. No groundwater encountered at time of drilling.									
			10								
			15								
			20								
			25								

See Subsurface Conditions section in the report for additional information.

FIGURE 3D

BORING LOG

Page: 1 of 1

BORING: B-5

CLIENT: Bank of Utah | PROJECT NUMBER: 4314-001-26

PROJECT: Bank of Utah | DATE STARTED: 6/12/26 | DATE FINISHED: 6/12/26

LOCATION: 1351 North Canyon Creek Parkway, Spanish Fork, Utah (40.1276°, -111.6396°) | GSH FIELD REP.: BB

DRILLING METHOD/EQUIPMENT: 4-1/4" ID Hollow-Stem Auger | HAMMER: Automatic | WEIGHT: 140 lbs | DROP: 30"

GROUNDWATER DEPTH: Not Encountered (6/12/26) | ELEVATION: ---

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
		Ground Surface	0								
	SM FILL	SILTY FINE TO MEDIUM SAND, FILL with fine gravel; brown									slightly moist dense
	CL	SILTY CLAY with trace fine to coarse sand; brown									moist medium stiff
			5			20.0		72.6			
		End of Exploration at 5.5'. No groundwater encountered at time of drilling.									
			10								
			15								
			20								
			25								

See Subsurface Conditions section in the report for additional information.

FIGURE 3E

CLIENT: Bank of Utah
PROJECT: Bank of Utah
PROJECT NUMBER: 4314-001-26

KEY TO BORING LOG

WATER LEVEL	USCS	DESCRIPTION	DEPTH (FT.)	BLOW COUNT	SAMPLE SYMBOL	MOISTURE (%)	DRY DENSITY (PCF)	% PASSING 200	LIQUID LIMIT (%)	PLASTICITY INDEX	REMARKS
①	②	③	④	⑤	⑥	⑦	⑧	⑨	⑩	⑪	⑫

COLUMN DESCRIPTIONS

① **Water Level:** Depth to measured groundwater table. See symbol below.

② **USCS:** (Unified Soil Classification System) Description of soils encountered; typical symbols are explained below.

③ **Description:** Description of material encountered; may include color, moisture, grain size, density/consistency,

④ **Depth (ft.):** Depth in feet below the ground surface.

⑤ **Blow Count:** Number of blows to advance sampler 12" beyond first 6", using a 140-lb hammer with 30" drop.

⑥ **Sample Symbol:** Type of soil sample collected at depth interval shown; sampler symbols are explained below.

⑦ **Moisture (%):** Water content of soil sample measured in laboratory; expressed as percentage of dryweight of

⑧ **Dry Density (pcf):** The density of a soil measured in laboratory; expressed in pounds per cubic foot.

⑨ **% Passing 200:** Fines content of soils sample passing a No. 200 sieve; expressed as a percentage.

⑩ **Liquid Limit (%):** Water content at which a soil changes from plastic to liquid behavior.

⑪ **Plasticity Index (%):** Range of water content at which a soil exhibits plastic properties.

⑫ **Remarks:** Comments and observations regarding drilling or sampling made by driller or field personnel. May include other field and laboratory test results using the following abbreviations:

CEMENTATION:

Weakly: Crumbles or breaks with handling or slight finger pressure.

Moderately: Crumbles or breaks with considerable finger pressure.

Strongly: Will not crumble or break with finger pressure.

MODIFIERS:

Trace <5%

Some 5-12%

With > 12%

MOISTURE CONTENT (FIELD TEST):

Dry: Absence of moisture, dusty, dry to the touch.

Moist: Damp but no visible water.

Saturated: Visible water, usually soil below water table.

Descriptions and stratum lines are interpretive; field descriptions may have been modified to reflect lab test results. Descriptions on the logs apply only at the specific boring locations and at the time the borings were advanced; they are not warranted to be representative of subsurface conditions at other locations or times.

UNIFIED SOIL CLASSIFICATION SYSTEM (USCS)

MAJOR DIVISIONS			USCS SYMBOLS	TYPICAL DESCRIPTIONS
COARSE-GRAINED SOILS More than 50% of material is larger than No. 200 sieve size.	GRAVELS More than 50% of coarse fraction retained on No. 4 sieve.	CLEAN GRAVELS (little or no fines)	GW	Well-Graded Gravels, Gravel-Sand Mixtures, Little or No Fines
			GP	Poorly-Graded Gravels, Gravel-Sand Mixtures, Little or No Fines
		GRAVELS WITH FINES (appreciable amount of fines)	GM	Silty Gravels, Gravel-Sand-Silt Mixtures
			GC	Clayey Gravels, Gravel-Sand-Clay Mixtures
	SANDS More than 50% of coarse fraction passing through No. 4 sieve.	CLEAN SANDS (little or no fines)	SW	Well-Graded Sands, Gravelly Sands, Little or No Fines
			SP	Poorly-Graded Sands, Gravelly Sands, Little or No Fines
		SANDS WITH FINES (appreciable amount of fines)	SM	Silty Sands, Sand-Silt Mixtures
			SC	Clayey Sands, Sand-Clay Mixtures
FINE-GRAINED SOILS More than 50% of material is smaller than No. 200 sieve size.	SILTS AND CLAYS Liquid Limit less than 50%		ML	Inorganic Silts and Very Fine Sands, Rock Flour, Silty or Clayey Fine Sands or Clayey Silts with Slight Plasticity
			CL	Inorganic Clays of Low to Medium Plasticity, Gravelly Clays, Sandy Clays, Silty Clays, Lean Clays
			OL	Organic Silts and Organic Silty Clays o f Low Plasticity
	SILTS AND CLAYS Liquid Limit greater than 50%		MH	Inorganic Silts, Micacious or Diatomacious Fine Sand or Silty Soils
			CH	Inorganic Clays of High Plasticity, Fat Clays
			OH	Organic Silts and Organic Clays of Medium to High Plasticity
HIGHLY ORGANIC SOILS			PT	Peat, Humus, Swamp Soils with High Organic Contents

Note: Dual Symbols are used to indicate borderline soil classifications.

STRATIFICATION:

DESCRIPTION	THICKNESS
Seam	up to 1/8"
Layer	1/8" to 12"

Occasional: One or less per 6" of thickness

Numerous; More than one per 6" of thickness

TYPICAL SAMPLER GRAPHIC SYMBOLS

- Bulk/Bag Sample
- Standard Penetration Split Spoon Sampler
- Rock Core
- No Recovery
- 3.25" OD, 2.42" ID D&M Sampler
- 3.0" OD, 2.42" ID D&M Sampler
- California Sampler
- Thin Wall

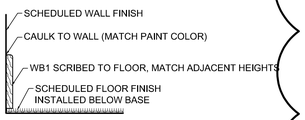
WATER SYMBOL

Water Level

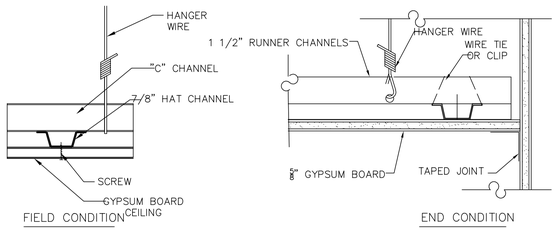
FIGURE 4

GSH

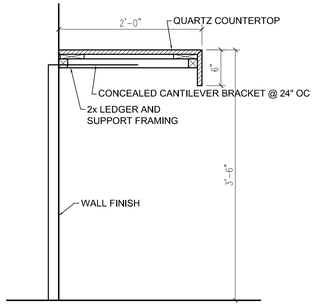
PLOT DATE: 08/27/2026



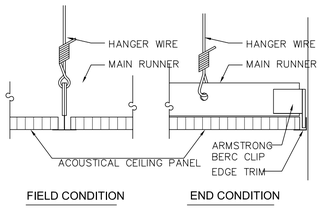
15 BASE DETAIL
1-1/2" = 1'-0"



12 DRYWALL CEILING DETAIL
3" = 1'-0"



8 DETAIL @ RESTROOM COUNTER
1" = 1'-0"



11 CEILING TILE DETAIL
3" = 1'-0"

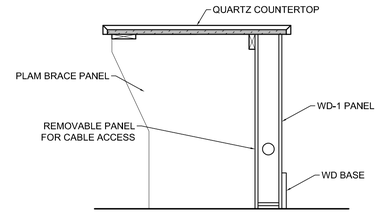
Notes:
1. A ceiling area of 144sf or less surrounded by walls that connect directly to structure above shall be exempt from the following lateral design requirements.

2. In each orthogonal direction, one end of the ceiling grid shall be attached to the closure angle. The other end in each direction shall be attached with an ICC evaluated & approved seismic clip system and .75" of clearance to allow free horizontal movement.

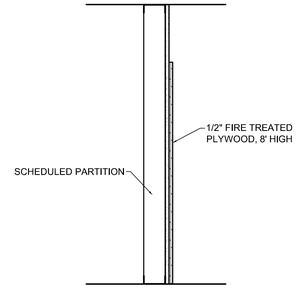
3. Lateral ceiling bracing is required @ 12'-0" o.c. in both directions for all ceilings greater than 1000sf.

4. Ceiling areas over 2500sf must have seismic separation joints.

5. Light fixtures, mechanical equipment, etc. must be supported independent of the ceiling support system.



7 SECTION @ TELLER DESK
1" = 1'-0"



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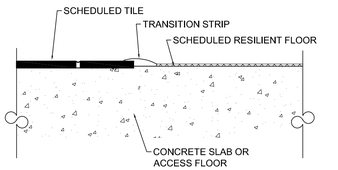
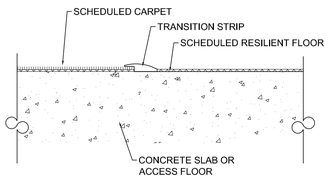
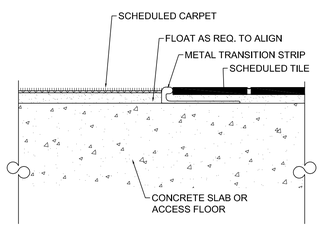
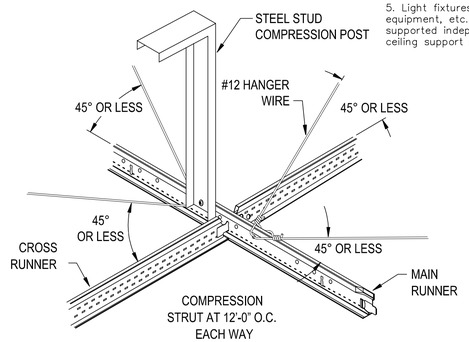


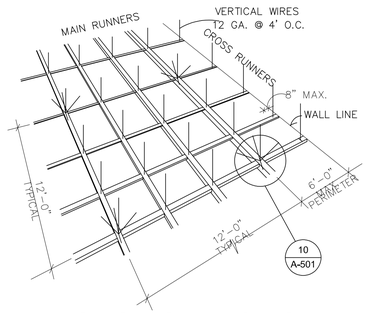




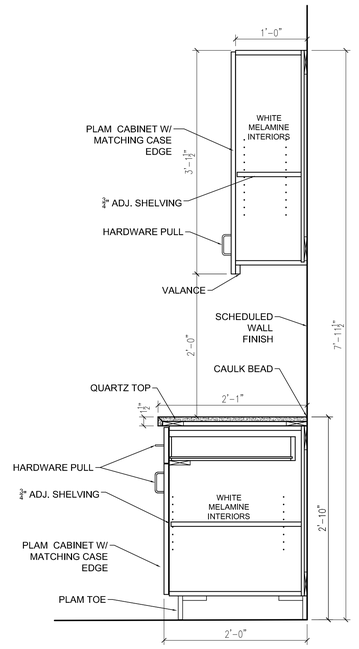
13 THRESHOLD DETAILS
3" = 1'-0"



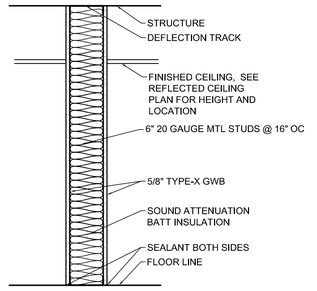
10 SEISMIC BRACING DETAIL
3" = 1'-0"



9 SEISMIC BRACING DETAIL
3" = 1'-0"



5 TYPICAL CABINET DETAIL
1" = 1'-0"



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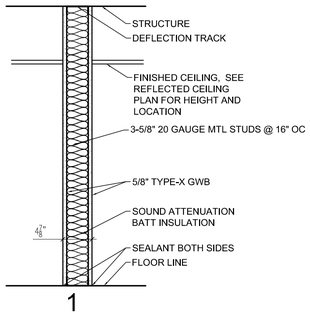


1

1 WALL PARTITION DETAILS
1" = 1'-0"

tjs architects

BANK OF UTAH
PERMIT SET
1351 NORTH CANYON CREEK PKWY
SPANISH FORK, UT

SEAL:

ISSUANCES:

PERMIT SET	6.22.26
ADD 001	8.27.26

PROJECT:

NUMBER:	26002
DATE:	6.22.26

SHEET:

A501

APPLIANCE SCHEDULE

ROOM NAME	ITEM	MANUFACTURER	MODEL #	REMARKS
CORRIDOR 1011	ELECTRIC WATER COOLER			SEE PLUMBING SCHEDULE
	FEC	LARSENS	2409	STAINLESS STEEL
WELLNESS 1004	REFRIGERATOR	SUMMIT	ASDS2413	
	MICROWAVE	GE	PCST221WSS	
BREAK 1021	REFRIGERATOR	GE	PWE23KYNFS	
	WATER DISPENSER	WATERSTAR	WS-FPI15	BY OWNER
	DISH WASHER	GE	PDT715SYVFS	
	MICROWAVE	GE	PCST221WSS	

NOTES:
1. VERIFY FINAL SELECTIONS WITH OWNER.
2. CONTRACTOR TO COORDINATE ALL POWER REQUIREMENTS FOR OWNER PROVIDED EQUIPMENT.
3. CONTRACTOR TO COORDINATE ALL PLUMBING SUPPLY/DRAIN LINES FOR OWNER PROVIDED EQUIPMENT.

TOILET ACCESSORY SCHEDULE

ROOM NAME		ITEM	MANUFACTURER	MODEL #	REMARKS
RESTROOMS 1012 1013	A	42" GRAB BAR	BOBRICK	B-9806 SERIES	
	B	36" GRAB BAR	BOBRICK	B-9806 SERIES	
	C	18" VERTICAL GRAB BAR	BOBRICK	B-9806 SERIES	
	D	TP DISPENSER			BY OWNER
	E	FEMININE PRODUCT DISPOSAL	BOBRICK	B-270	
	F	MIRROR	BOBRICK	B-2908 2436	
	G	SOAP DISPENSER			BY OWNER
	H	PAPER TOWEL DISPENSER			BY OWNER
	J	TRASH CAN	SIMPLE HUMAN	WBB2328641	

NOTES:
1. SELECTIONS TO BE FINALIZED BY OWNER PRIOR TO ORDER.
2. GC TO PROVIDE BACKING/BLOCKING AT ALL ACCESSORY LOCATIONS.
3. STANDARD MIRRORS TO BE ONE-PIECE, ROLL-FORMED 3/4" X 3/4" ANGLE-FRAME, STAINLESS STEEL ANGLE WITH SATIN FINISH. CORNERS WELDED, GROUND AND POLISHED SMOOTH. BEVELED FRAME EDGE AT MIRROR FOR IMPROVED APPEARANCE. NO. 1 QUALITY, 1/4" (6MM) GLASS MIRROR; WARRANTED AGAINST SILVER SPOILAGE FOR 15 YEARS. GALVANIZED STEEL BACK. SECURED TO CONCEALED WALL HANGER WITH THEFT-RESISTANT MOUNTING.

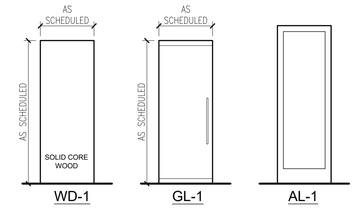
SIGNAGE SCHEDULE

ROOM NAME	ITEM	MANUFACTURER	FINISH
MENS 1012	RESTROOM SIGN	8"X8" SQUARE WITH ROUNDED EDGES	FROSTED CLEAR ACRYLIC WITH STAINLESS STEEL CAPS, CHARCOAL GREY ICONS, TEXT, AND BRAILLE
WOMENS 1013	RESTROOM SIGN	8"X8" SQUARE WITH ROUNDED EDGES	FROSTED CLEAR ACRYLIC WITH STAINLESS STEEL CAPS, CHARCOAL GREY ICONS, TEXT, AND BRAILLE

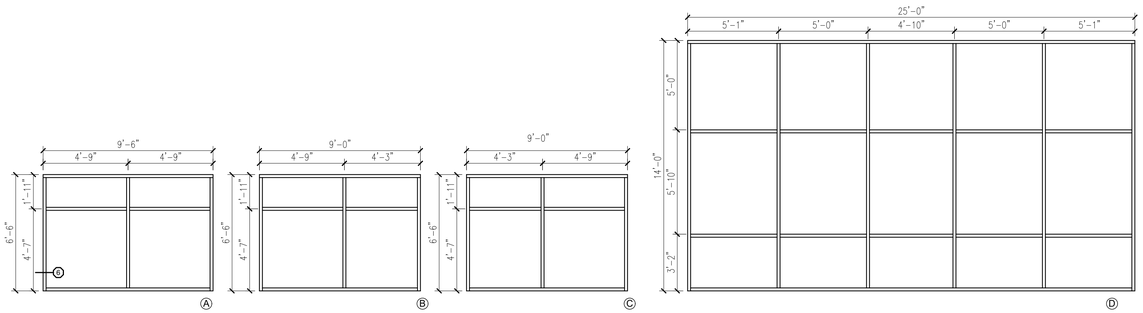
DOOR SCHEDULE

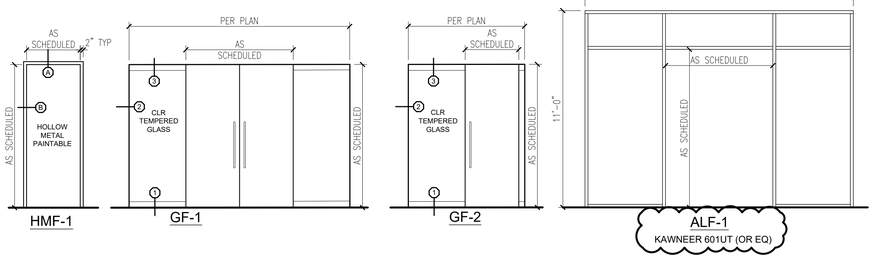
#	SIZE	THK	DOOR TYPE	FRAME TYPE	LOCK SET	CLOSER	RATING	REMARKS
1001A	(2)3'-0" x 9'-0"		AL-1	ALF-1	PANIC	YES	---	OPENER, CARD READER, CONTINUOUS HINGE, ELECTRONIC PANIC W/ INTERNAL BOLTS
1002A	(2)3'-0" x 9'-0"		GL-1	GF-1	PULL/PULL	YES	---	
1004A	3'-0" x 9'-0"		GL-1	GF-2	PULL/PULL	W/ HOLD OPEN	---	
1005A	3'-0" x 9'-0"	1-3/4"	WD-1	HMF-1	STOREROOM	YES	---	ELECTRIC STRIKE, CARD READER
1006A	3'-0" x 9'-0"	1-3/4"	WD-1	HMF-1	STOREROOM	YES	---	ELECTRIC STRIKE, CARD READER
1007A	3'-0" x 9'-0"	1-3/4"	WD-1	HMF-1	STOREROOM	YES	---	ELECTRIC STRIKE, CARD READER
1008A	3'-0" x 9'-0"	1-3/4"	WD-1	HMF-1	STOREROOM	YES	---	ELECTRIC STRIKE, CARD READER
1009A	3'-0" x 9'-0"	1-3/4"	WD-1	HMF-1	STOREROOM	YES	---	ELECTRIC STRIKE, CARD READER, OCCUPANCY INDICATOR
1012A	3'-0" x 8'-0"	1-3/4"	WD-1	HMF-1	STOREROOM	YES	---	ELECTRIC STRIKE, CARD READER, OCCUPANCY INDICATOR
1013A	3'-0" x 8'-0"	1-3/4"	WD-1	HMF-1	STOREROOM	YES	---	ELECTRIC STRIKE, CARD READER, OCCUPANCY INDICATOR
1014A	3'-0" x 9'-0"		GF-1	GL-2	PULL/PULL	W/ HOLD OPEN	---	
1016A	3'-0" x 9'-0"		GF-1	GL-2	PULL/PULL	W/ HOLD OPEN	---	
1017A	3'-0" x 9'-0"		GF-1	GL-2	PULL/PULL	W/ HOLD OPEN	---	
1018A	3'-0" x 9'-0"		GF-1	GL-2	PULL/PULL	W/ HOLD OPEN	---	
1022A	3'-0" x 9'-0"		GF-1	GL-2	PULL/PULL	W/ HOLD OPEN	---	
1023A	3'-0" x 9'-0"		GF-1	GL-2	PULL/PULL	W/ HOLD OPEN	---	
1024A	3'-0" x 9'-0"		GF-1	GL-2	PULL/PULL	W/ HOLD OPEN	---	

DOOR SCHEDULE NOTES:
1. GC TO VERIFY ALL DOOR SIZES PRIOR TO ORDER.
2. ALL DOORS TO HAVE 4" RETURN ON HINGE SIDE UNLESS OTHERWISE NOTED.
3. 1/2" UNDERCUT AT ALL DOORS UNLESS NOTED OTHERWISE BY HVAC DESIGNER.
4. VERIFY LOCKING OWNER.
5. TYPICAL DOOR HARDWARE TO BE SCHLAGE L SERIES M52 LEVER IN BLACK (622) FINISH WITH MATCHING ESCUTCHEON, COORDINATE CORES WITH OWNER.
6. DOOR OPERATORS TO BE ASSA ABLOY SW150 (OR EQ)
7. DOOR HINGES, CLOSERS, ETC. TO MATCH HARDWARE SELECTION.
8. FLOOR STOP TO BE IVES FS410, US19 FINISH
9. WALL STOP TO BE IVES WS407CCV, US19 FINISH
10. WD DOORS TO BE QUARTER SAWN WALNUT W/ CLEAR FINISH, MATCH ARCH SAMPLE.
11. HM DOORS & FRAMES TO BE SPRAY PAINTED. COLOR PER FINISH SCHEDULE
12. FRAMELESS GLASS ENTRY SYSTEM TO MATCH EXISTING (VERIFY THE FOLLOWING):
    - 12.1. LADDER PULL — 30LPMBL
    - 12.2. BOTTOM PATCH — CRL380HMBL
    - 12.3. TOP PATCH — PH21AMBL
    - 12.4. U-CHANNEL — UCBL3812CL
    - 12.5. MAGLOCK — M490P ATS/LED 24 VDC
    - 12.6. REQUEST TO EXIT — SCANII 24 VDC
    - 12.7. CARD READER — OWNER TO VERIFY
13. MAGLOCKS TO COMPLY WITH THE FOLLOWING:
    - 13.1. OCCUPANT SENSOR TO BE LOCATED ON EGRESS SIDE SHALL UNLOCK DOOR
    - 13.2. LOCKS TO UNLOCK AT LOSS OF POWER
    - 13.3. MANUAL UNLOCKING DEVICE TO BE LOCATED WITHIN 60"
    - 13.4. FIRE ALARM ACTIVATION TO UNLOCK
    - 13.5. LISTED IN ACCORDANCE WITH UL294.



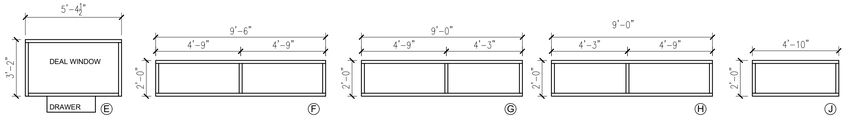
DOOR TYPES

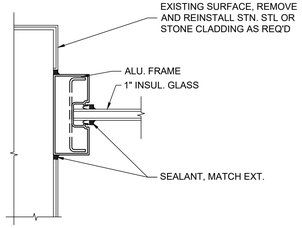




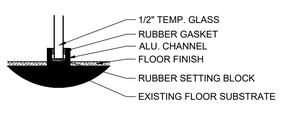
FRAME TYPES

WINDOW NOTES:
1. GC TO VERIFY ALL WINDOW OPENINGS PRIOR TO ORDER.
2. GC TO VERIFY ALL FINISHES WITH OWNER PRIOR TO ORDER.
3. CONTRACTOR TO INSTALL WINDOWS PER MANUF. GUIDELINES AND REQUIREMENTS.
4. ALUMINUM FRAME TO BE KAWNEER 451UT (OR EQ) BLACK ANODIZED.
5. INSULATED GLASS TO BE SOLARBAN 70 (U-VAL: 0.38), TEMPERED AS REQUIRED.
6. DEAL WINDOW TO BE HAMILTON SAFE 035D WITH DEAL DRAWER
7. TEMPERED GLAZING AT ALL THE FOLLOWING LOCATIONS:
   - 7.1. PANES LARGER THAN 9sf
   - 7.2. BOTTOM EDGE OF GLAZING LESS THAN 18" AFF
   - 7.3. TOP EDGE OF GLAZING MORE THAN 36" AFF
   - 7.4. WITHIN 36" OF STAIRS/LANDINGS

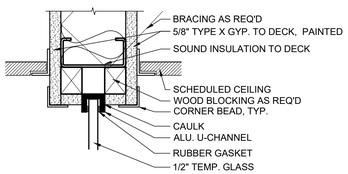




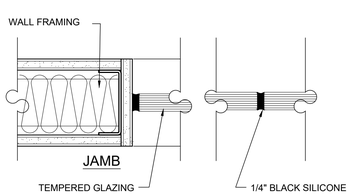
6 JAMB DETAIL (TYP.)
3" = 1'-0"



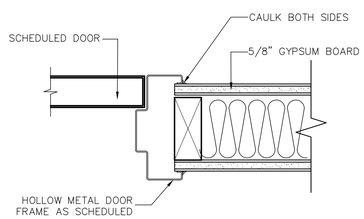
5 GLASS DETAIL, TYP
3" = 1'-0"



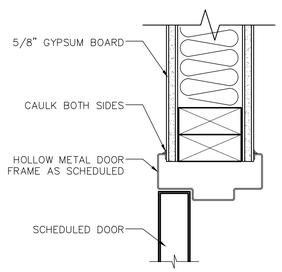
4 GLASS DETAIL @ CLG
3" = 1'-0"



3 DETAIL @ GLASS WALL
3" = 1'-0"



B JAMB DETAIL
3" = 1'-0"



A HEAD DETAIL
3" = 1'-0"

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BANK OF UTAH
PERMIT SET
1351 NORTH CANYON CREEK PKWY
SPANISH FORK, UT

SEAL:

ISSUANCES:

PERMIT SET	6.22.26
ADD 001	8.27.26

PROJECT:

NUMBER:	26002
DATE:	6.22.26

SHEET:

A601

2"
1"
0"

SYMBOL LEGEND

SYMBOL	DESCRIPTION
HVAC PIPING	
HWS	HOT WATER SUPPLY
HWR	HOT WATER RETURN
TWS	TEMPERED WATER SUPPLY
CWS	CHILLED WATER SUPPLY
CWR	CHILLED WATER RETURN
RL	REFRIGERANT LIQUID
RS	REFRIGERANT SUCTION
CDWS	CONDENSER WATER SUPPLY
CDWR	CONDENSER WATER RETURN
D	DRAIN LINE
(E)	EXISTING PIPE
(E)	EXISTING PIPE TO BE REMOVED

ABBREVIATIONS

NOTE: ALL ABBREVIATIONS MAY NOT BE USED

AD	ACCESS DOOR
AIR COND	AIR CONDITION(-ING,-ED)
APD	AIR PRESSURE DROP
BD	BALANCING DAMPER
BHP	BRAKE HORSE POWER
BTU	BRITISH THERMAL UNIT
BTUH	BTU/HOUR
CFH	CUBIC FEET PER HOUR
CFM	CUBIC FEET PER MINUTE
CLG	COOLING
COMP	COMPONENT
COND	CONDENS(-ER, -ING, -ATION)
CV	CONTROL VALVE
CW	COLD WATER
DIA	DIAMETER
DISCH	DISCHARGE
DP	DEPTH OR DEEP
DB	DRY BULB TEMPERATURE
(E)	EXISTING
EER	ENERGY EFFICIENCY RATIO
EFF	EFFICIENCY
EG	ETHYLENE GLYCOL
ELEC	ELECTRIC
ELEV	ELEVATION
ENT	ENTERING
EVAP	EVAPORAT(-E, -ING, -ED, -OR)
EWT	ENTERING WATER TEMP
EXT	EXTERNAL
(F)	FUTURE
F	FAHRENHEIT
FC	FLEXIBLE CONNECTION
FD	FIRE DAMPER
FLA	FULL LOAD AMPS
FPI	FINS PER INCH
FPM	FEET PER MINUTE
FPS	FEET PER SECOND
FSD	FIRE SMOKE DAMPER
FT	FEET
GAL	GALLON(S)
GPH	GALLONS PER HOUR
GPM	GALLONS PER MINUTE
HD	HEAD
HG	MERCURY
HR	HOUR
HT	HEIGHT
HTG	HEATING
HP	HORSE POWER
HW	HOT WATER
HZ	HERTZ(FREQUENCY)
ID	INSIDE DIAMETER
IN	INCH
KW	KILOWATT
LAT	LEAVING AIR TEMPERATURE
LBS	POUNDS
LG	LENGTH
LH	LATENT HEAT
LRA	LOCKED ROTOR AMPS
LVG	LEAVING
LWT	LEAVING WATER TEMP
MBH	THOUSAND BTU PER HOUR
MCA	MINIMUM CIRCUIT AMPS
MFR	MANUFACTURER
MIN	MINIMUM
N/A	NOT APPLICABLE
NC	NORMALLY CLOSED
NC	NOISE CRITERIA
NIC	NOT IN CONTRACT
NO	NORMALLY OPEN
NPSH	NET POSITIVE SUCTION HEAD
NTS	NOT TO SCALE
OA	OUTSIDE AIR
OD	OUTSIDE DIAMETER
OZ	OUNCE
PD	PRESSURE DROP OR DIFF.
PG	PROPYLENE GLYCOL
PH	PHASE
PPM	PARTS PER MILLION
PRESS	PRESSURE
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PSIA	PSI ABSOLUTE
PSIG	PSI GAUGE
R	THERMAL RESISTANCE
RA	RETURN AIR
RECIRC	RECIRCULATE
REFR	REFRIGERATION
REQD	REQUIRED
RPM	REVOLUTIONS PER MINUTE
RW	RAINWATER
SA	SUPPLY AIR
SC	SHADING COEFFICIENT
SCW	SOFT COLD WATER
SF	SAFETY FACTOR
SH	SENSIBLE HEAT
SL	SEA LEVEL
SP	STATIC PRESSURE
SPEC(S)	SPECIFICATION(S)
SQ	SQUARE
STD	STANDARD
TEMP	TEMPERATURE
TSTAT	THERMOSTAT
V	VOLT
VAC	VACUUM
VAV	VARIABLE AIR VOLUME
VEL	VELOCITY
VENT	VENT, VENTILATION
VFD	VARIABLE FREQUENCY DRIVE
WC	WATER COLUMN
WG	WATER GAUGE
WPD	WATER PRESSURE DROP
WB	WET BULB

DEFINITIONS

NOTE: ALL DEFINITIONS MAY NOT BE USED.

INDICATED: THE TERM "INDICATED" REFERS TO GRAPHIC REPRESENTATIONS, NOTES, OR SCHEDULES ON THE DRAWINGS, OTHER PARAGRAPHS OR SCHEDULES IN THE SPECIFICATIONS, AND SIMILAR REQUIREMENTS IN THE CONTRACT DOCUMENTS. WHERE TERMS SUCH AS "SHOWN", "NOTED", "SCHEDULED", AND "SPECIFIED" ARE USED, IT IS TO HELP THE READER LOCATE THE REFERENCE, NO LIMITATION ON LOCATION IS INTENDED.

DIRECTED: TERMS SUCH AS "DIRECTED", "REQUESTED", AUTHORIZED", "SELECTED", "APPROVED", "REQUIRED", AND "PERMITTED" MEAN "DIRECTED BY THE ENGINEER", "REQUESTED BY THE ENGINEER", AND SIMILAR PHRASES.

APPROVE: THE TERM "APPROVED", WHERE USED IN CONJUNCTION WITH THE ENGINEER'S ACTION ON THE CONTRACTOR'S SUBMITTALS, APPLICATIONS, AND REQUESTS, IS LIMITED TO THE ENGINEER'S DUTIES AND RESPONSIBILITIES AS STATED IN GENERAL AND SUPPLEMENTARY CONDITIONS.

FURNISH: THE TERM "FURNISH" IS USED TO MEAN "SUPPLY AND DELIVER TO THE PROJECT SITE, READY FOR UNLOADING, UNPACKING, ASSEMBLY, INSTALLATION, AND SIMILAR OPERATIONS."

INSTALL: THE TERM "INSTALL" IS USED TO DESCRIBE OPERATIONS AT PROJECT SITE INCLUDING THE ACTUAL "UNLOADING, UNPACKING, ASSEMBLY, ERECTION, PLACING, ANCHORING, APPLYING, WORKING TO DIMENSION, FINISHING, CURING, PROTECTING, CLEANING, AND SIMILAR OPERATIONS TO MAKE THE ITEM FULLY OPERATIONAL."

PROVIDE: THE TERM "PROVIDE" MEANS "TO FURNISH AND INSTALL, COMPLETE AND READY FOR THE INTENDED USE."

INSTALLER: AN "INSTALLER" IS THE CONTRACTOR OR AN ENTITY ENGAGED BY THE CONTRACTOR, EITHER AS AN EMPLOYEE, SUBCONTRACTOR, OR SUB-SUBCONTRACTOR, FOR PERFORMANCE OF A PARTICULAR CONSTRUCTION ACTIVITY, INCLUDING INSTALLATION, ERECTION, APPLICATION, AND SIMILAR OPERATIONS. INSTALLERS ARE REQUIRED TO BE EXPERIENCED IN THE OPERATIONS THEY ARE ENGAGED TO PERFORM.

SYMBOL LEGEND

SYMBOL	DESCRIPTION
VALVES, METERS, AND GAUGES	
	SHUT OFF VALVE
	GATE VALVE
	CHECK VALVE
	AUTO 2-WAY VALVE
	AUTO 3-WAY VALVE
	GLOBE VALVE
	BALL VALVE
	RELIEF VALVE
	CHAIN OPERATED GATE VALVE
	PRESSURE REDUCING VALVE
	BUTTERFLY VALVE
	SOLENOID VALVE
	ANGLE VALVE
	VENTURI
	BALANCING OR PLUG COCK
	FLOW SETTER
	EXPANSION VALVE (REFRIG.)
T	TEMPERATURE SENSOR
MAV	MANUAL AIR VENT
	STRAINER
	GAUGE COCK
	FLEXIBLE CONNECTION
	PRESSURE GAUGE
	THERMOMETER
	VICTAULIC COUPLING
	REDUCER CONCENTRIC
	REDUCER ECCENTRIC
	REFRIGERANT SITE GLASS
	REFRIGERANT STRAINER
	REFRIGERANT FILTER DRIER
	90° ELBOW UP
	90° ELBOW DOWN
	90° TEE UP
	90° TEE DOWN
	UNION
	CAPPED PIPE
	ANCHOR
	FLOAT AND THERMOSTATIC TRAP
HVAC SYMBOLS	
T	THERMOSTAT
S	TEMPERATURE SENSOR
H	HUMIDISTAT
PLUMBING SYMBOLS	
C.B.	CATCH BASIN
M.H.	MANHOLE
W.H.	WALL HYDRANT
H.B.	HOSE BIBB
	CLEANOUT TO GRADE
	FLOOR CLEANOUT
	WALL CLEANOUT

SYMBOL LEGEND

SYMBOL	DESCRIPTION	
DUCTWORK		
SINGLE LINE	DOUBLE LINE	DESCRIPTION
		RECTANGULAR SUPPLY DUCT UP
		RECTANGULAR SUPPLY DUCT DOWN
		RECTANGULAR RETURN DUCT UP
		RECTANGULAR RETURN DUCT DOWN
		RECTANGULAR EXHAUST DUCT UP
		RECTANGULAR EXHAUST DUCT DOWN
		ROUND DUCT UP
		ROUND DUCT DOWN
		ACOUSTICALLY LINED RECTANGULAR DUCT
		90° RECTANGULAR ELBOW WITH TURNING VANES
		90° RADIUS ELBOW R=1.5
		DUCT SIZE OR SHAPE TRANSITION
		OPPOSED BLADE BALANCING DAMPER (O.B.D.) IN RECT DUCT
		BUTTERFLY BALANCING DAMPER IN ROUND DUCTS
		COMBINATION TEE
		SPLITTER DAMPER
		SQUARE OR RECTANGULAR CEILING DIFFUSER
		ROUND CEILING DIFFUSER
		SIDEWALL REGISTER SUPPLY OR RETURN
		ROUND FLEXIBLE DUCT
		RETURN GRILLE
		EXHAUST GRILLE
FSD	FSD	FIRE/SMOKE DAMPER
FD	FD	FIRE DAMPER
FC	FC	FLEXIBLE CONNECTION
		EXISTING DUCT
		DUCT TO BE REMOVED

GENERAL MECHANICAL NOTES

1. ALL CEILING DIFFUSERS SHOWN AS SUCH ARE CD-1, CFM AS NOTED, UNLESS OTHERWISE NOTED. (X / CFM)
2. ALL CEILING RETURN GRILLES SHOWN AS SUCH ARE RG-1 UNLESS OTHERWISE NOTED. PROVIDE SOUND BOOT (/)
3. ALL CEILING EXHAUST GRILLES SHOWN AS SUCH ARE EG-1, CFM AS NOTED, UNLESS OTHERWISE NOTED. (X / CFM)
4. DO NOT ROUTE DUCTS AND PIPES ABOVE ELECTRICAL PANELS. ALL ELECTRICAL PANELS MUST HAVE CLEAR ACCESS SPACE IN FRONT OF PANEL 4'-0" DEEP AND 6'-6" HIGH. DO NOT ROUTE DUCTS AND PIPES IN ELECTRICAL ROOMS, EXCEPT DUCTS AND PIPES SERVING THE ROOM OR WITHIN APPROPRIATE ENCLOSURE.
5. COORDINATE EXACT LOCATIONS OF CEILING DIFFUSERS AND GRILLES WITH ARCHITECTURAL REFLECTED CEILING PLANS.
6. ALL DUCT DIMENSIONS ARE INSIDE FREE AREA DIMENSIONS. ADJUST SHEET METAL DIMENSION FOR LINED DUCT.
7. ALL FIRE DAMPERS SHOWN ARE 1-1/2 HOUR UNLESS OTHERWISE NOTED.
8. IF CONTRACTOR ENCOUNTERS MATERIAL WHICH MAY CONTAIN ASBESTOS IMMEDIATELY STOP WORK IN THIS AREA AND NOTIFY THE OWNER.
9. PROVIDE CEILING ACCESS PANELS AS REQUIRED WHERE MECHANICAL EQUIPMENT, VALVES, HEAT PUMPS, FIRE DAMPERS, ETC. ARE LOCATED ABOVE INACCESSIBLE CEILINGS. ACCESS PANELS TO BE LISTED AND FIRE RATED EQUAL TO OR GREATER THAN THE RATING OF THE ASSEMBLY THEY ARE INSTALLED IN.
10. ALL DUCT AND FLUE PENETRATIONS THRU 1 HOUR ROOF ASSEMBLY TO BE ENCLOSED WITH 2 SHEET ROCK LAYERS FROM SHEET ROCK AT BOTTOM OF ROOF TRUSSES TO ROOF DECK.
11. STEEL ROOF DECK SHALL NOT BE USED TO SUPPORT LOADS FROM PIPING, DUCTWORK OR EQUIPMENT, UNLESS NOTED OTHERWISE. HANGER LOADS LESS THAN 50 LBS. MAY BE HUNG FROM THE STEEL ROOF DECK IN CASES WHEN HANGING FROM THE STEEL ROOF DECK CANNOT BE AVOIDED; THE ATTACHMENT METHOD MUST DISTRIBUTE THE LOAD ACROSS THE DECK AS APPROVED BY THE STRUCTURAL ENGINEER.
12. THE EQUIPMENT INSTALLER IS TO APPLY AND SIGN A CERTIFICATION LABEL TO EACH GAS-FIRED APPLIANCE, STATING THE APPLIANCE HAS BEEN ADJUSTED OR MODIFIED PER MANUFACTURER'S REQUIREMENTS FOR OPERATION AT THE PROJECT ALTITUDE AND WITH THE BTU-CONTENT OF THE AVAILABLE FUEL-GAS.

SYMBOL LEGEND

SYMBOL	DESCRIPTION
REFERENCE AND LINE SYMBOLS	
# / SHEET	DETAIL INDICATOR: # INDICATES DETAIL NUMBER, SHEET INDICATES DRAWING SHEET WHERE DETAIL IS SHOWN.
# / SHEET	ELEVATION OR SECTION INDICATOR, EXTERIOR: # INDICATES ELEVATION OR SECTION NUMBER, SHEET INDICATES DRAWING SHEET WHERE ELEVATION OR SECTION IS SHOWN.
# / SHEET	ELEVATION OR SECTION INDICATOR, INTERIOR: # INDICATES ELEVATION OR SECTION NUMBER, SHEET INDICATES DRAWING SHEET WHERE ELEVATION OR SECTION IS SHOWN.
TYPE / CFM / SIZE	DIFFUSER/GRILLE INDICATOR.
TYPE / SIZE	DIFFUSER/GRILLE INDICATOR.
	NEW CONNECTION POINT TO EXISTING

MECH/PLUMB SHEET INDEX

MECHANICAL SCOPE OF WORK

CONSTRUCTION NOTES:

BALANCE TO AIR FLOWS INDICATED ON MECHANICAL PLAN.

TERMINAL SUPPLY AND RETURN GRILLES ARE TO INCORPORATE INTEGRAL BALANCING DAMPERS.

COORDINATE INSTALLATION OF NEW EQUIPMENT AND ACCESSORIES WITH OTHER TRADES.

SUPPORT DUCTWORK, GRILLES AND DIFFUSERS FROM STRUCTURE.

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SEAL:

ISSUANCES:

	PERMIT	06.26.2026
1	ADDENDUM-01	08.27.2026

PROJECT:

NUMBER: 26002

DATE: 08.27.2026

SHEET:

MECHANICAL GENERAL NOTES & LEGEND

M001

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