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7. Backing removed, and weld tabs removed (if required)	PERFORM	PERFORM	AWC 360-16: Table C.N5-4.3	welded, visually inspect the weld access hole for cracks.
8. Repair activities	PERFORM	PERFORM	AWC 360-16: Table C.N5-4.3	
9. Document acceptance or rejection of welded joint or member	PERFORM	PERFORM	AWC 360-16: Table C.N5-4.3	
10. No prohibited welds have been added without the approval of the EOR	OBSERVE	OBSERVE	AWC 360-16: Table C.N5-4.3	
END SECTION				
STEEL - NON DESTRUCTIVE TESTING SECTION				
NONDESTRUCTIVE TESTING OF WELDED JOINTS (REFERENCE 2021 IBC 1705.2.1 AND AISC 360-16: SECTION N5.5)				
TASK	INSPECTION TYPE	REFERENCE STANDARD	DESCRIPTION	
1. Use of qualified nondestructive testing personnel	PERFORM	AWC 360-16: Section N5.5	Visual weld inspection and nondestructive testing (NDT) shall be conducted by personnel qualified in accordance with AWS D1.	
2. CJP groove welds	PERFORM	AWC 360-16: Section N5.5	Ultrasonic testing (UT) shall be performed on 100% of CJP groove welds for materials greater than 5/16" (8mm) thick.	
3. Welded joints subject to fatigue	PERFORM	AWC 360-16: Section N5.5	Ultrasonic testing (UT) shall be performed on 100% of welded joints identified on contract drawings as being subject to fatigue.	
END SECTION				
STRUCTURAL - STEEL - BOLTING SECTION				
STEEL INSPECTION TASKS PRIOR TO BOLTING (REFERENCE 2021 IBC 1705.2.1 AND AISC 360-16: Table N5.6-1)				
TASK	QC	QA	REFERENCE STANDARD	DESCRIPTION
1. Manufacturer's certifications available for fastener materials	OBSERVE	PERFORM	AWC 360-16: Table N5.6-1	
2. Fasteners marked in accordance with ASTM requirements	OBSERVE	OBSERVE	AWC 360-16: Table N5.6-1	
3. Correct fasteners selected for joint detail (grade, type, bolt length if threads are to be excluded from shear plane)	OBSERVE	OBSERVE	AWC 360-16: Table N5.6-1	
4. Correct bolting procedure selected for joint detail	OBSERVE	OBSERVE	AWC 360-16: Table N5.6-1	
5. Connecting elements, including appropriate faying surface condition and hole preparation, if specified, meet applicable requirements	OBSERVE	OBSERVE	AWC 360-16: Table N5.6-1	
6. Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used	PERFORM	OBSERVE	AWC 360-16: Table N5.6-1	
7. Protected storage provided for bolts, nuts, washers, and other fastener components	OBSERVE	OBSERVE	AWC 360-16: Table N5.6-1	
STEEL INSPECTION TASKS DURING BOLTING (REFERENCE 2021 IBC 1705.2.1 AND AISC 360-16: Table N5.6-2)				
TASK	QC	QA	REFERENCE STANDARD	DESCRIPTION
1. Fastener assemblies placed in all holes and washers and nuts are positioned as required	OBSERVE	PERFORM	AWC 360-16: Table N5.6-2	
2. Joint brought to the snug-tight condition prior to the pretensioning operation	OBSERVE	OBSERVE	AWC 360-16: Table N5.6-2	
3. Fastener component not turned by the wrench prevented from rotating	OBSERVE	OBSERVE	AWC 360-16: Table N5.6-2	
4. Fasteners are pretensioned in accordance with the RCSC Specification, progressing systematically from the most rigid joint toward the free edges	OBSERVE	OBSERVE	AWC 360-16: Table N5.6-2	
STEEL INSPECTION TASKS AFTER BOLTING (REFERENCE 2021 IBC 1705.2.1 AND AISC 360-16: Table N5.6-3)				
TASK	QC	QA	REFERENCE STANDARD	DESCRIPTION
1. Document acceptance or rejection of bolted connections	PERFORM	PERFORM	AWC 360-16: Table N5.6-3	
END SECTION				
STRUCTURAL - STEEL - OTHER INSPECTIONS				
OTHER STEEL INSPECTIONS - VERIFY THE FOLLOWING ARE IN COMPLIANCE (REFERENCE 2021 IBC 1705.2.1)				
TASK	INSPECTION TYPE	REFERENCE STANDARD	DESCRIPTION	
1. Anchor rods and other embedments supporting structural steel	PERFORM	ACI 318: 17.8.2 and AISC 360-16 Section N.8	Verify the diameter, grade, type, and length of the anchor rod or embedment, and the spacing and depth of embedment prior to placement of concrete.	
2. Fabricated steel or erected steel frame	OBSERVE	AWC 360-16 Section N.8	Verify compliance with the details shown on the construction documents, such as bracing, stiffeners, member locations and proper application of joint details at each connection.	
END SECTION				
STRUCTURAL - STEEL - ADDITIONAL INSPECTION REQUIREMENTS FOR SEISMIC FORCE RESISTING SYSTEMS *				
STEEL INSPECTION PRIOR TO WELDING (REFERENCE 2021 IBC 1705.13.1, AISC 341-16: Section J6, Table J6.1)				
TASK	QC	QA	REFERENCE STANDARD	DESCRIPTION
1. Fit-up of groove welds (including joint geometry)	PERFORM/OBSERVE**	OBSERVE	AWC 341-16: Table J6.1	- Joint preparation - Dimensions (alignment, root opening, root face, bevel) - Cleanliness (condition of steel surfaces) - Tacking (tack weld quality and location) - Backing type and fit (if applicable)
2. Fit-up of fillet welds	PERFORM/OBSERVE**	OBSERVE	AWC 341-16: Table J6.1	
3. Dimensions (alignment, gaps at root)				
4. Cleanliness (condition of steel surfaces)				
STEEL INSPECTION DURING WELDING (REFERENCE 2021 IBC 1705.13.1, AISC 341-16: Section J6, Table J6.2)				
TASK	QC	QA	REFERENCE STANDARD	DESCRIPTION
1. Welding Procedures Specification followed	OBSERVE	OBSERVE	AWC 341-16: Table J6.2	Intermix of filler metals avoided unless approved
2. Use of qualified welders	OBSERVE	OBSERVE	AWC 341-16: Table J6.2	
3. No welding over cracked tacks	OBSERVE	OBSERVE	AWC 341-16: Table J6.2	
STEEL INSPECTION AFTER WELDING (REFERENCE 2021 IBC 1705.13.1, AISC 341-16: Section J6, Table J6.3)				
TASK	QC	QA	REFERENCE STANDARD	DESCRIPTION
1. Placement of reinforcing or contouring fillet welds (if required)	PERFORM	PERFORM	AWC 341-16: Table J6.3	
2. Backing removed, weld tabs removed and finished, and fillet welds added (if required)	PERFORM	PERFORM	AWC 341-16: Table J6.3	
OTHER INSPECTION TASKS (REFERENCE 2021 IBC 1705.13.1, AISC 341-16: Section J6, Table J6.1 & J6.10.1)				
TASK	QC	QA	REFERENCE STANDARD	DESCRIPTION
1. RBS requirements, if applicable	PERFORM	PERFORM	AWC 341-16: Table J6.1	Dimensional tolerances
2. Protected zones - no holes and unapproved attachments made by fabricator or erector	PERFORM	PERFORM	AWC 341-16: Table J6.1	
3. H-plies - Protected Zones	PERFORM	PERFORM	AWC 341-16: Table J6.1	
No holes and unapproved attachments made by the responsible contractor, as applicable.				
* The additional requirements of AWS/D1.8/D1.8M shall apply to all inspections and testing. ** Following performance of this inspection task for ten welds to be made by a given welder, with the welder demonstrating understanding of requirements and possession of skills and tools to verify these items, the Perform designation of this task shall be reduced to Observe, and the welder shall perform this task. Should the inspector determine that the welder has discontinued performance of this task, the task shall be returned to Perform until such time as the inspector has re-established adequate assurance that the welder will perform the inspection tasks listed.				
END SECTION				
STRUCTURAL - STEEL - ADDITIONAL TESTING REQUIREMENTS FOR SEISMIC FORCE RESISTING SYSTEMS *				
NONDESTRUCTIVE TESTING OF WELDED JOINTS - VERIFY THE FOLLOWING ARE IN COMPLIANCE (REFERENCE 2021 IBC 1705.13.1, AISC 341-16: Section J6.2)				
TASK	INSPECTION TYPE	REFERENCE STANDARD	DESCRIPTION	
1. CJP groove welds	PERFORM	AWC 341-16: Section J6.2	Ultrasonic testing (UT) shall be performed on 100% of CJP groove welds for materials greater than 5/16" thick (8mm). Magnetic particle testing (MT) shall be performed on 25% of all beam-to-column CJP groove welds.	
2. Column splice and column to baseplate PJP groove welds	PERFORM	AWC 341-16: Section J6.2	UT shall be performed by QA on 100% of partial-joint penetration PJP groove welds in column splices and column to base plate welds.	
3. Base metal for lamellar tearing and laminations	PERFORM	AWC 341-16: Section J6.2	After joint completion, base metal thicker than 1 1/2 in. (38 mm) loaded in tension in the through-thickness direction in T- and corner-joints, where the connected material is greater than 3/4 in. (19 mm) and contains CJP groove welds, shall be ultrasonically tested for discontinuities behind and adjacent to the fusion line of such welds. Any base metal discontinuities found within 1/4 of the steel surface shall be accepted or rejected on the basis of criteria of AWS D1.1/D1.1M Table 6.2, where t is the thickness of the part subjected to the through-thickness strain.	
4. Beam cope and access holes.	PERFORM	AWC 341-16: Section J6.2	At welded splices and connections, thermally cut surfaces of beam copes and access holes shall be tested using MT or dye penetrant testing (DT), when the flange thickness exceeds 1 1/2 in. for rolled shapes, or when the web thickness exceeds 1 1/2 in. for built-up shapes.	
5. Reduced beam section repair	PERFORM	AWC 341-16: Section J6.2	MT shall be performed on any weld and adjacent area of the reduced beam section (RBS) cut surface that has been repaired by welding, or on the base metal of the RBS cut surface if a sharp notch has been removed by grinding.	
6. Weld tab removal sites	PERFORM	AWC 341-16: Section J6.2	At the end of welds where weld tabs have been removed, MT shall be performed on the same beam-to-column joints receiving UT as required by item 1 above.	
7. K-area	PERFORM	AWC 341-16: Section J6.2	Where welding of doubler plates, continuity plates or stiffeners has been performed in the k-area, the web shall be tested for cracks using MT. The MT inspection area shall include the k-area base metal within 3-inches of the weld. The MT shall be performed no sooner than 48 hours following completion of the welding.	
* The additional requirements of AWS/D1.8/D1.8M shall apply to all inspections and testing.				
END SECTION				

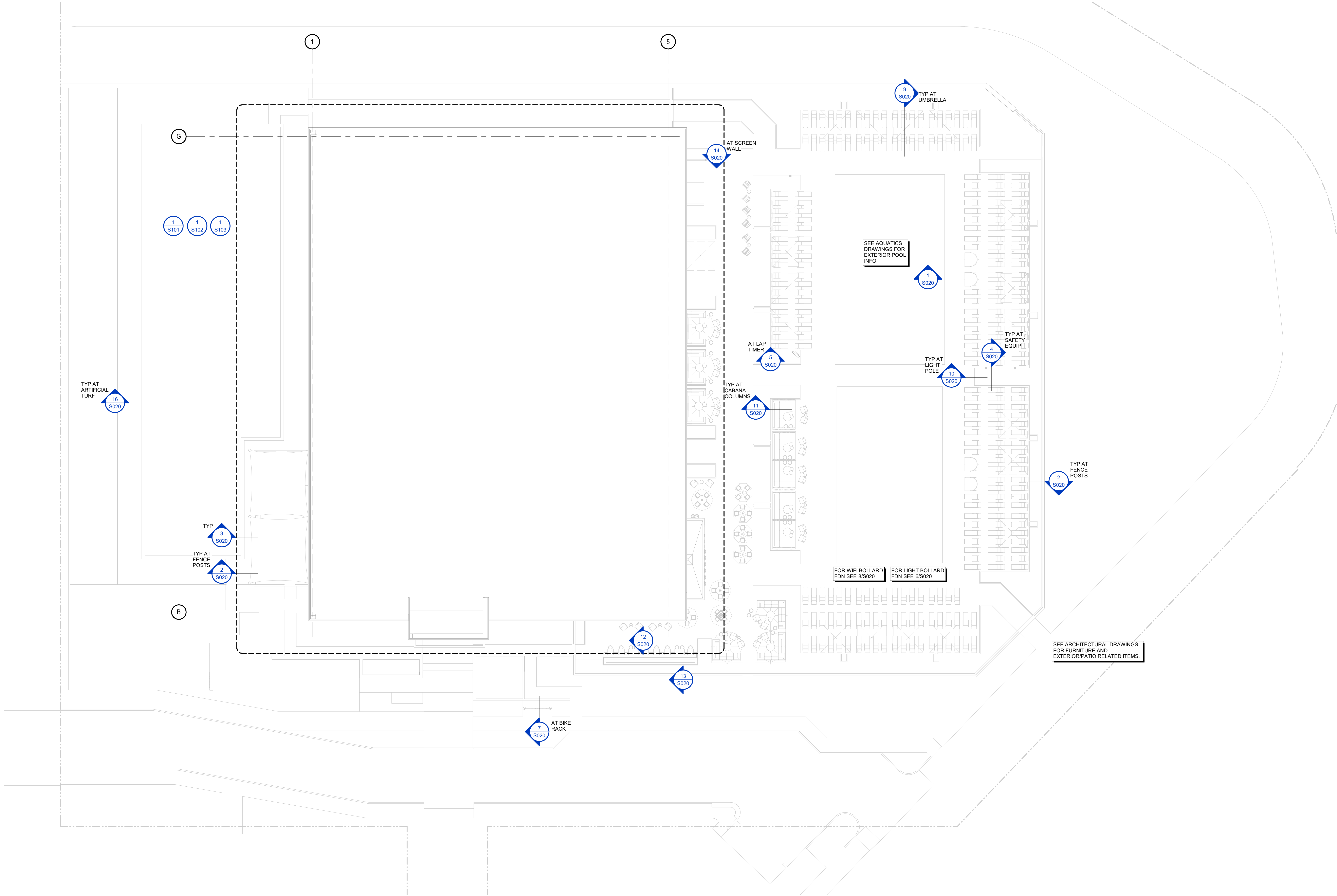
STRUCTURAL - STEEL - ADDITIONAL INSPECTION REQUIREMENTS FOR COMPOSITE STRUCTURES (Also reference Concrete Construction Section for all concrete-related inspections and testing of composite steel construction.)					
COMPOSITE CONSTRUCTION PRIOR TO PLACING CONCRETE – VERIFY THE FOLLOWING ARE IN COMPLIANCE 2021 IBC 1705.13.1 / AISC 341-16: Section J9					
TASK	QC	QA	REFERENCE STANDARD	DESCRIPTION	
1. Placement and installation of steel headed stud anchors	OBSERVE	OBSERVE	ASTM A31-16: Table J9.1		
2. Material identification of reinforcing steel (Type/Grade)	OBSERVE	OBSERVE	AISC 341-16: Table J9.1		
3. Determination of carbon equivalent for reinforcing steel other than ASTM A706	OBSERVE	OBSERVE	AISC 341-16: Table J9.1		
4. Proper reinforcing steel size, spacing, clearances, support, and orientation	OBSERVE	OBSERVE	AISC 341-16: Table J9.1		
5. Reinforcing steel has not been re-bent in the field	OBSERVE	OBSERVE	AISC 341-16: Table J9.1		
6. Reinforcing steel has been bent and supported as required	OBSERVE	OBSERVE	AISC 341-16: Table J9.1		
7. Required reinforcing steel clearances have been provided	OBSERVE	OBSERVE	AISC 341-16: Table J9.1		
8. Composite member has required size	OBSERVE	OBSERVE	AISC 341-16: Table J9.1		
END SECTION					
STRUCTURAL - OPEN-WEB STEEL JOISTS SECTION					
OPEN-WEB STEEL JOISTS AND JOIST GIRDERS					
TASK	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCE STANDARD	DESCRIPTION	
1. Installation of open-web steel joists and joist girders					
a. End connections – welded or bolted		X	SJI C/J or SJI 100, as applicable		
b. Bridging – horizontal and diagonal					
1. Standard bridging		X	SJI C/J or SJI 100, as applicable		
2. Bridging that differs from the SJI specification listed in Section 2207.1.		X			
END SECTION					
STRUCTURAL - COLD-FORMED METAL DECK - PLACEMENT SECTION					
METAL DECK INSPECTION PRIOR TO DECK PLACEMENT – VERIFY THE FOLLOWING ARE IN COMPLIANCE					
TASK	PERFORM	OBSERVE	REFERENCE STANDARD	IBC REFERENCE	DESCRIPTION
1. Verify compliance of materials (deck and all deck accessories) with construction documents, including profiles, material properties, and base metal thickness	X		SDI O/AOC-2017, Appendix 1, Table 1.1	1705.2.2	
2. Document acceptance or rejection of deck and deck accessories	X		SDI O/AOC-2017, Appendix 1, Table 1.1	1705.2.2	
METAL DECK INSPECTION DURING DECK PLACEMENT – VERIFY THE FOLLOWING ARE IN COMPLIANCE					
TASK	PERFORM	OBSERVE	REFERENCE STANDARD	IBC REFERENCE	DESCRIPTION
1. Verify compliance of deck and all deck accessories installation with construction documents	X		SDI O/AOC-2017, Appendix 1, Table 1.2	1705.2.2	
2. Verify deck materials are represented by the mill certifications that comply with the construction documents	X		SDI O/AOC-2017, Appendix 1, Table 1.2	1705.2.2	
3. Document acceptance or rejection of installation of deck and deck accessories	X		SDI O/AOC-2017, Appendix 1, Table 1.2	1705.2.2	
METAL DECK INSPECTION AFTER DECK PLACEMENT – VERIFY THE FOLLOWING ARE IN COMPLIANCE					
TASK	PERFORM	OBSERVE	REFERENCE STANDARD	IBC REFERENCE	DESCRIPTION
1. Welding procedure specification (WPS) available	X		SDI O/AOC-2017, Appendix 1, Table 1.3	1705.2.2	
2. Manufacturer certifications for welding consumables available	X		SDI O/AOC-2017, Appendix 1, Table 1.3	1705.2.2	
3. Material identification (type/grade)	X		SDI O/AOC-2017, Appendix 1, Table 1.3	1705.2.2	
4. Check welding equipment	X		SDI O/AOC-2017, Appendix 1, Table 1.3	1705.2.2	
END SECTION					
STRUCTURAL - COLD-FORMED METAL DECK – WELDING SECTION					
METAL DECK INSPECTION OR EXECUTION PRIOR TO WELDING					
TASK	PERFORM	OBSERVE	REFERENCE STANDARD	IBC REFERENCE	DESCRIPTION
1. Weld procedure specifications (WPS) available		X	SDI O/AOC-2017, Appendix 1, Table 1.3	1705.2.2	
2. Manufacturer certifications for welding consumables available		X	SDI O/AOC-2017, Appendix 1, Table 1.3	1705.2.2	
3. Material identification (type/grade)		X	SDI O/AOC-2017, Appendix 1, Table 1.3	1705.2.2	
4. Check welding equipment		X	SDI O/AOC-2017, Appendix 1, Table 1.3	1705.2.2	
METAL DECK INSPECTION OR EXECUTION DURING WELDING					
TASK	PERFORM	OBSERVE	REFERENCE STANDARD	IBC REFERENCE	DESCRIPTION
1. Use of qualified welders		X	SDI O/AOC-2017, Appendix 1, Table 1.4	1705.2.2	
2. Control and handling of welding consumables		X	SDI O/AOC-2017, Appendix 1, Table 1.4	1705.2.2	
3. Environmental conditions (wind speed, moisture, temperature)		X	SDI O/AOC-2017, Appendix 1, Table 1.4	1705.2.2	
4. WPS followed		X	SDI O/AOC-2017, Appendix 1, Table 1.4	1705.2.2	
METAL DECK INSPECTION OR EXECUTION TASKS AFTER WELDING					
TASK	PERFORM	OBSERVE	REFERENCE STANDARD	IBC REFERENCE	DESCRIPTION
1. Verify size and location of welds, including support, sidelap, and perimeter welds		X	SDI O/AOC-2017, Appendix 1, Table 1.5	1705.2.2	
2. Welds meet visual acceptance criteria		X	SDI O/AOC-2017, Appendix 1, Table 1.5	1705.2.2	
3. Verify repair activities		X	SDI O/AOC-2017, Appendix 1, Table 1.5	1705.2.2	
4. Document acceptance or rejection of welds		X	SDI O/AOC-2017, Appendix 1, Table 1.5	1705.2.2	
END SECTION					
STRUCTURAL - COLD-FORMED METAL DECK – MECHANICAL FASTENING					
METAL DECK INSPECTION OR EXECUTION PRIOR TO MECHANICAL FASTENING					
TASK	PERFORM	OBSERVE	REFERENCE STANDARD	IBC REFERENCE	DESCRIPTION
1. Manufacturer installation instructions available for mechanical fasteners		X	SDI O/AOC-2017, Appendix 1, Table 1.6	1705.2.2	
2. Proper tools available for fastener installation		X	SDI O/AOC-2017, Appendix 1, Table 1.6	1705.2.2	
3. Proper storage for mechanical fasteners		X	SDI O/AOC-2017, Appendix 1, Table 1.6	1705.2.2	
METAL DECK INSPECTION OR EXECUTION DURING MECHANICAL FASTENING					
TASK	PERFORM	OBSERVE	REFERENCE STANDARD	IBC REFERENCE	DESCRIPTION
1. Fasteners are positioned as required		X	SDI O/AOC-2017, Appendix 1, Table 1.7	1705.2.2	
2. Fasteners are installed in accordance with manufacturer's instructions		X	SDI O/AOC-2017, Appendix 1, Table 1.7	1705.2.2	
METAL DECK INSPECTION OR EXECUTION TASKS AFTER MECHANICAL FASTENING					
TASK	PERFORM	OBSERVE	REFERENCE STANDARD	IBC REFERENCE	DESCRIPTION
1. Check spacing, type, and installation of support fasteners		X	SDI O/AOC-2017, Appendix 1, Table 1.8	1705.2.2	
2. Check spacing, type, and installation of sidelap fasteners		X	SDI O/AOC-2017, Appendix 1, Table 1.8	1705.2.2	
3. Check spacing, type, and installation of perimeter fasteners		X	SDI O/AOC-2017, Appendix 1, Table 1.8	1705.2.2	
4. Verify repair activities		X	SDI O/AOC-2017, Appendix 1, Table 1.8	1705.2.2	
5. Document acceptance or rejection of mechanical fasteners		X	SDI O/AOC-2017, Appendix 1, Table 1.8	1705.2.2	
END SECTION					
STRUCTURAL - CONCRETE CONSTRUCTION SECTION					
CONCRETE CONSTRUCTION, INCLUDING COMPOSITE DECK					
TASK	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCE STANDARD	NOTES	
1. Prior to placement, concrete mixture for intended location.	X		ACI 318 CH, 19, 26.4.3, 28.4.4, 26.13.3.2, IBC 1904.1-1904.2		
2. Prior to concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	X		ASTM C172, ASTM C31, ACI 318, 26.5, 26.12	See notes directly below for specific testing requirements.	
Strength test samples: sampling procedure: ASTM C172. Cylinder molding and curing procedure: ASTM C31/C31M. cylinder specimens, standard cured. Sample concrete and make one set of five 4-in. or four 6-in. cylinders for each day's pour between one and 25 cubic yards. If a day's pour exceeds 25 cubic yards, make five 4-in. or four 6-in. cylinders for each additional 50 cubic yards. When volume of concrete for any class of concrete would provide less than five sets of cylinders, take samples from five randomly selected batches, or from every batch when less than five batches are used. Make one additional cylinder during cold weather concreting, and field cure.					
Field Testing: Slump test method: ASTM C143/C143M. Air content test method: ASTM C173/C173M or ASTM C231. Temperature test method: ASTM C1064/C1064M. At the time fresh concrete is sampled, measure slump and temperature for each compressive					

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OVERALL SITE REFERENCE PLAN

1/16" = 1'-0"



LIFETIME
HEALTHY WAY OF LIFE
2900 CORPORATE PLACE CHANHASSEN, MN 55317

CONSULTANT

LIFETIME
PROPERTY DEVELOPMENT

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763.558.9100
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PROJECT INFORMATION

Holladay, UT

1975 East Rodeo Drive
Holladay, Utah

DRAWING LOG

REV	DATE	DESCRIPTION
1		
2		

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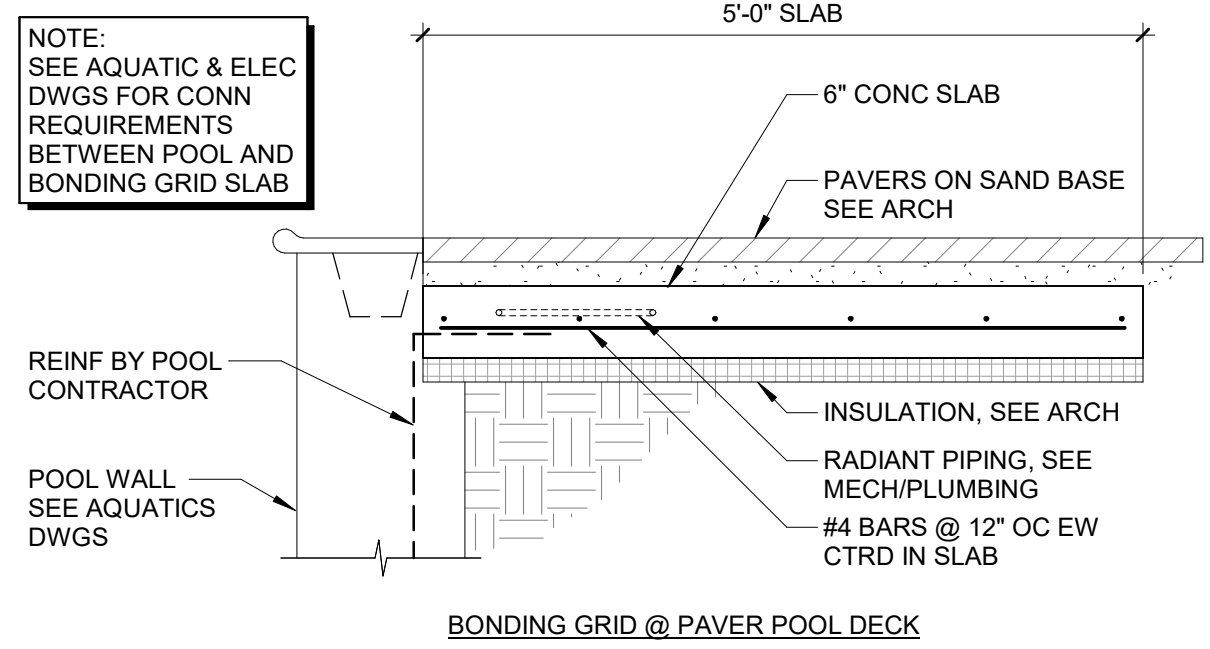
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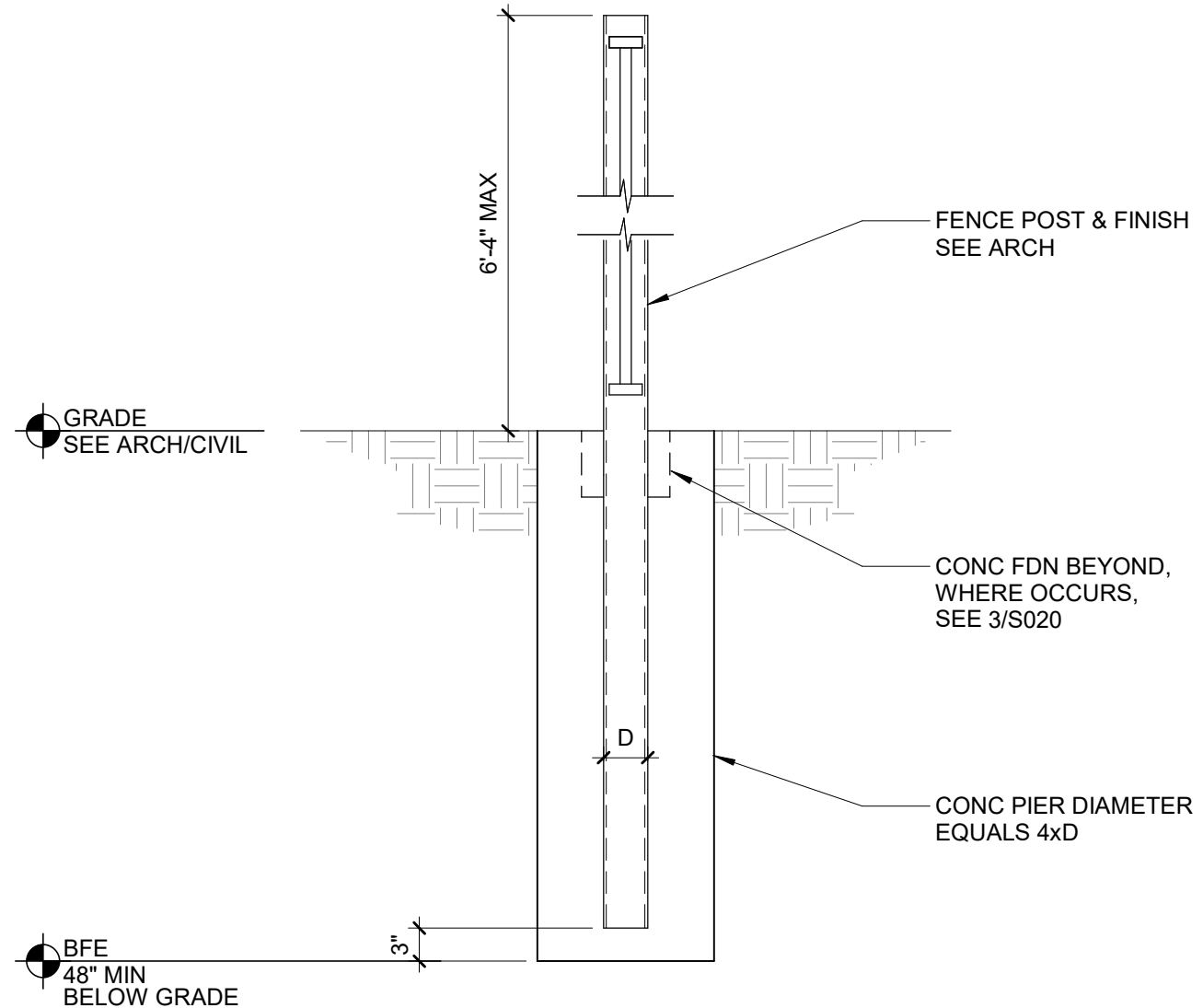
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OVERALL SITE REFERENCE PLAN

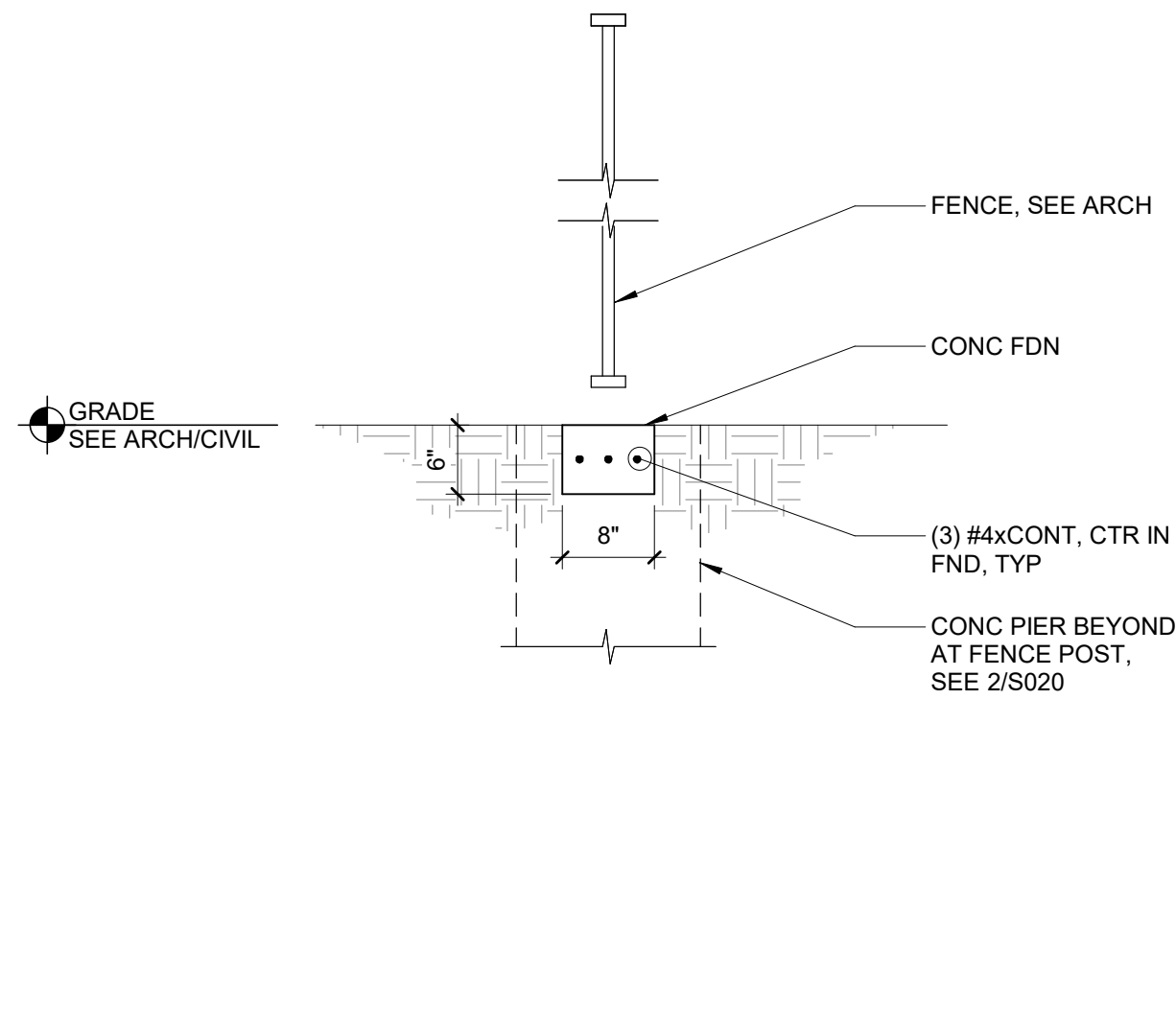
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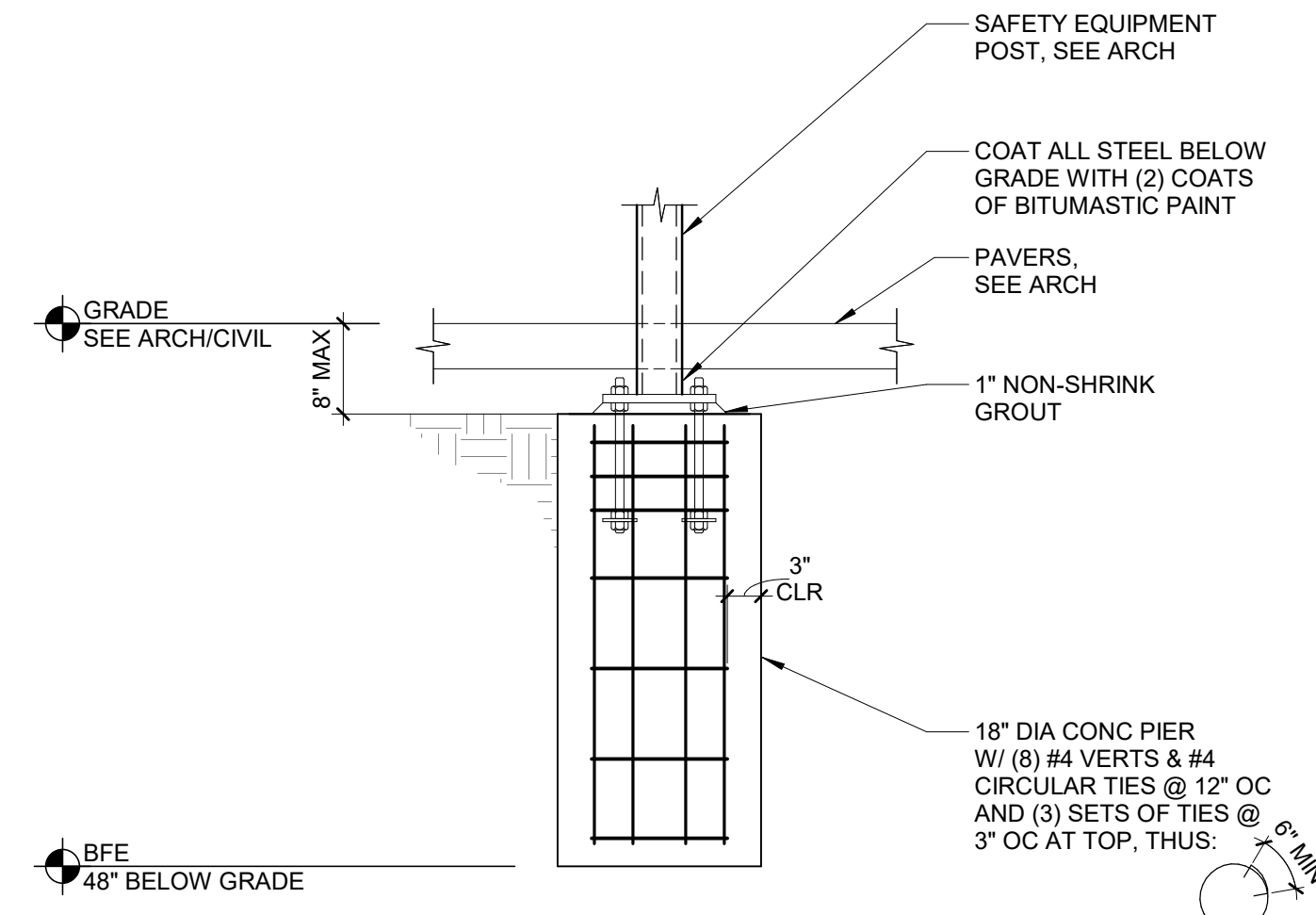
1 POOL/SPA EDGE DETAILS
3/4" = 1'-0"



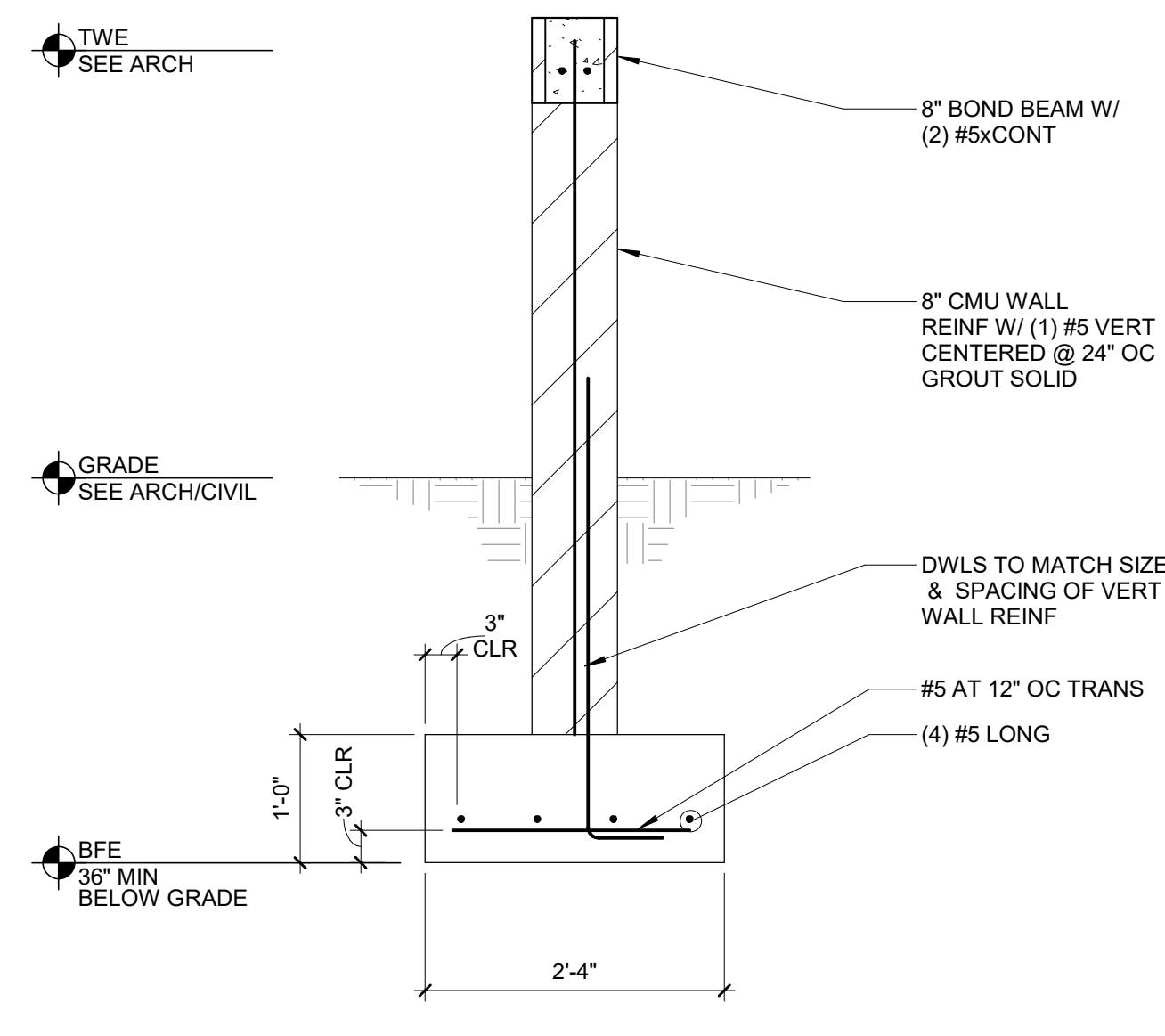
2 STEEL FENCE FDN
3/4" = 1'-0"



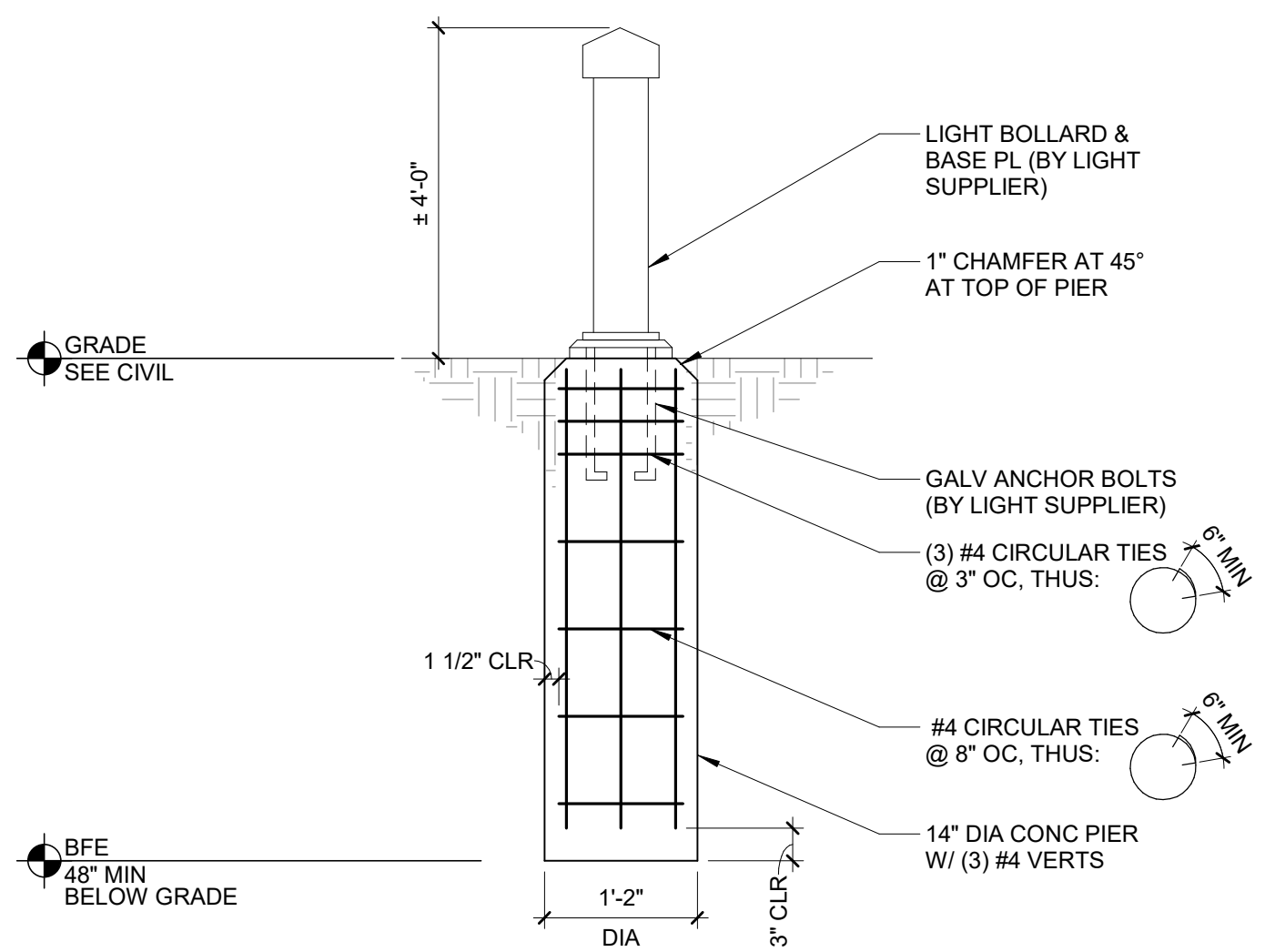
3 STEEL FENCE APRON
3/4" = 1'-0"



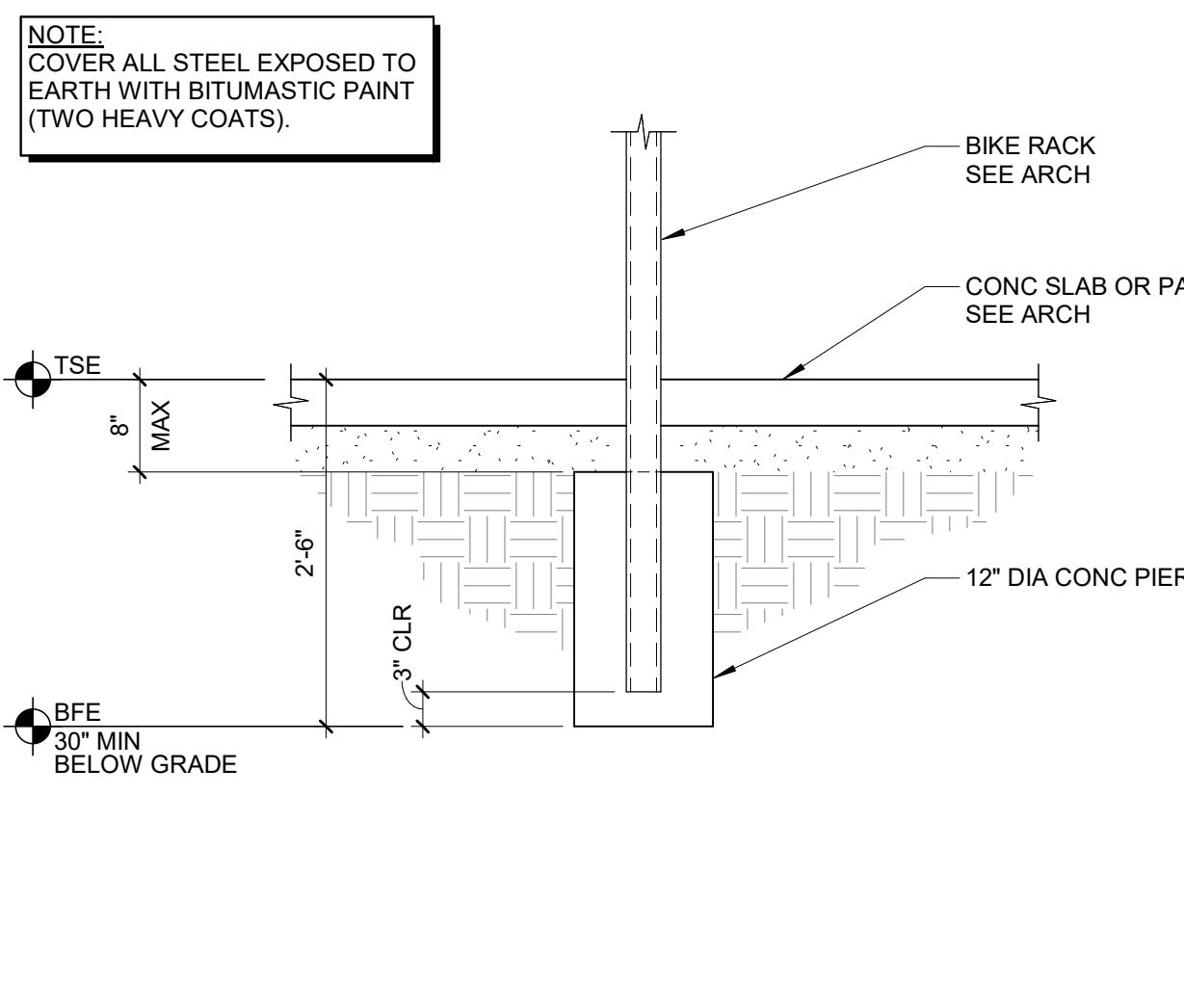
4 POOL SAFETY EQUIP FOUNDATION
3/4" = 1'-0"



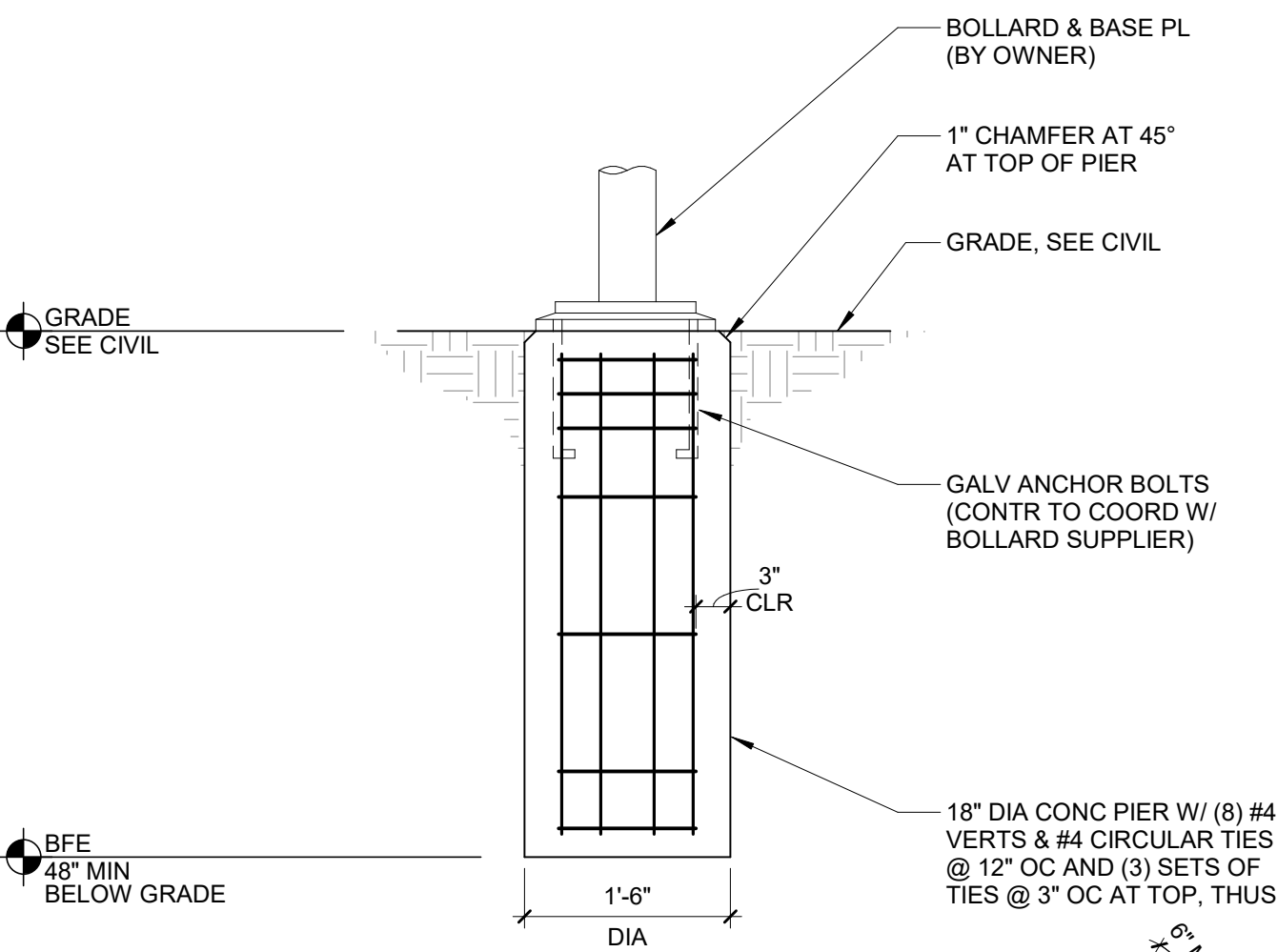
5 LAP TIMER PIER DETAIL
3/4" = 1'-0"



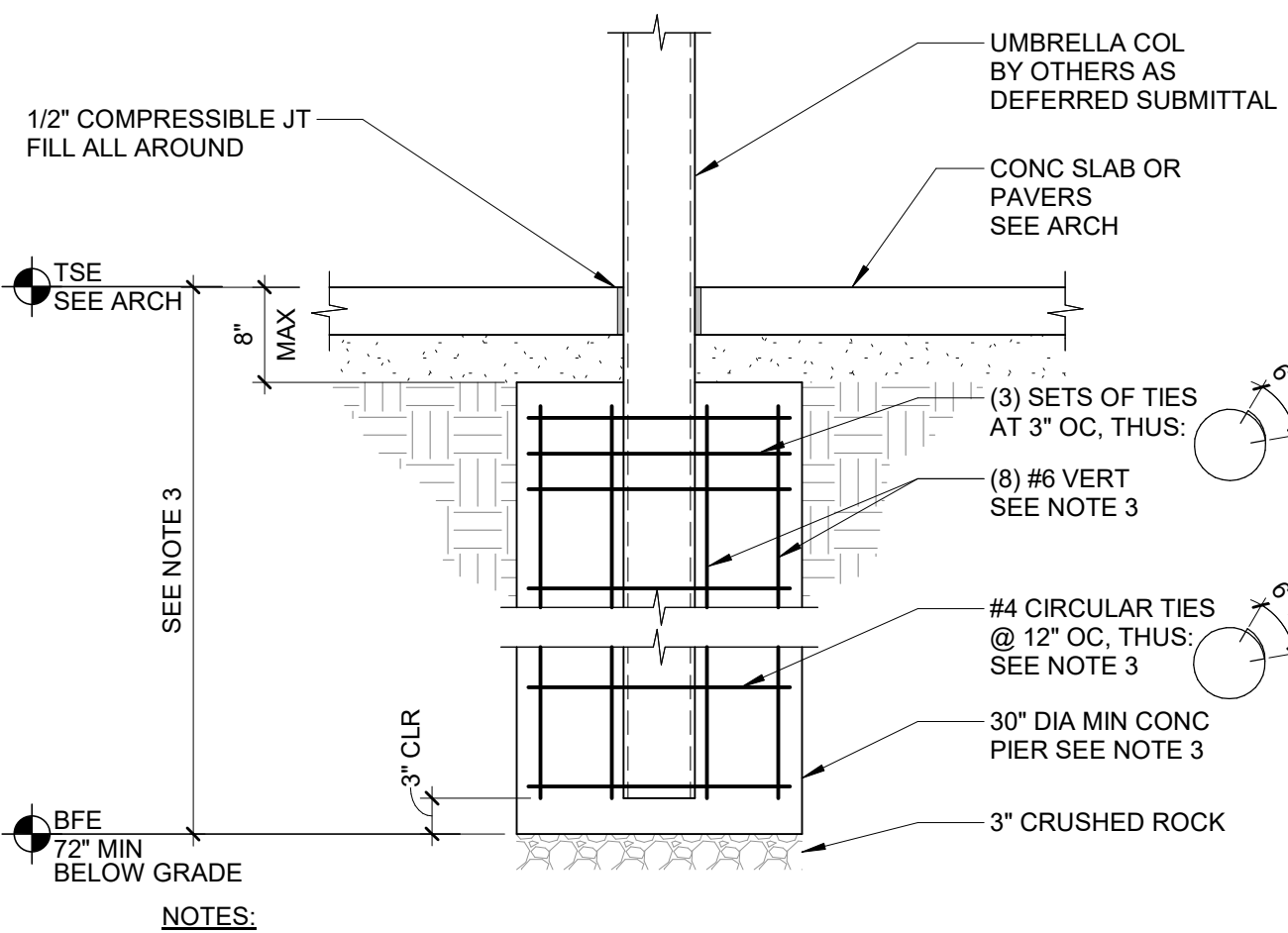
6 TYP LIGHT BOLLARD FDN AT EXT POOL
3/4" = 1'-0"



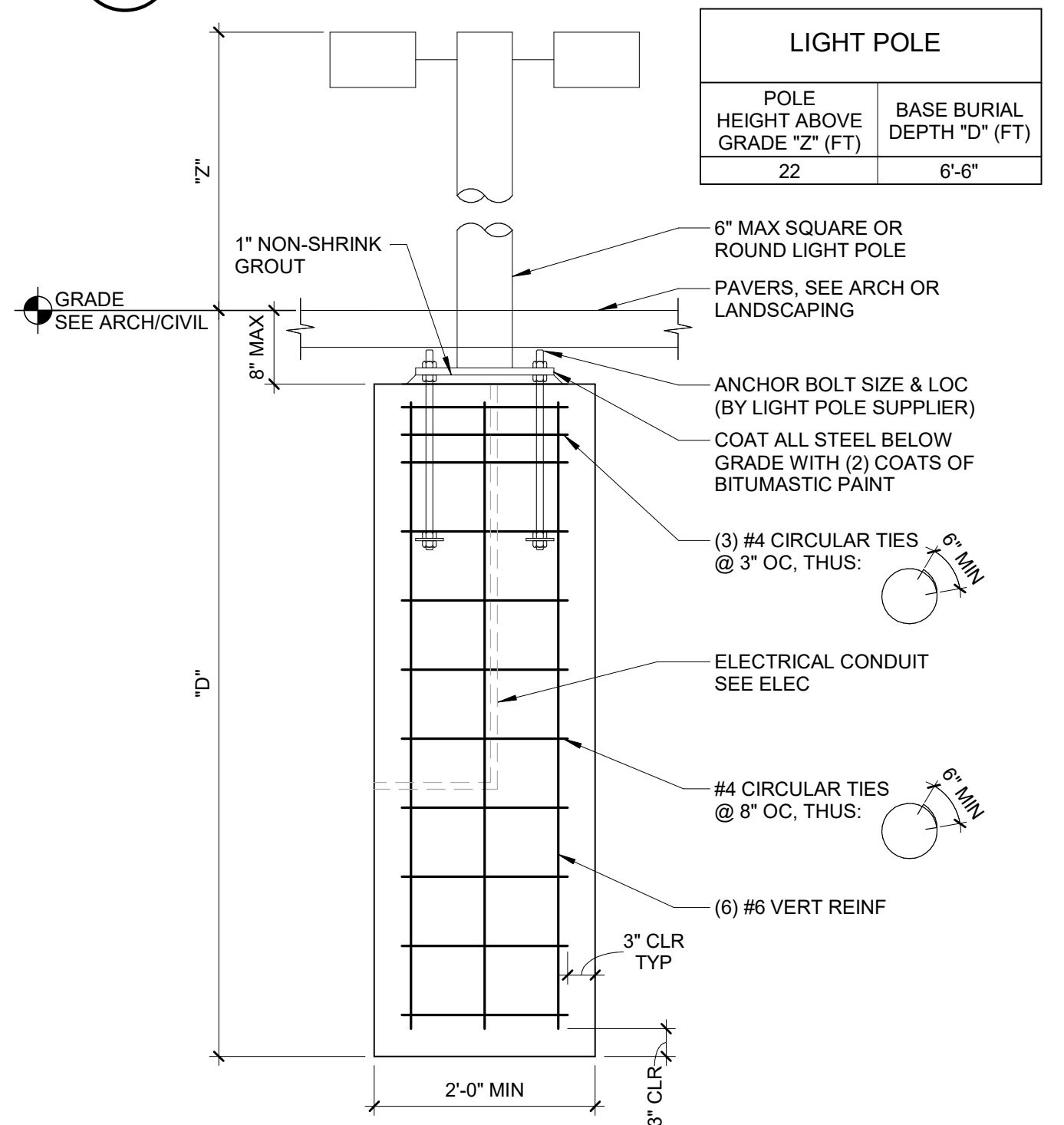
7 BIKE RACK FOUNDATION DETAIL
3/4" = 1'-0"



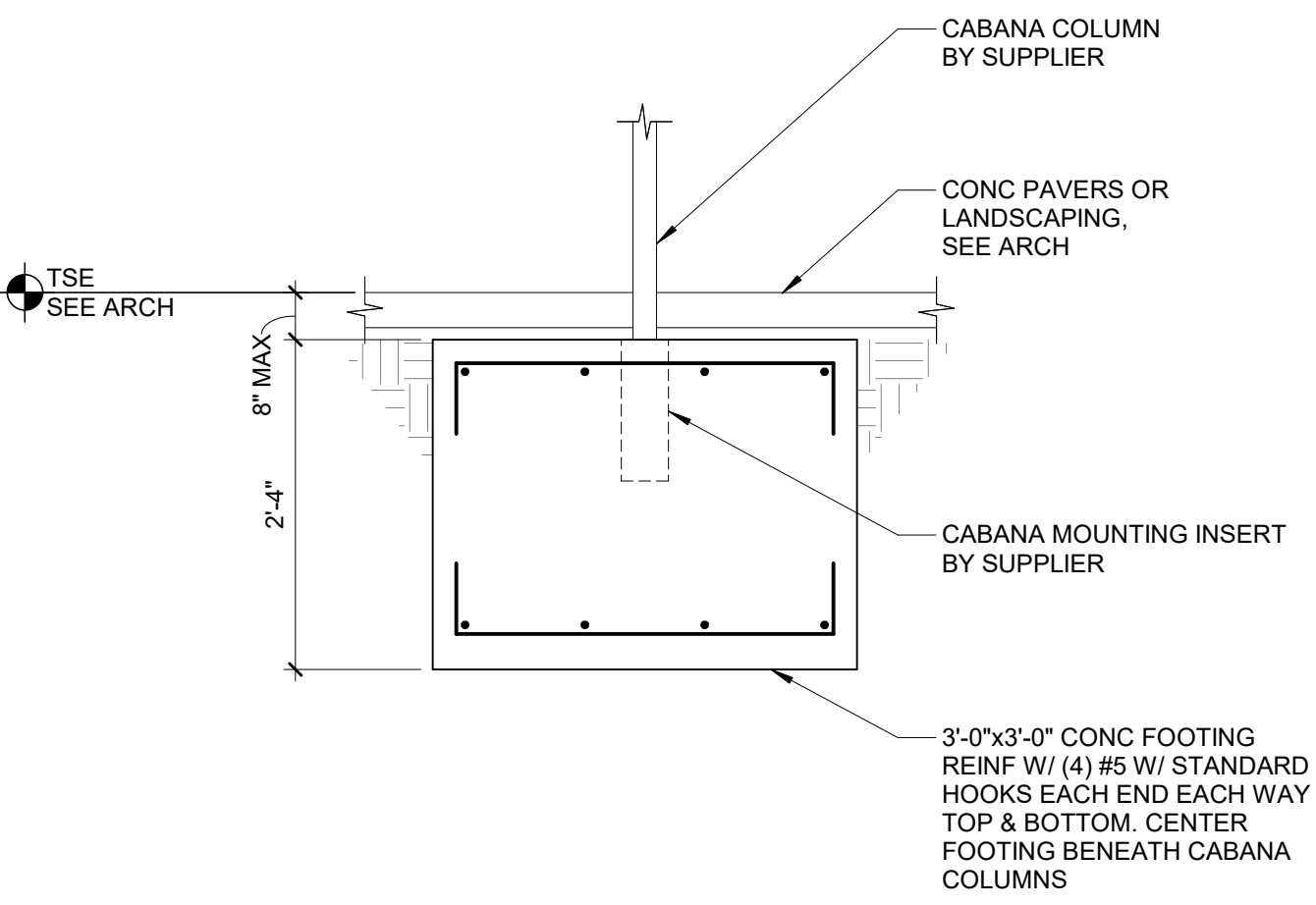
8 TYP WIFI BOLLARD FDN DETAIL
3/4" = 1'-0"



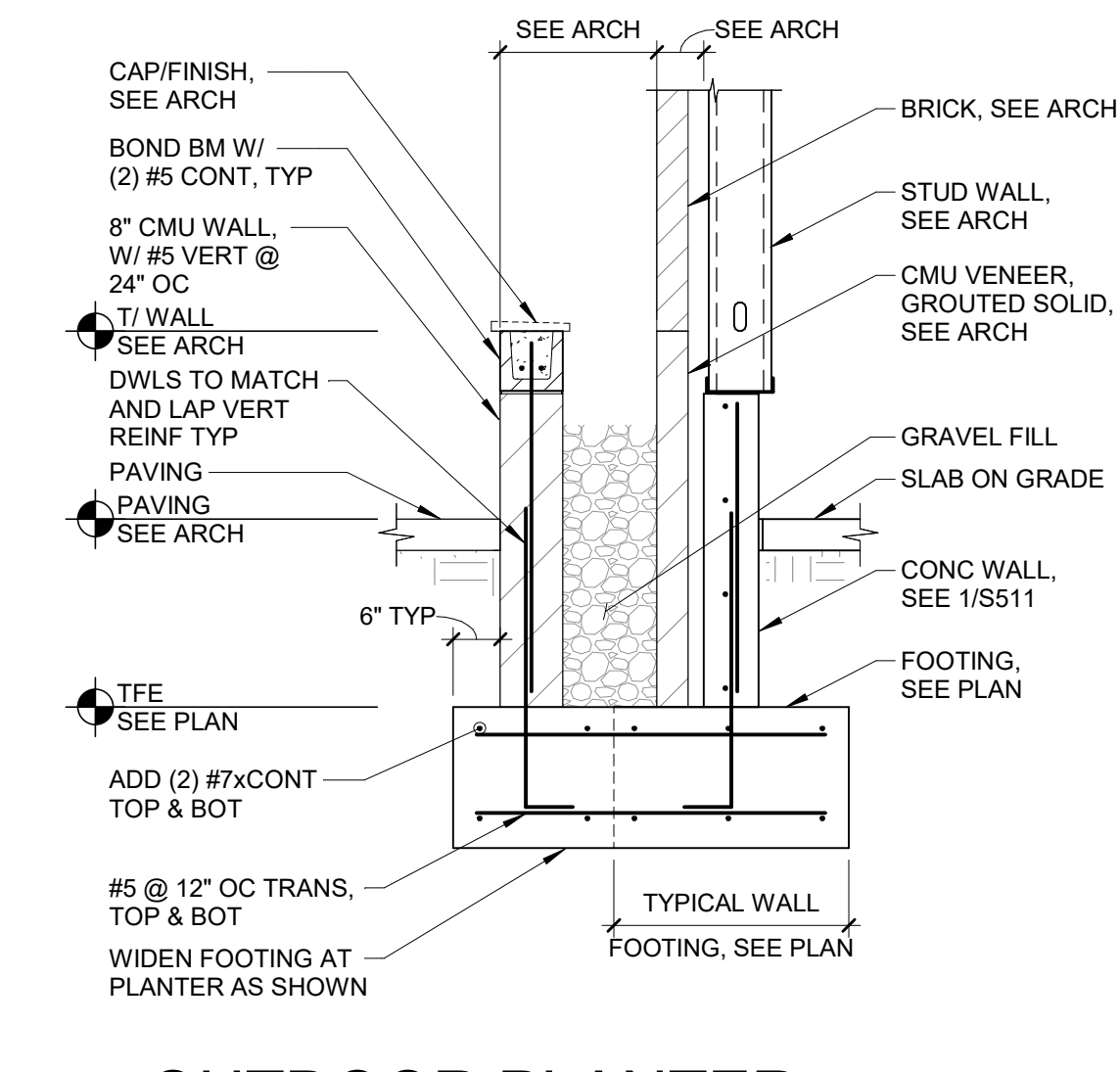
9 TYP UMBRELLA FOUNDATION DETAIL
3/4" = 1'-0"



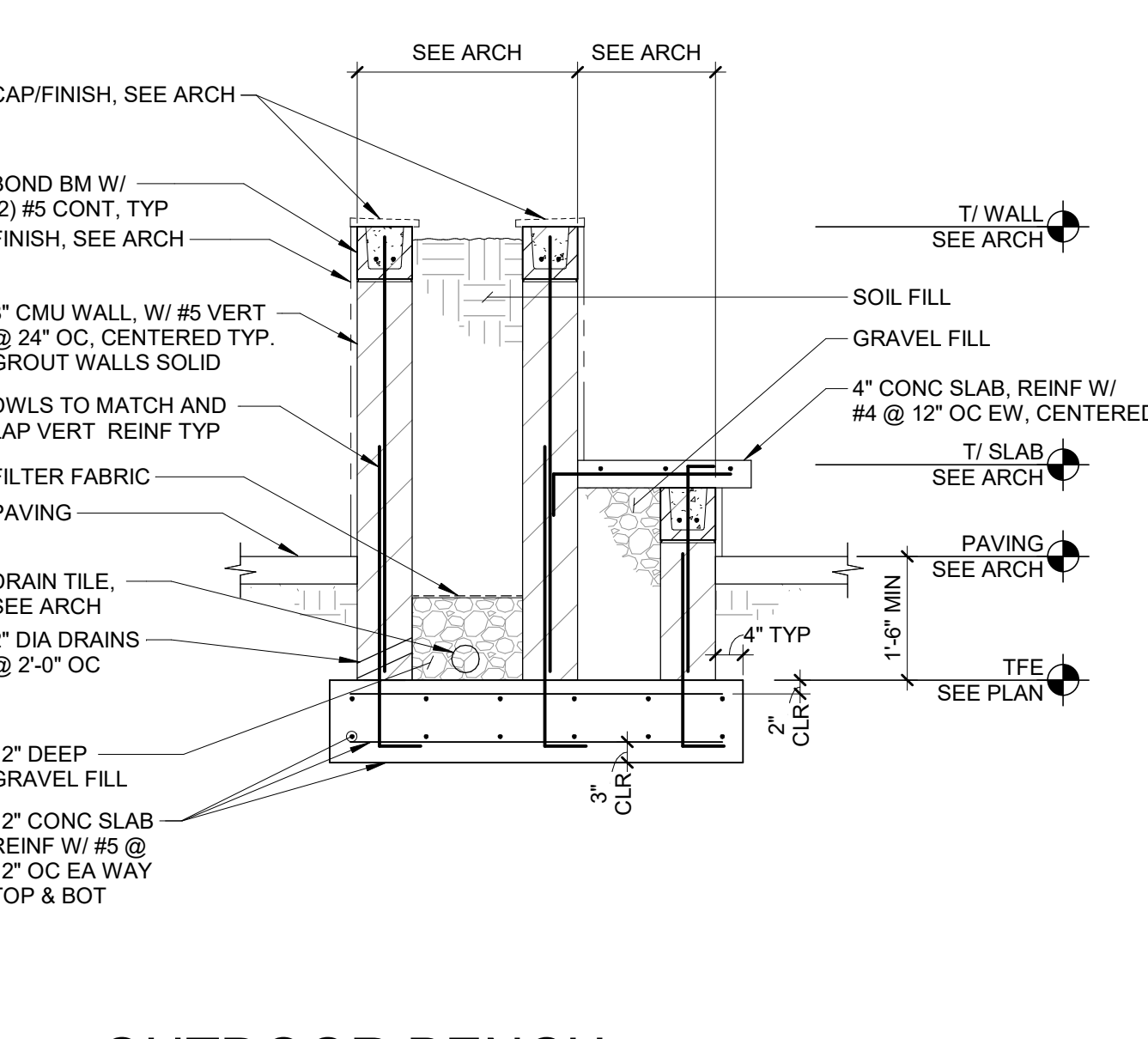
10 TYP POOL DECK LIGHT POLE FDN
3/4" = 1'-0"



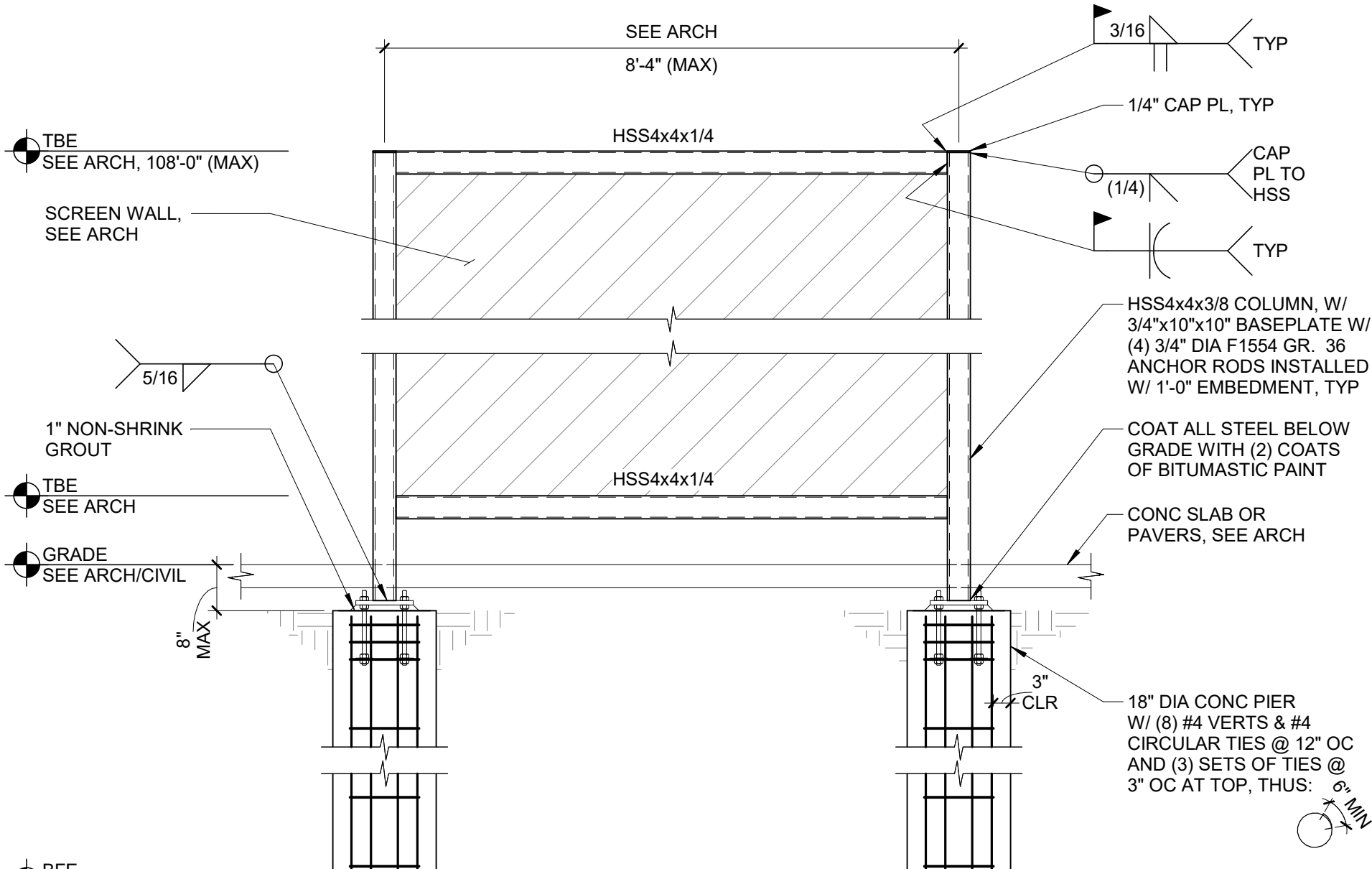
11 CABANA FOUNDATION DETAIL
3/4" = 1'-0"



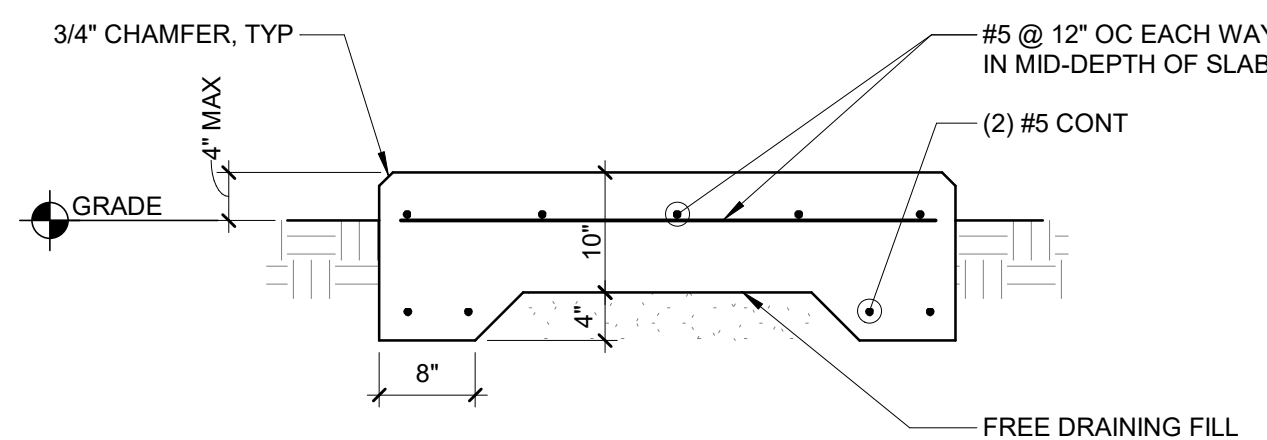
12 OUTDOOR PLANTER FOUNDATION SECTION
1/2" = 1'-0"



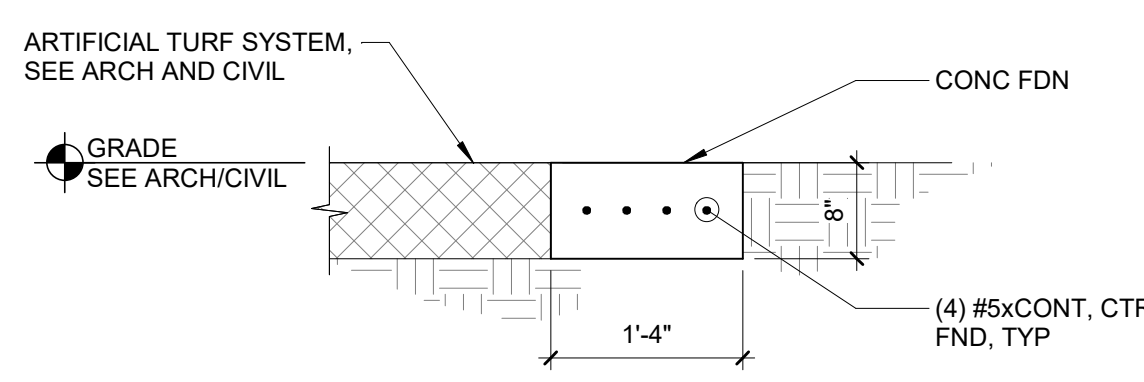
13 OUTDOOR BENCH FOUNDATION SECTION
1/2" = 1'-0"



14 SCREENWALL FOUNDATION
1/2" = 1'-0"



15 EXTERIOR EQUIPMENT SLAB
3/4" = 1'-0"



16 ARTIFICIAL TURF APRON
3/4" = 1'-0"



1. FOR GENERAL STRUCTURAL NOTES SEE SHEET S501.
2. FOR TYPICAL CONCRETE DETAILS SEE SHEET S501 & S502.
3. TOP OF REINFORCED CONCRETE SHALL BE AT INTERIOR FINISHINGS AND 99-0" TO TOP AT INTERIOR FINISHINGS.
4. TOP OF SLAB ELEVATION (TSE) = 100'-0" US. SEE CIVIL FOR USDS DATA ELEVATION.
5. PROVIDE 4" CONCRETE SLAB ON GRADE REINFORCED WITH 3.0 LBS PER CUBIC YARD OF MESH OR 1/2" DIA. WARPED BARS @ 18" ON CENTER PER THICK BAY PER GEOTECHNICAL REPORT.
6. "CX" ON PLAN DENOTES COLUMN TYPE. SEE SCHEDULE ON S501.
7. "FX" ON PLAN DENOTES FOOTING. SEE SCHEDULE ON S501.
8. "PX" ON PLAN DENOTES CONCRETE PIER. SEE SCHEDULE ON S501.
9. TOP OF PIER SHALL BE AT INTERIOR FINISHING.
10. "MX" ON PLAN DENOTES MAT FOUNDATION. SEE SCHEDULE ON S501.
11. "WY" ON PLAN DENOTES WALL FOOTING. SEE SCHEDULE ON S501.
12. "CY" ON PLAN DENOTES CONCRETE YARD. SEE SCHEDULE ON S501.
13. "S-" DENOTES STEPPED FOOTING. SEE TYPICAL DETAILS.
14. "C" ON PLAN DENOTES CONCRETE COLUMNS. COLUMNS ARE CENTERED ON GRID INTERSECTIONS UN.
15. "CJ" AND "SJ" ON PLAN DENOTE CONSTRUCTION JOINT AND CONTROL JOINT. CONSTRUCTION JOINTS SHALL BE LOCATED AT THE FACE OF THE WALL OR GRADE SHALL BE DONE AS SOON AS CONCRETE HAS HARDENED SUFFICIENTLY TO PERMIT CURING WITHOUT CHIPPING, SPALLING OR TEARING BUT NOT MORE THAN 24 HOURS AFTER POURING.
16. FOR GENERAL STRUCTURAL NOTES FOR ADD. INFO. CONTRACTOR TO CONSULT ARCHITECTURAL DRAWINGS AND DETAILS. PROVIDE A SUBMITTAL OF THE CONSTRUCTION/CONCRETE JOINT LOCATIONS TO THE DESIGN TEAM FOR REVIEW.
17. PROVIDE TEMPORARY BRACING FOR ALL CONCRETE WALLS, COLUMNS, AND PLUMB CONNECTION.
18. PROVIDE UNDERGROUND UTILITY FOR LOCAL SECTIONS SEE CIVIL, BASEMENT, ELECTRICAL AND FIRE PROTECTION DRAWINGS.

SOIL IMPROVEMENTS BENEFIT THE BUILDING WILL BE COMPLETED BY OTHERS, UNDER A SEPARATE PERMIT. FOR THE BUILDING DESIGN, THE FOLLOWING IS REQUIRED:

- 1) THE 15% ALLOWABLE BEARING CAPACITY AND MAT FOOTINGS SHALL BE UNIFORMLY IMPROVED TO SUPPORT THE ALLOWABLE BEARING PRESSURE CALLED OUT IN THE STRUCTURAL NOTES ON THE FOUNDATION. THE ALLOWABLE BEARING CAPACITY OF THE SOIL IMPROVEMENT SYSTEM, THE ALLOWABLE BEARING PRESSURE SHALL BE MULTIPLIED BY THE FOOTING PLAN DIMENSIONS TO OBTAIN THE ALLOWABLE LOAD.
- 2) THE MAXIMUM TOTAL SETTLEMENT BENEATH AN INDIVIDUAL FOOTING SHALL NOT EXCEED 1". THE MAXIMUM DIFFERENTIAL SETTLEMENT SHALL NOT EXCEED 0.005" PER INCH OF 30-FOOT AND 100-FOOT INTERVALS SHALL NOT BE GREATER THAN 1/2" AND 1" RESPECTIVELY.
- 3) THE SOIL BENEATH ALL SLABS SHALL BE IMPROVED TO PROVIDE UNIFORM SUPPORT AND SHALL PROVIDE THE FOLLOWING:
 - A) TOTAL LONG-TERM SETTLEMENT OF LESS THAN 1/4" UNDER 30-FOOT INTERVALS.
 - B) MAXIMUM DIFFERENTIAL SETTLEMENT OF 3/16" OVER A DISTANCE OF 16'-0" UNDER 300 PSF LOADING.

THE MINIMUM MODULUS OF 75 GPC/INCH BENEATH THE FULL EXTENT OF THE SLAB.



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[illegible]

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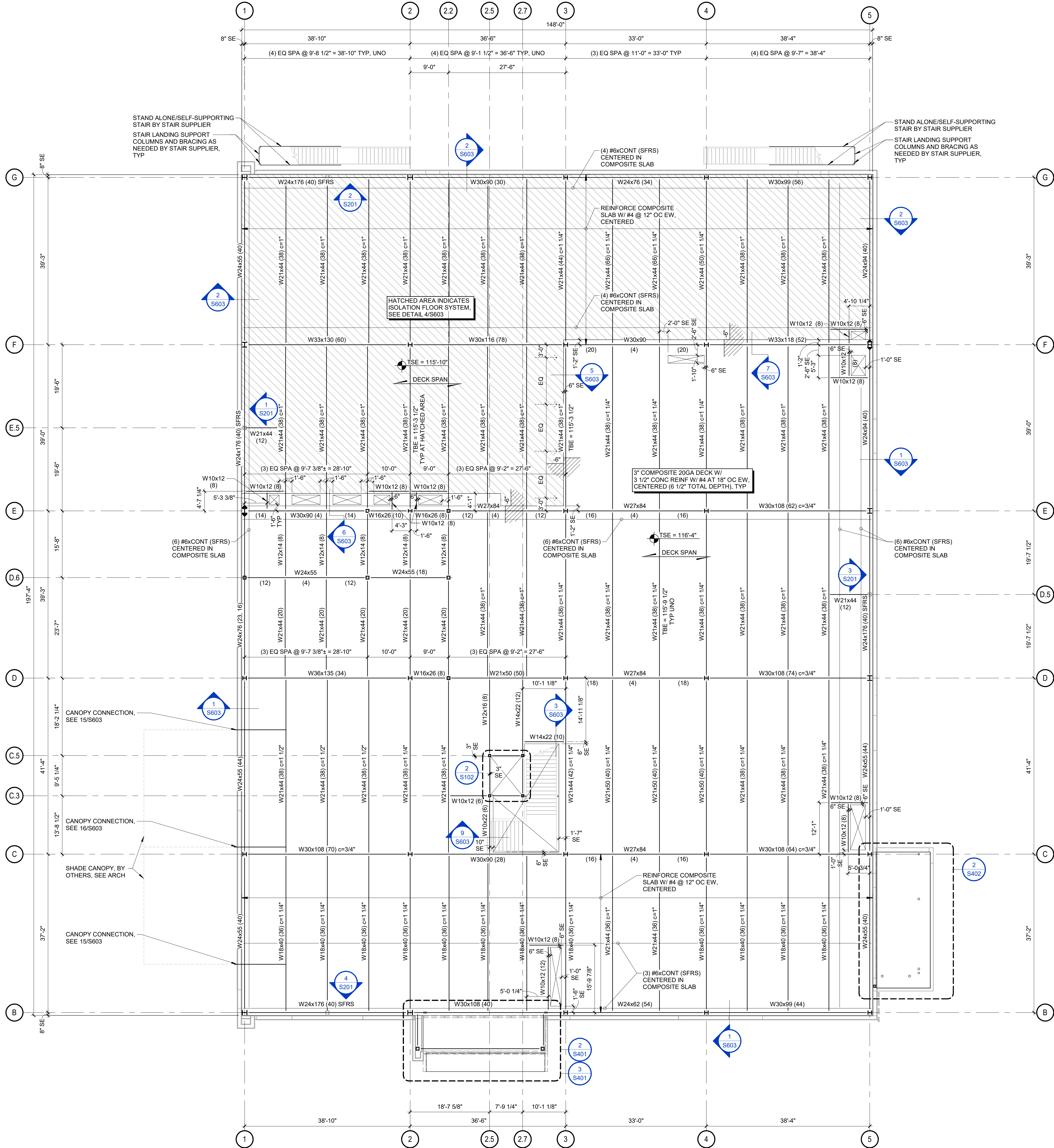


S101

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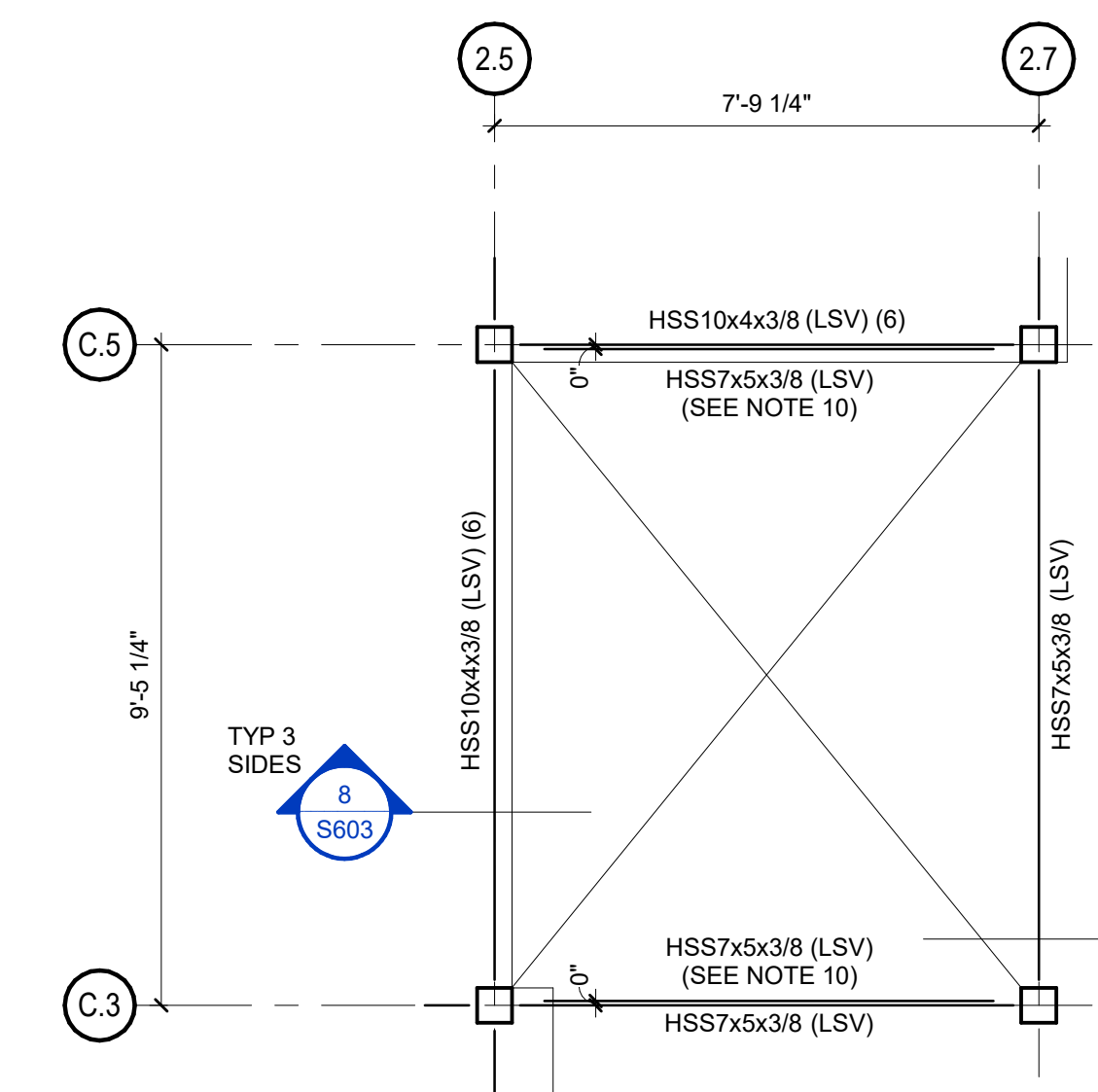
FOUNDATION PLAN

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C
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1
S102
3/32" = 1'-0"

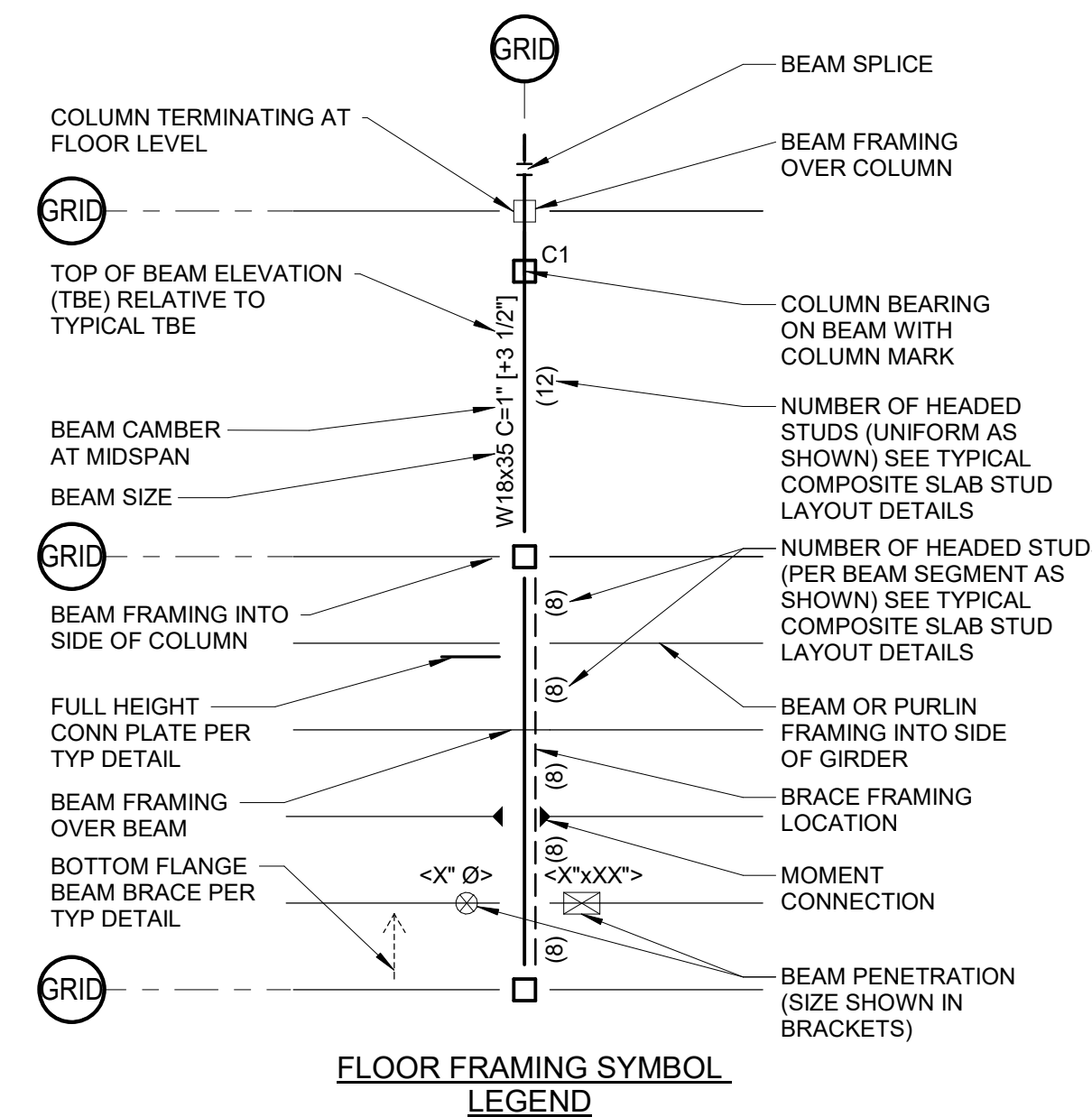
SECOND FLOOR FRAMING PLAN



2
S102
3/8" = 1'-0"

ENLARGED FRAMING AT ELEVATOR

- ### FLOOR FRAMING PLAN NOTES
- FOR GENERAL STRUCTURAL NOTES SEE SHEET S001.
 - FOR TYPICAL FLOOR FRAMING DETAILS SEE SHEETS S601 & S602.
 - TOP OF BEAM ELEVATION (TBE) VARIES. SEE PLANS. [X] INDICATES VARIATIONS OF TOP OF MEMBER FROM TYPICAL TOP OF BEAM ELEVATION IN INCHES.
 - TYPICAL TOP OF BEAM ELEVATION AT SECOND FLOOR = 11'-9 1/2" UNO.
 - HEADED STUDS ARE 3/4" DIA. SEE S3602 FOR LENGTH.
 - BRACE ALL COLUMNS THAT DO NOT HAVE BEAMS FRAMING INTO BOTH FLANGE AND WEB (AT LEAST ONE SIDE) UNTIL CONCRETE HAS CURED 5 DAYS.
 - ALL GIRDER CONNECTIONS SHALL BE DOUBLE ANGLE CONNECTIONS UNO. SEE TYPICAL DETAILS. A GIRDER IS DEFINED AS A STEEL BEAM WITH CONCENTRATED LOADS FROM OTHER BEAMS, PURLINS, OR COLUMNS FRAMING INTO IT.
 - ALL PURLIN CONNECTIONS SHALL BE SINGLE PLATE CONNECTIONS UNO. SEE TYPICAL DETAILS. A PURLIN IS DEFINED AS A STEEL BEAM WITH UNIFORM LOADS FROM FLOOR SLAB/DECKING ONLY.
 - MEMBERS DESIGNATED AS "SFRS" AND THEIR CONNECTIONS ARE PART OF THE BUILDING SEISMIC-FORCE-RESISTING SYSTEM AND ARE SUBJECT TO THE ADDITIONAL SEISMIC REQUIREMENTS FOR SPECIAL INSPECTIONS. REFER TO THE SPECIAL INSPECTION SCHEDULE ON SHEET S002 FOR A SUMMARY OF THE REQUIREMENTS.
 - BOTTOM FLANGE BEAM BRACES SHALL BE EQUALLY SPACED UNO.
 - 7x5x3/8 (LSV) - T/O BEAM EL = 10'-0"; GC VERIFY ELEVATION WITH ELEVATOR SUPPLIER.
 - BRACING AND ANCHORAGE OF NON-STRUCTURAL COMPONENTS (CEILINGS, PIPING, CONDUIT, EQUIPMENT, ETC.) SHALL BE DESIGNED/DETAILED BY OTHERS. DESIGN SEISMIC LOADING SHALL BE AS REQUIRED IN CHAPTER 13 OF ASCE 7-16 UTILIZING THE BASIC SEISMIC ACCELERATION PARAMETERS SHOWN IN THE STRUCTURAL NOTES ON SHEET S001. REFER TO THE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL AND FIRE PROTECTION DRAWINGS AND SPECIFICATIONS FOR ADDITIONAL INFORMATION.



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PROJECT INFORMATION

Holladay, UT

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DRAWING LOG

REV	DATE	DESCRIPTION

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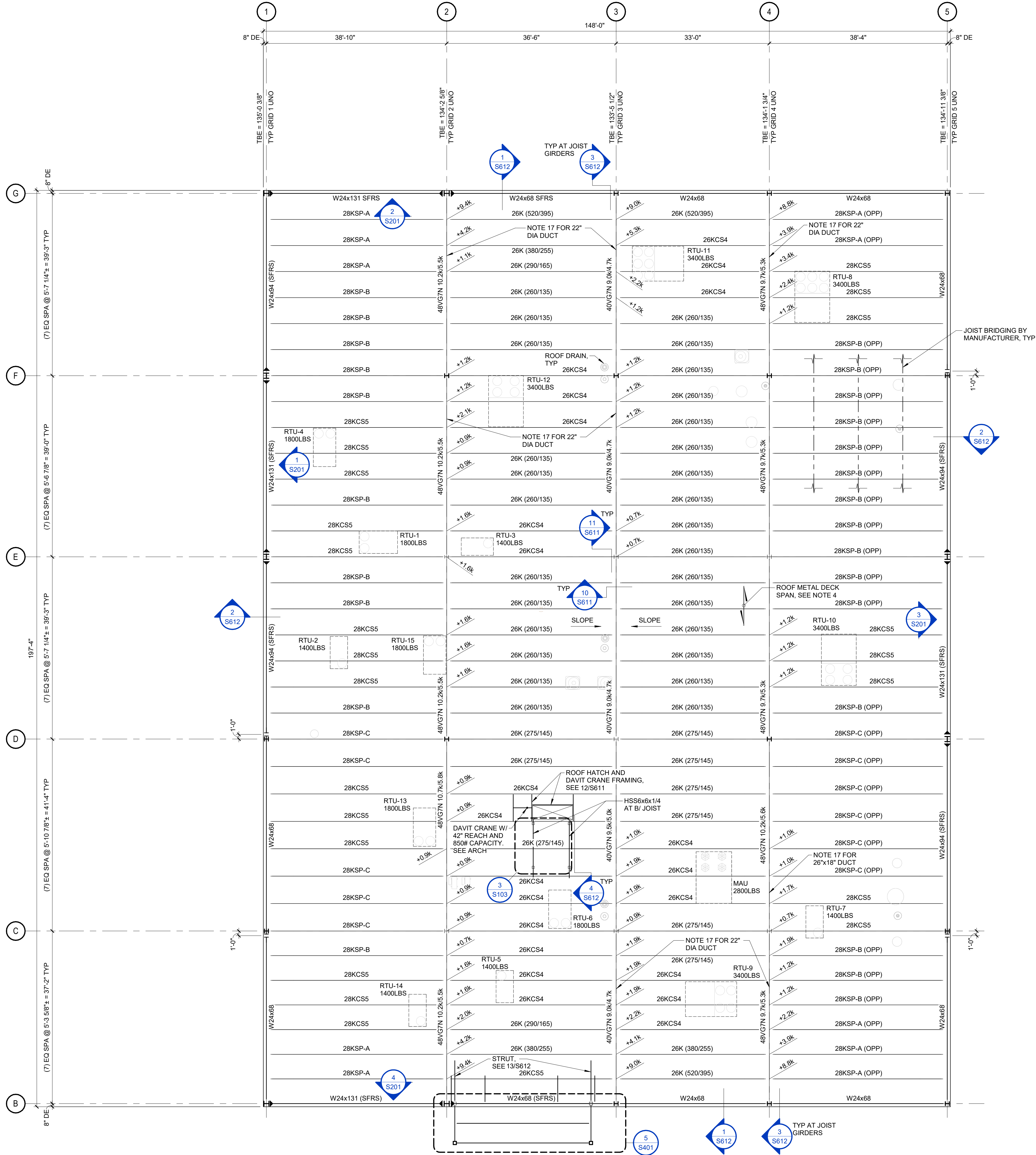
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BRIAN M. DALTON
STATE OF UTAH
No. 9832536

SECOND FLOOR FRAMING PLAN

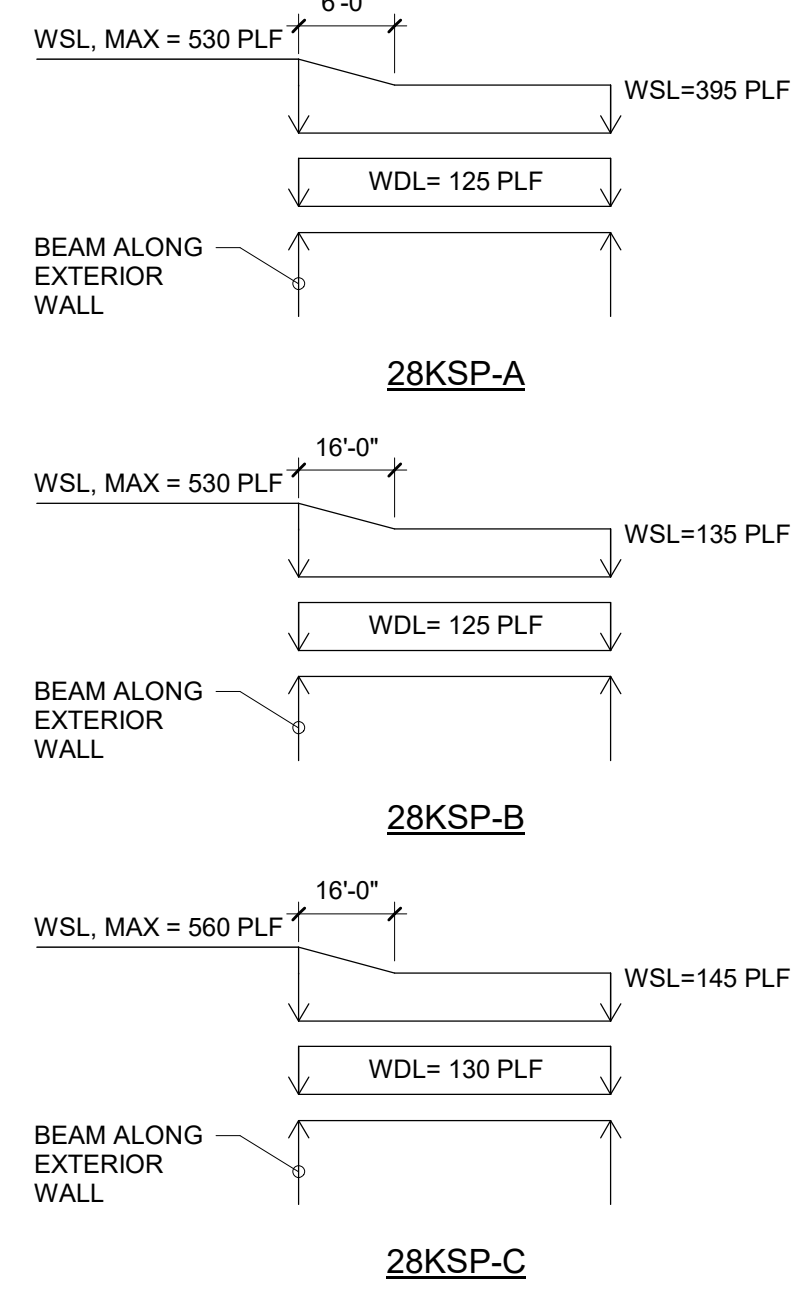
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1 S103 ROOF FRAMING PLAN

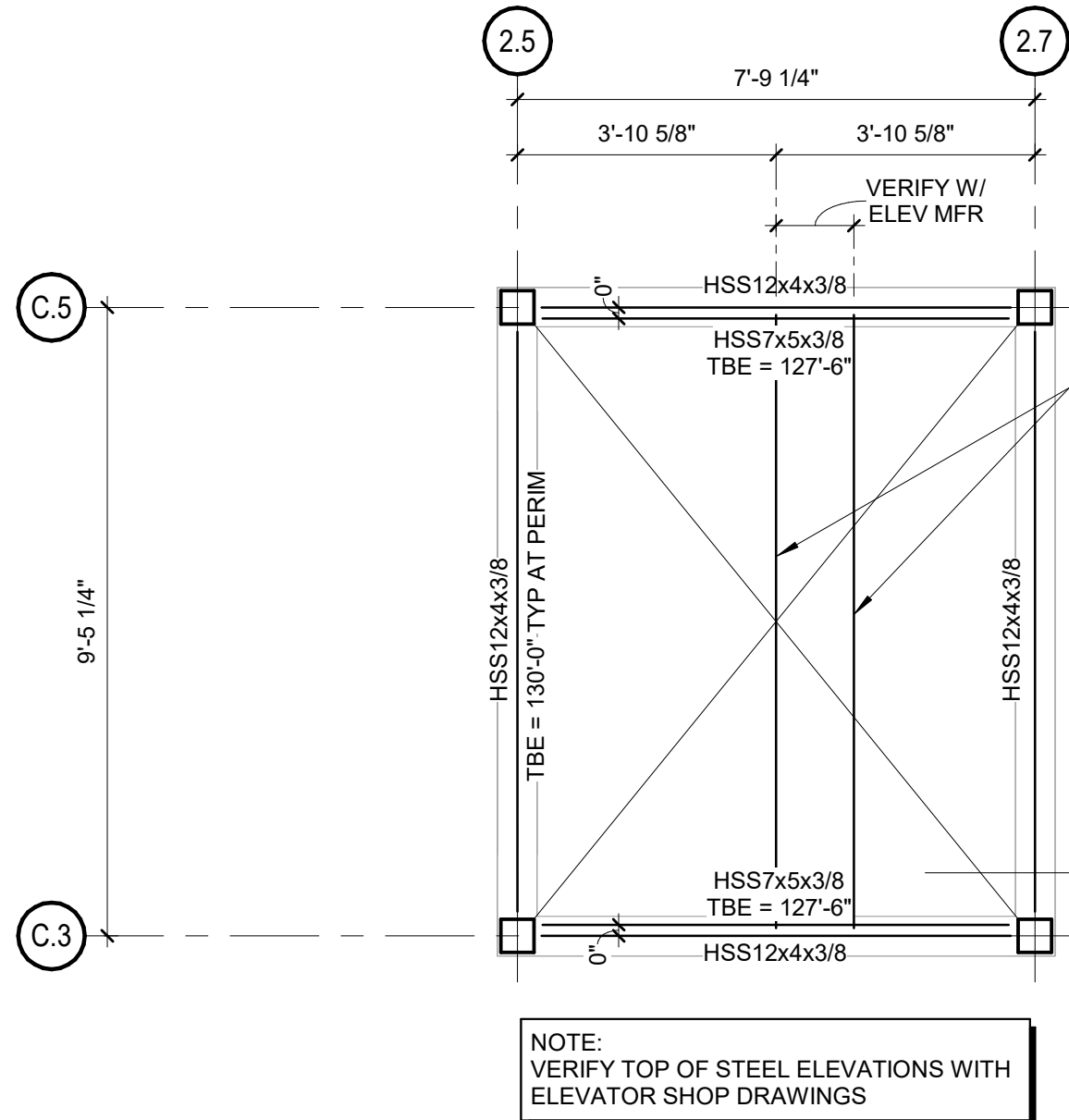
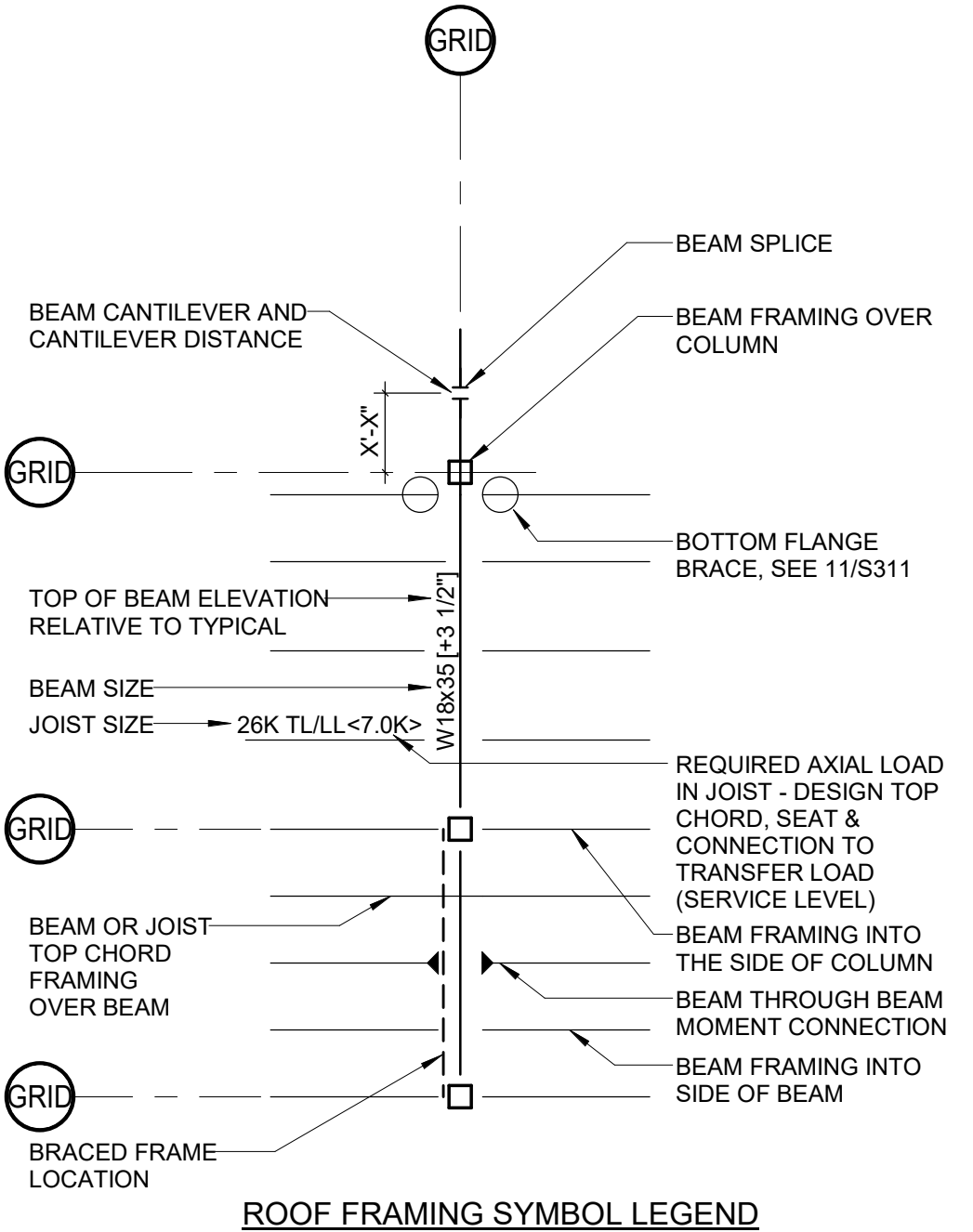
3/32" = 1'-0"

2 S103 SPECIAL JOIST LOADING DIAGRAMS



ROOF FRAMING PLAN NOTES

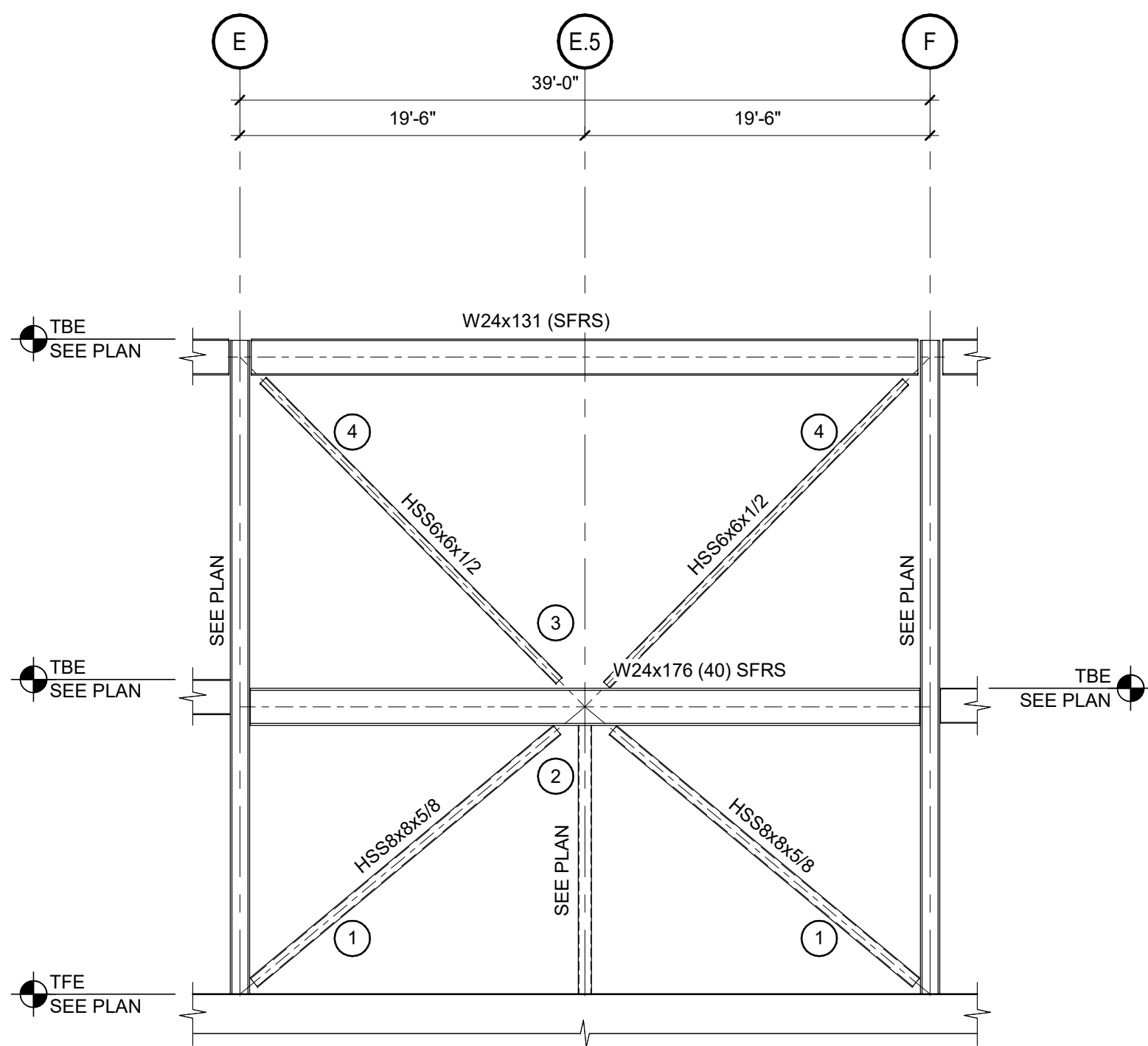
- FOR GENERAL STRUCTURAL NOTES SEE SHEET S001.
- FOR TYPICAL ROOF FRAMING DETAILS SEE SHEET S611.
- TOP OF BEAM ELEVATION (TBE) = VARIES; SEE PLAN.
- PROVIDE 1 1/2" DEEP x 18GA. G80 GALV TYPE PLB OR DGB ACOUSTICAL ROOF DECK WITH INTERLOCKING SIDELAPS (VULCRAFT PLB/PUNCHLOK OR ASC DGB/DILT/TAIRP). FOR DECK FASTENING SEE DETAIL 1/S611.
- DE ON PLAN DENOTES EDGE OF DECK OR DECK ANGLE.
- OSHA BOLTED JOIST CONNECTIONS ARE NOT SHOWN IN THE DETAILS. GENERAL CONTRACTOR TO COORDINATE THEIR LOCATIONS & QUANTITIES.
- ALL GIRDER CONNECTIONS SHALL BE DOUBLE ANGLE CONNECTIONS UNO, SEE TYPICAL DETAILS. A GIRDER IS DEFINED AS A STEEL BEAM WITH CONCENTRATED LOADS FROM OTHER BEAMS, PURLINS, JOISTS, OR COLUMNS FRAMING INTO IT.
- ALL PURLIN CONNECTIONS SHALL BE SINGLE PLATE CONNECTIONS UNO, SEE TYPICAL DETAILS. A PURLIN IS DEFINED AS A STEEL BEAM WITH UNIFORM LOADS FROM FLOOR SLAB/DECKING ONLY.
- FOR FRAMES AT ROOF OPENING & MECHANICAL EQUIPMENT SEE TYPICAL DETAILS. VERIFY SIZE & LOCATION OF ROOF OPENINGS WITH OTHER TRADES.
- JOIST GIRDER TAG / JOIST TAG LEGEND:
 - JOIST DEPTH IN INCHES
 - K SERIES JOIST PER SJI
 - TOTAL SERVICE LOAD (PLF)
 - LIVE OR SNOW LOAD SERVICE LOAD (PLF)
 - JOIST GIRDER DEPTH IN INCHES
 - NO. OF JOISTS SPACES PER SPAN
 - TOTAL UNFACTORED LOAD PER PANEL POINT (KIPS)
 - LIVE OR SNOW LOAD UNFACTORED LOAD PER PANEL POINT (KIPS)
- 28KSP-X INDICATES JOIST WITH SPECIAL LOADING. REFER TO SPECIAL JOIST LOADING DIAGRAMS ON THIS SHEET.
- FOR MECHANICAL UNIT MAXIMUM WEIGHTS SEE PLAN. MAXIMUM WEIGHTS INDICATED ON PLAN ACCOUNT FOR CURB WEIGHT.
- JOIST/JOIST GIRDER SUPPLIER SHALL DESIGN ALL JOISTS AND JOIST GIRDERS AND THEIR CONNECTIONS TO SUPPORTING STRUCTURE FOR A MINIMUM SERVICE LEVEL AXIAL LOAD OF 5.0kips AND 10.0kips RESPECTIVELY.
- BRACING AND ANCHORAGE OF NON-STRUCTURAL COMPONENTS (CEILING, PIPING, CONDUIT, EQUIPMENT, ETC.) SHALL BE DESIGNED/DETAILED BY OTHERS. DESIGN SEISMIC LOADING SHALL BE AS REQUIRED IN CHAPTER 13 OF ASCE 7-16 UTILIZING THE BASIC SEISMIC ACCELERATION PARAMETERS SHOWN IN THE STRUCTURAL NOTES ON SHEET S001. REFER TO THE ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL AND FIRE PROTECTION DRAWINGS AND SPECIFICATIONS FOR ADDITIONAL INFORMATION.
- MEMBERS DESIGNATED AS "SFRS" AND THEIR CONNECTIONS ARE PART OF THE BUILDING SEISMIC-FORCE-RESISTING SYSTEM AND ARE SUBJECT TO THE ADDITIONAL SEISMIC REQUIREMENTS FOR SPECIAL INSPECTIONS. REFER TO THE SPECIAL INSPECTION SCHEDULE ON SHEET S002 FOR A SUMMARY OF THE REQUIREMENTS.
- JOIST GIRDER SHALL BE DESIGNED/CONFIGURED TO ALLOW PASSAGE THROUGH THE JOIST GIRDER OF THE DUCT SIZE NOTED; CONTRACTOR TO COORDINATE.



3 S103 ENLARGED FRAMING PLAN AT ELEVATOR

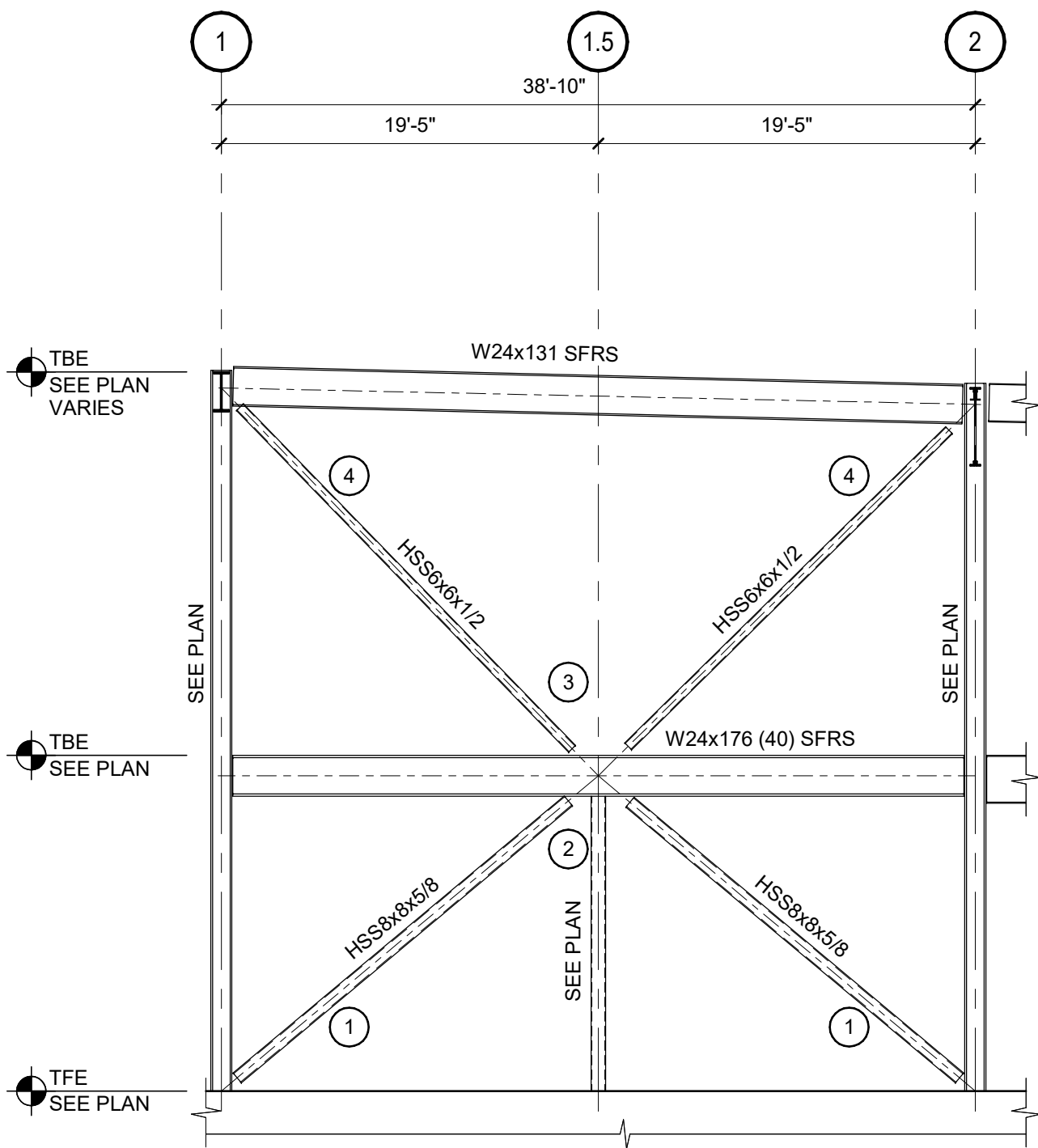
3/8" = 1'-0"

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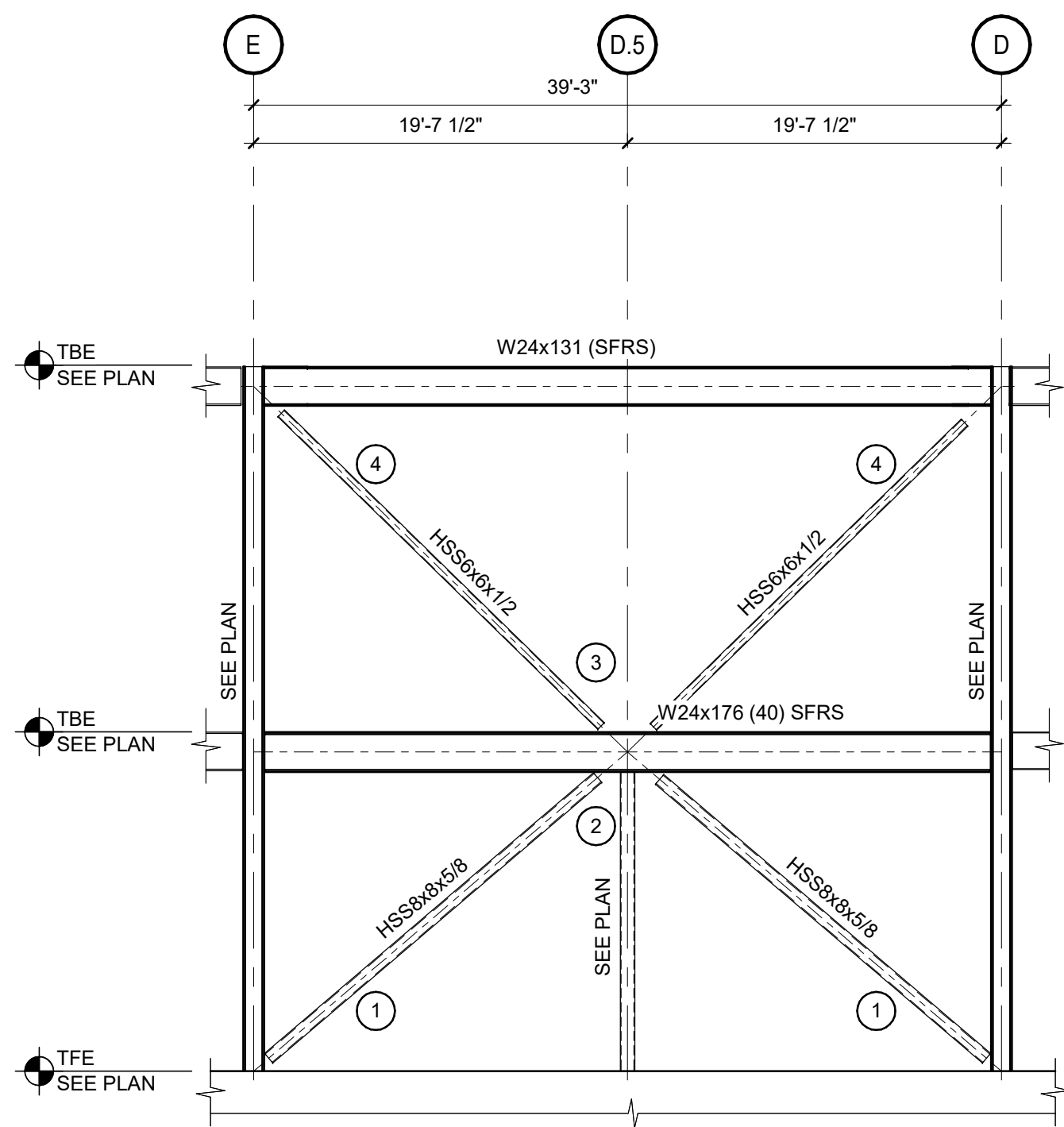
1
S201
1/8" = 1'-0"

BRACE FRAME ELEVATION ALONG GRID 1



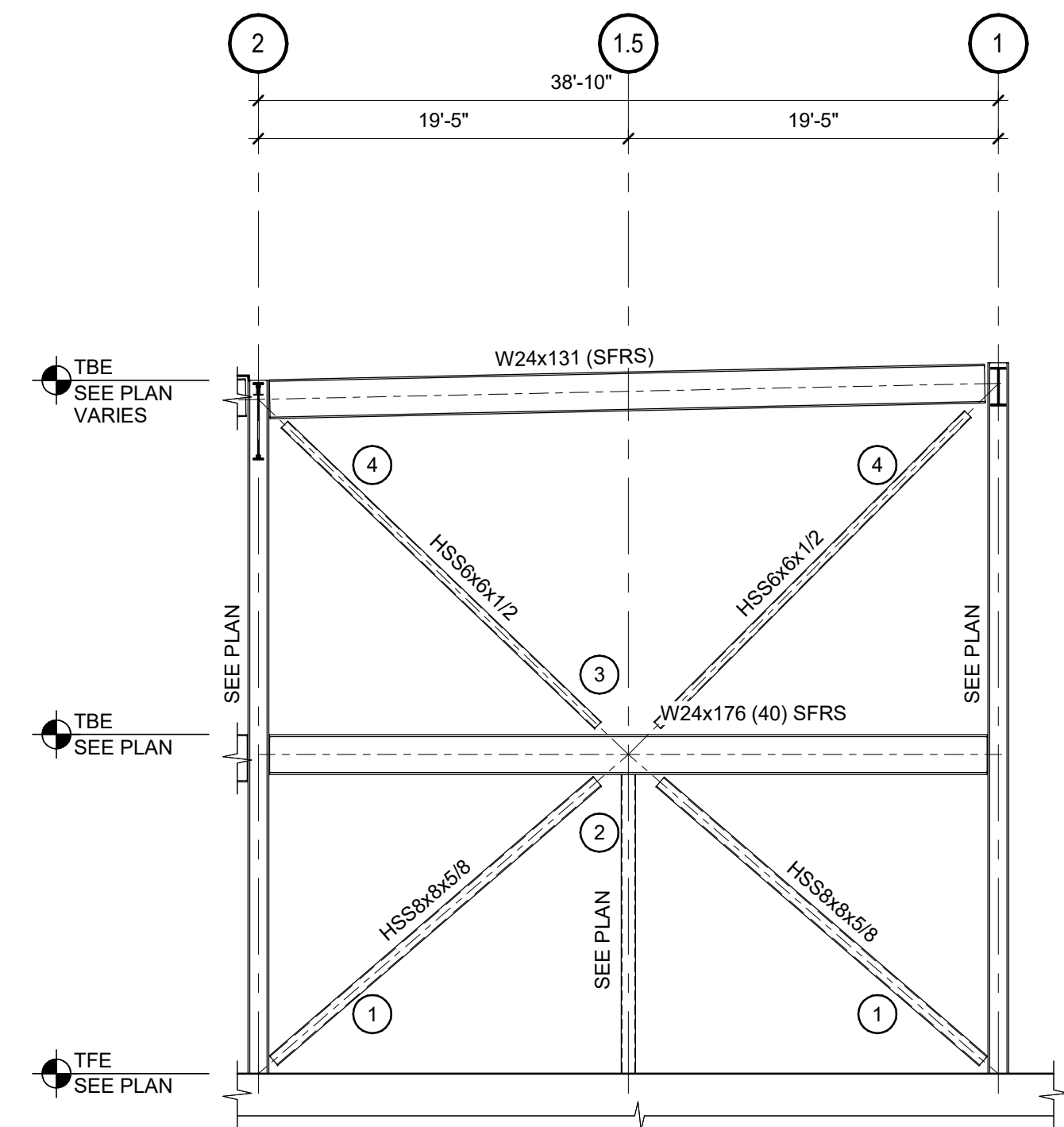
2
S201
1/8" = 1'-0"

BRACE FRAME ELEVATION ALONG GRID G



3
S201
1/8" = 1'-0"

BRACE FRAME ELEVATION ALONG GRID 5



4
S201
1/8" = 1'-0"

BRACE FRAME ELEVATION ALONG GRID B

- NOTES:
1. IN NO CASE SHALL ANY CONNECTIONS FOR ARCHITECTURAL, MECHANICAL, OR ELECTRICAL COMPONENTS OR OTHER ELEMENTS NOT SHOWN ON THESE DRAWINGS BE MADE TO DIAGONAL BRACING OR BRACING CONNECTIONS.
 2. (X) INDICATES BRACED FRAME CONNECTION TYPE PER THE BRACED FRAME CONNECTION SCHEDULE ON 3/S211.
 3. ALL BEAMS, COLUMNS, BRACES AND CONNECTIONS SHOWN/ANNOTATED ON THIS SHEET ARE PART OF THE BUILDING SEISMIC-FORCE-RESISTING SYSTEM AND ARE SUBJECT TO THE ADDITIONAL SEISMIC REQUIREMENTS FOR SPECIAL INSPECTIONS. REFER TO THE SPECIAL INSPECTIONS SCHED FOR A SUMMARY OF THE REQUIREMENTS.
 4. ALL BRACE WORK POINTS (WPI) ARE AT THE INTERSECTION OF COLUMN AND BEAM CENTERLINES, UNLESS NOTED OTHERWISE.

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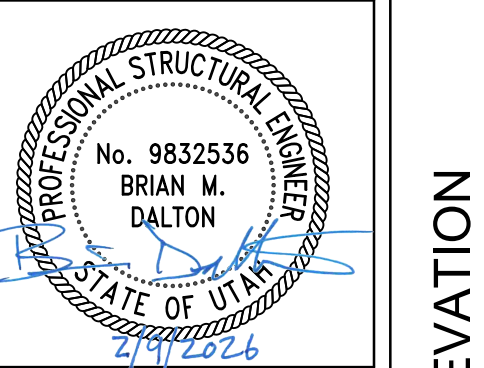
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REV DATE DESCRIPTION

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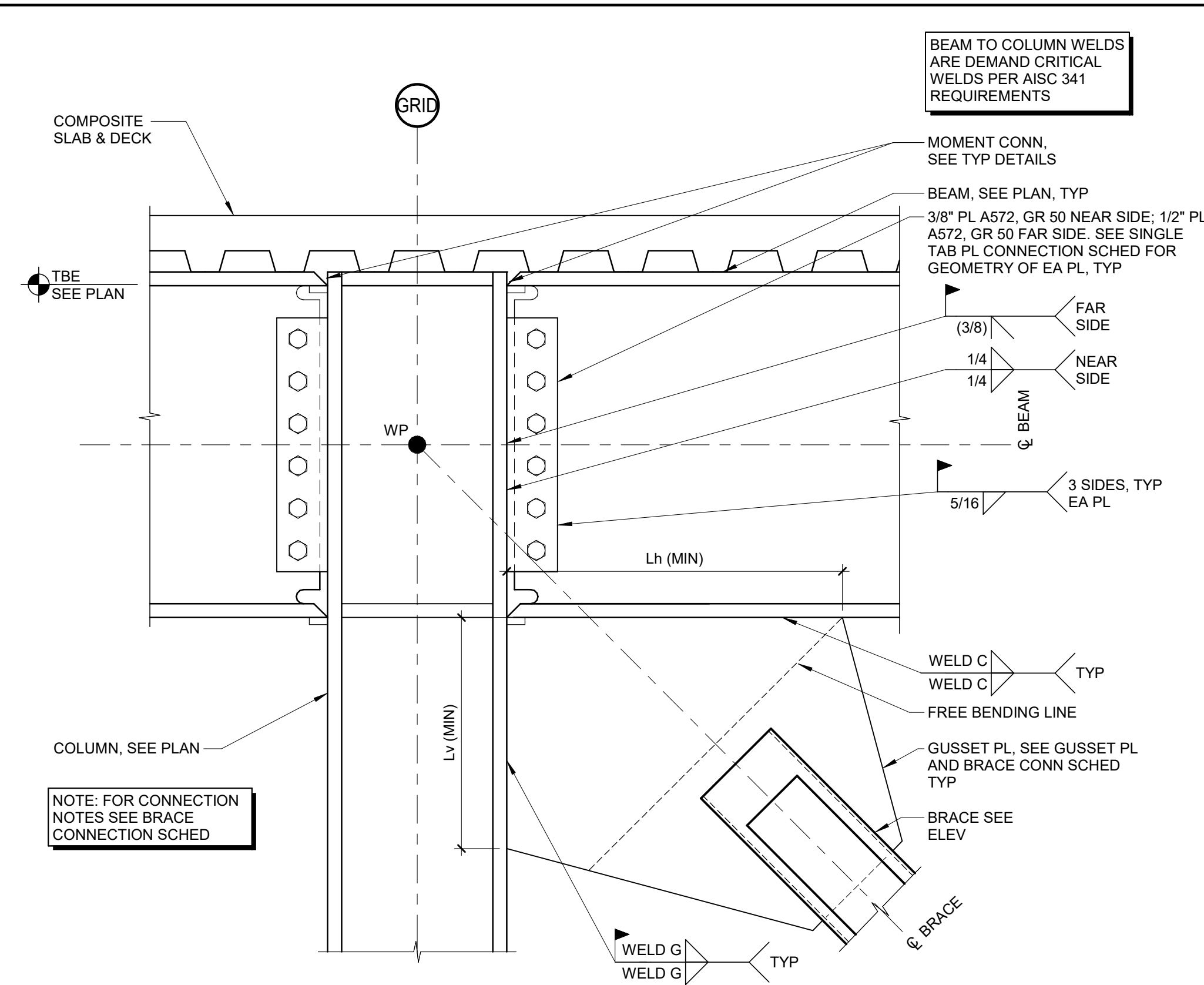
ISSUE DATE: 02/09/26

SHEET NO.

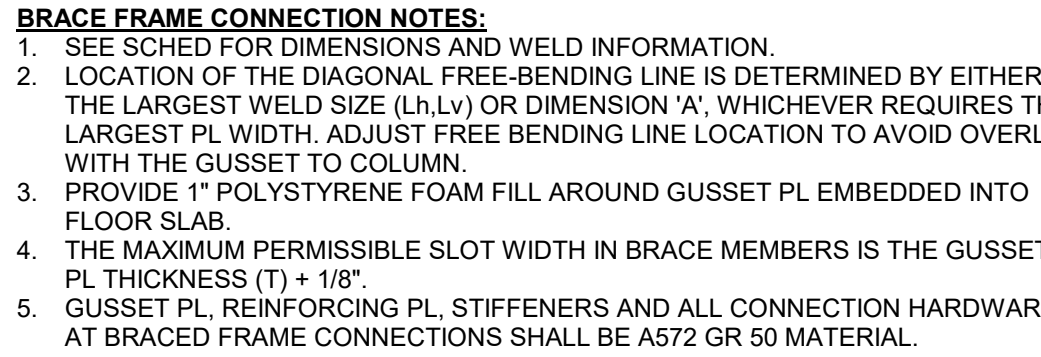
S201

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BRACED FRAME ELEVATION



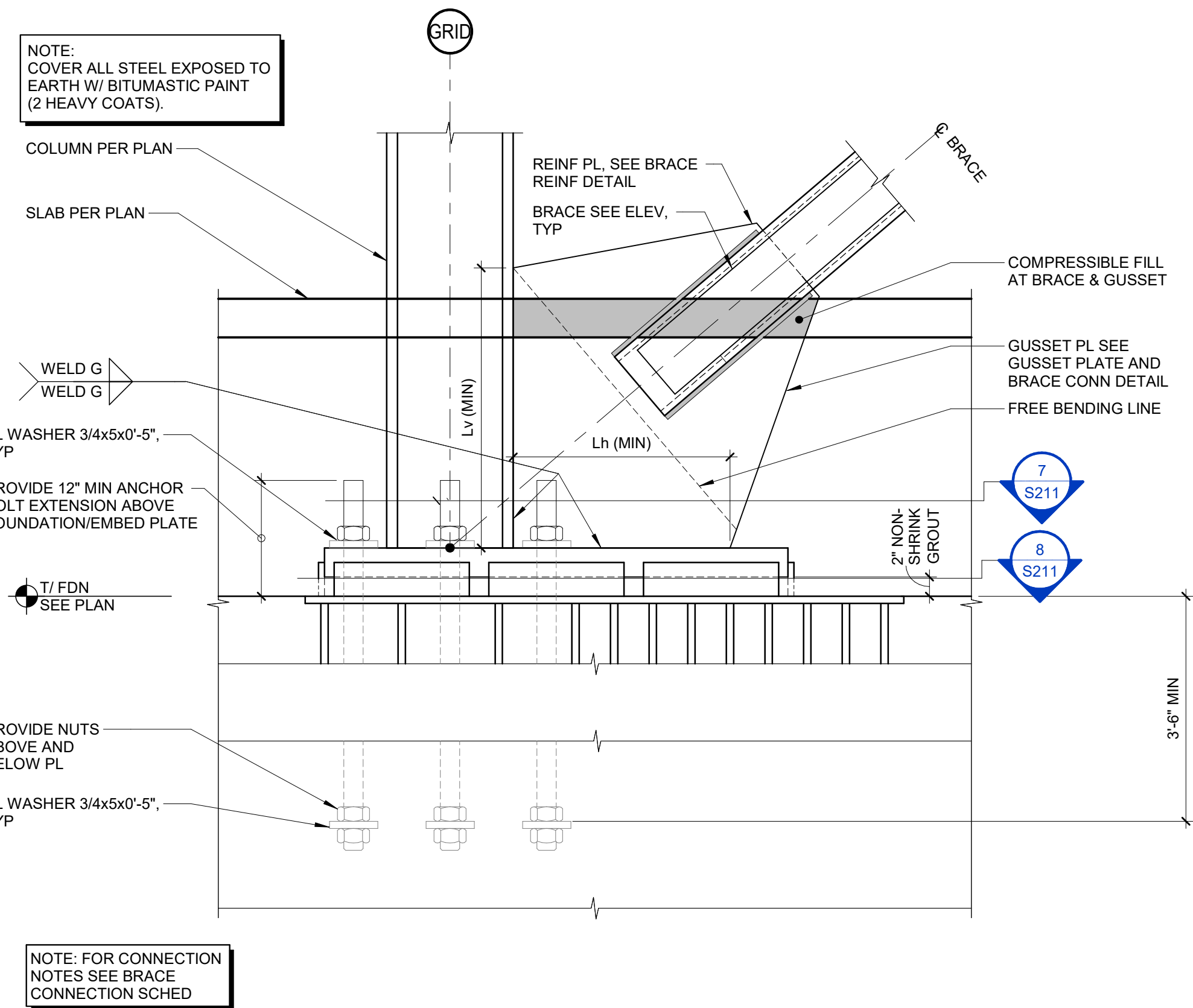
BRACE SIZE	REINFORCEMENT PLATE	WELD SIZE (W)
HSS8x8x5/8	1/2"x6"x3'-0"	1/4
HSS6x6x1/2	1/2"x4"x2'-0"	1/4



4
S211

BRACE CONN DETAIL

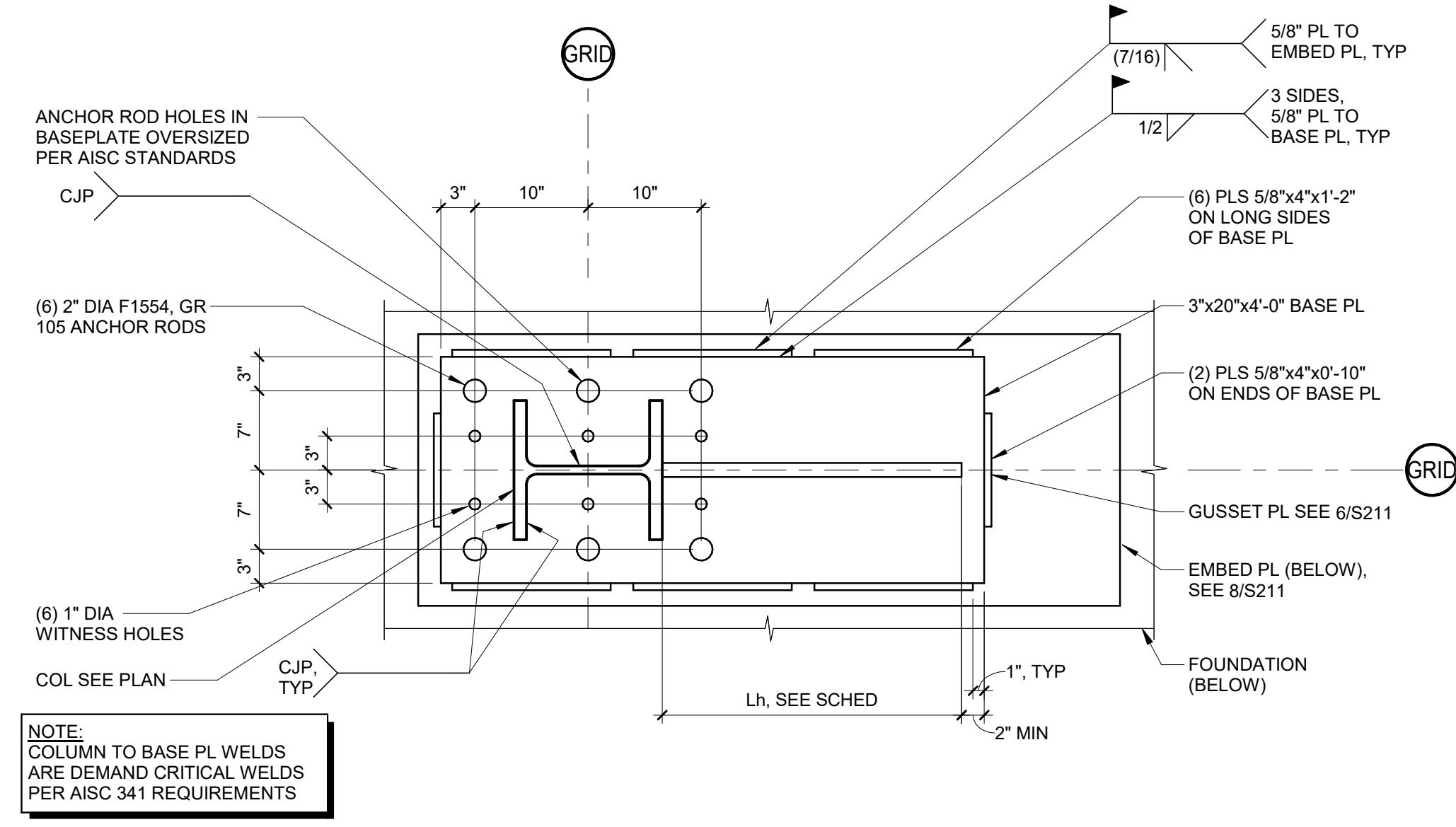
1 1/2" = 1'-0"



6
S211

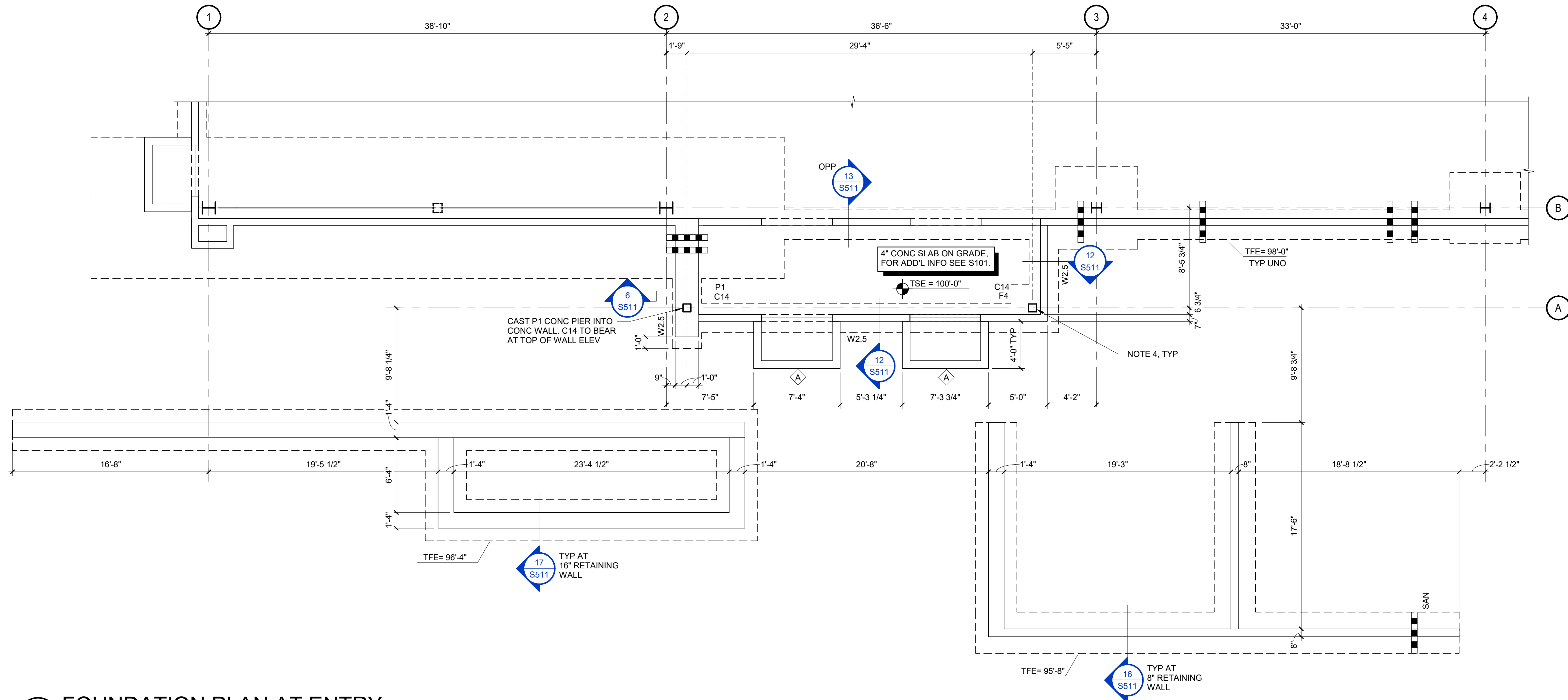
BRACE CONN DETAIL

1" = 1'-0"



7 BASE PLATE DETAIL
S211 1" = 1'-0"



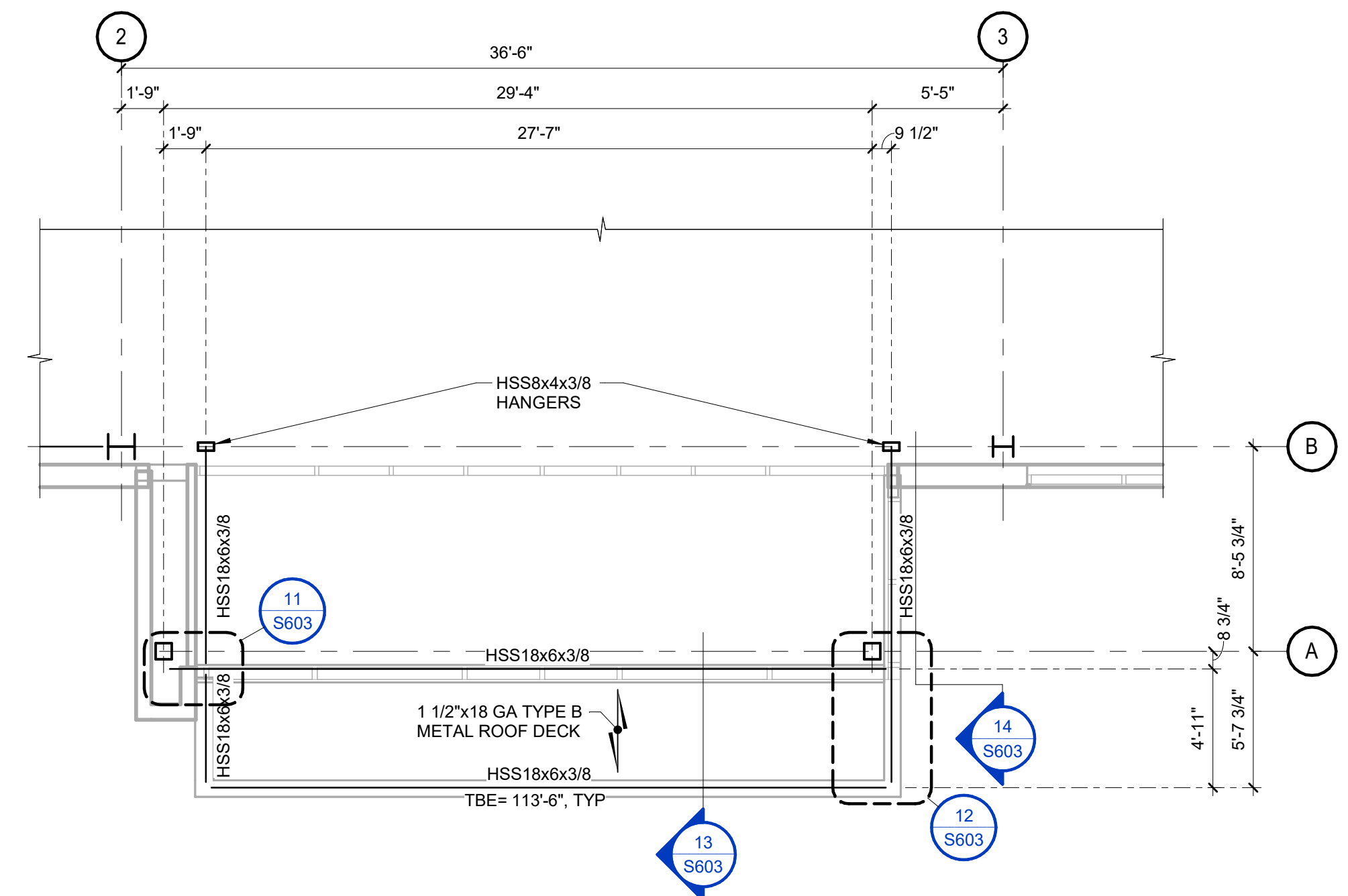


1 FOUNDATION PLAN AT ENTRY

S401 3/16" = 1'-0"

NOTES:

1. FOR GENERAL STRUCTURAL NOTES SEE SHEET S001.
2. FOR TYPICAL CONCRETE DETAILS SEE SHEET S501 & S502.
3. SEE SHEET S101 FOR ADDITIONAL NOTES.
4. PROVIDE TEMPORARY BLOCKOUT IN FOUNDATION WALL TO INSTALL COLUMN AND BASE CONNECTION IF NEEDED.

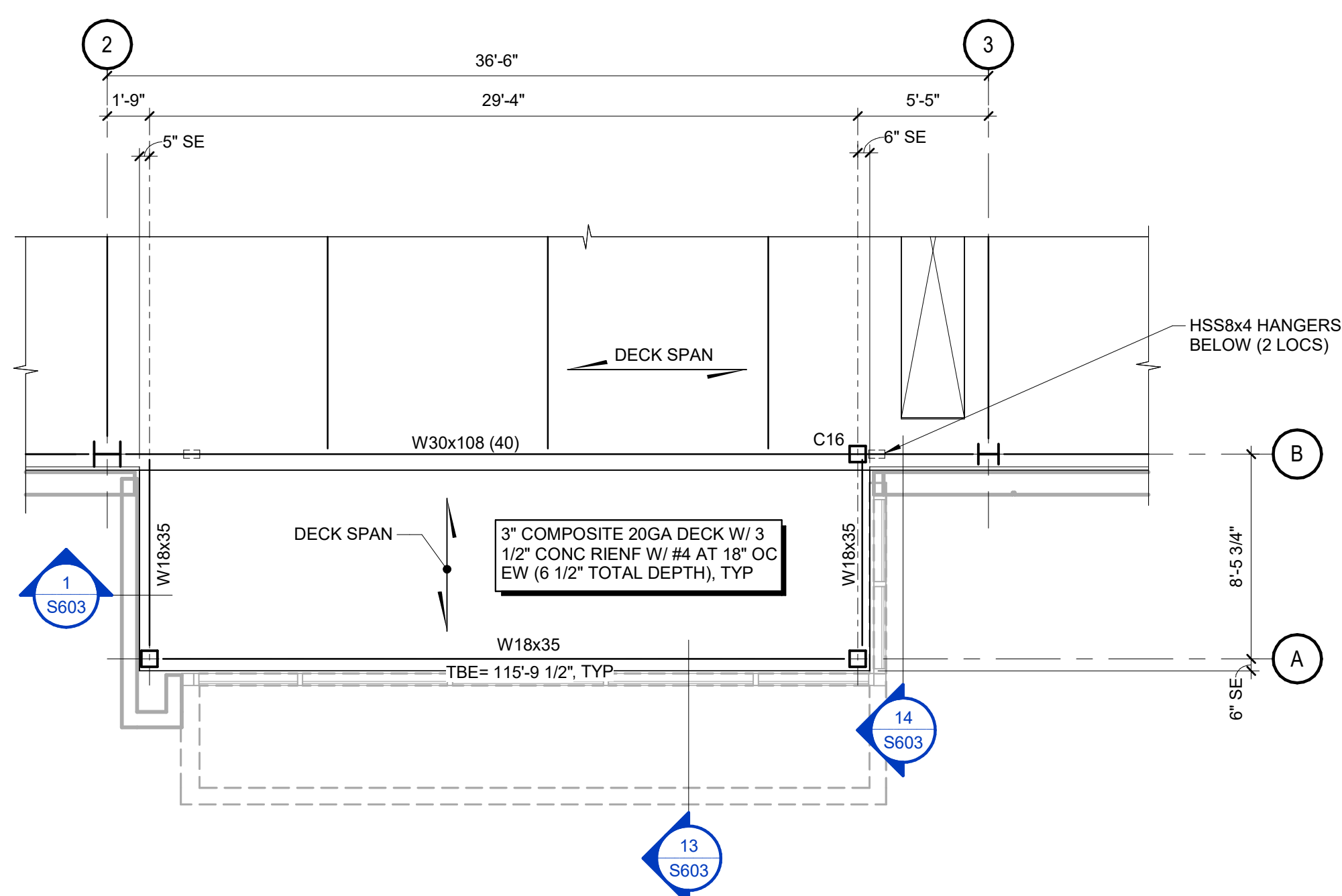


2 CANOPY FRAMING PLAN AT ENTRY

S401 3/16" = 1'-0"

NOTES:

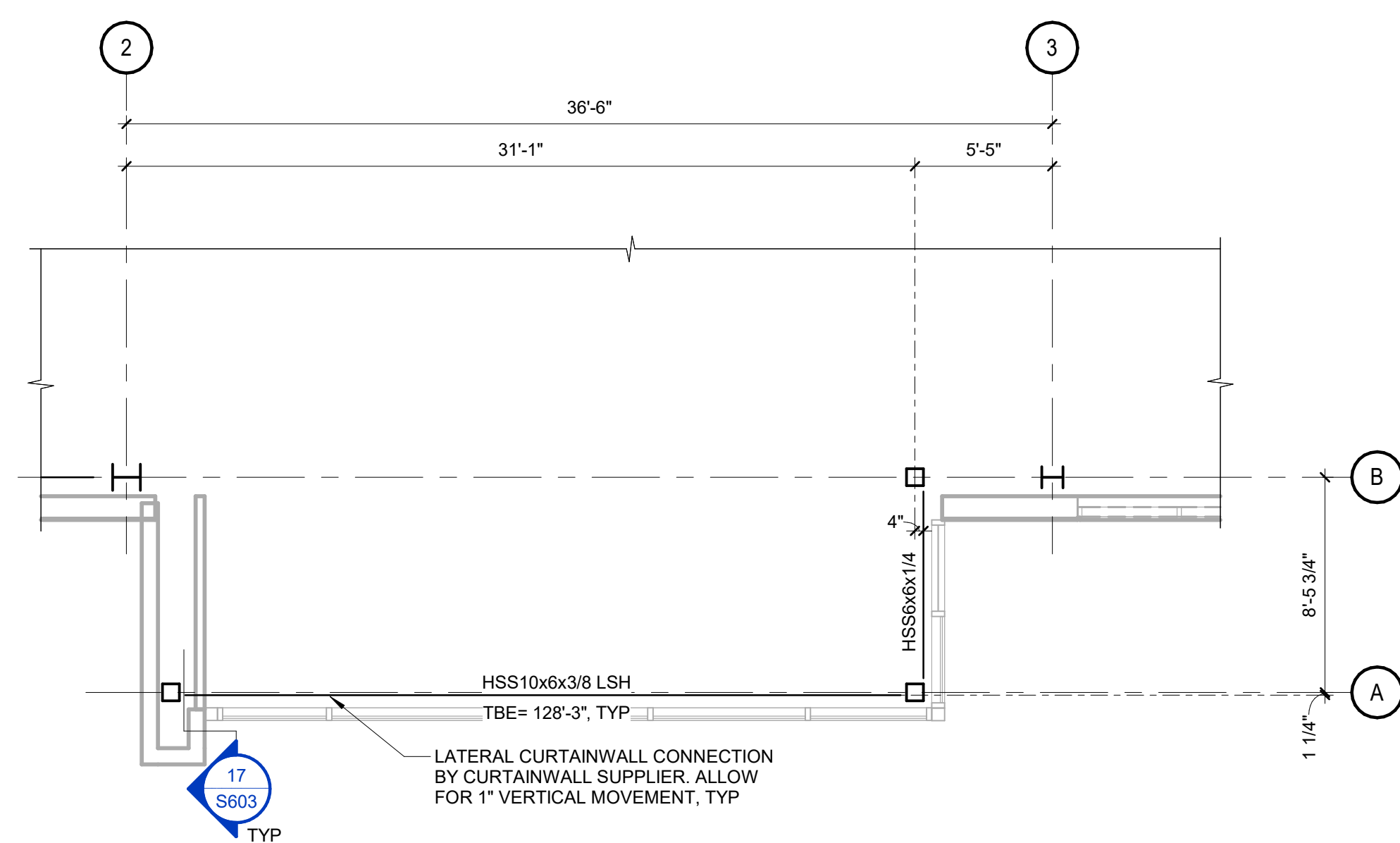
1. FOR GENERAL STRUCTURAL NOTES SEE SHEET S001.
2. FOR TYPICAL FRAMING DETAILS SEE SHEET S601.



3 SECOND FLOOR FRAMING PLAN AT ENTRY

S401 3/16" = 1'-0"

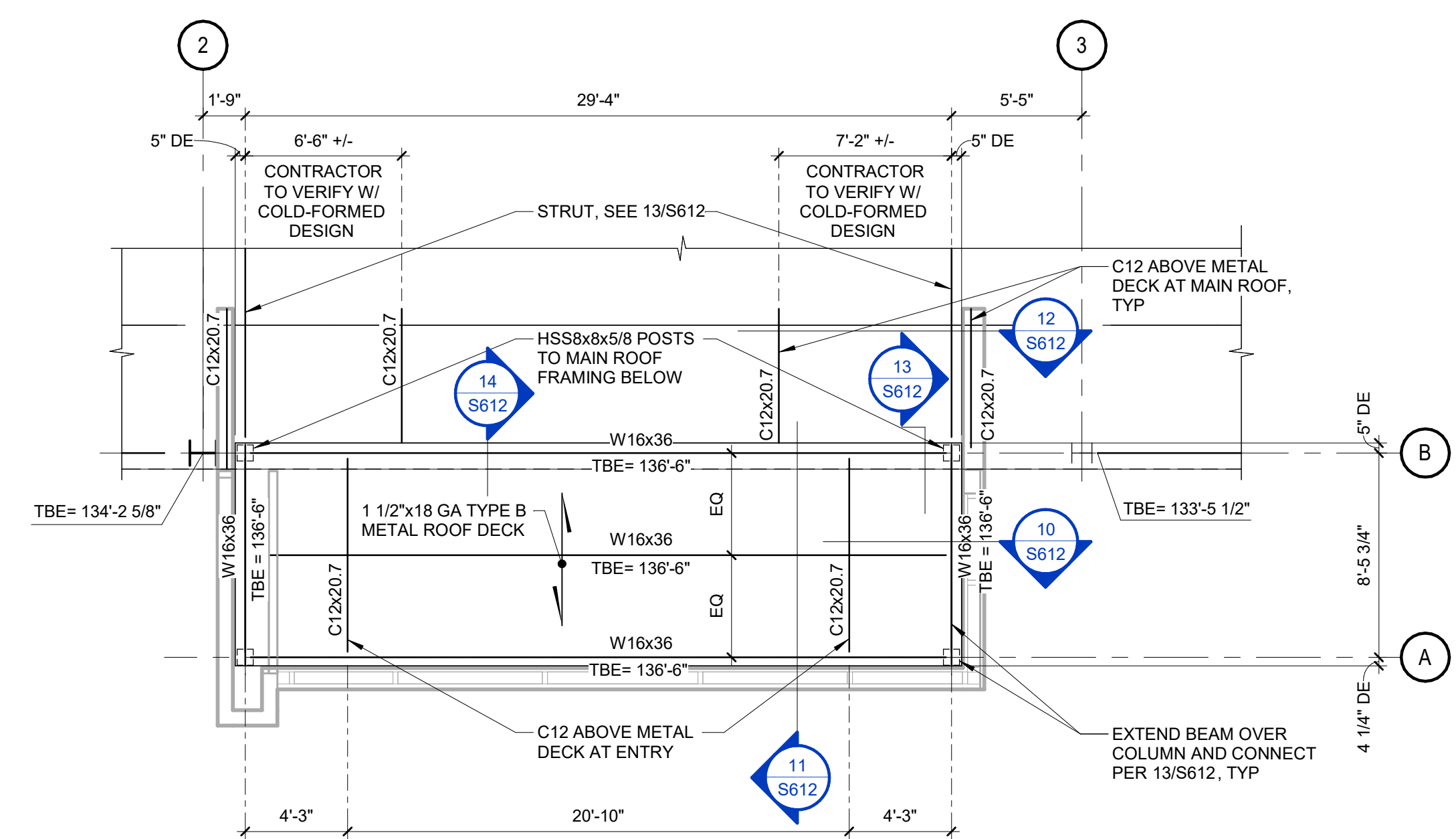
NOTES:
1. FOR GENERAL STRUCTURAL NOTES SEE SHEET S001.
2. FOR TYPICAL FLOOR FRAMING DETAILS SEE SHEET SEE S601 & S602.
SEE SHEET S102 FOR ADDITIONAL NOTES.



4 GIRT FRAMING AT ENTRY

4
S401 3/16" = 1'-0"

NOTES:
1. FOR GENERAL STRUCTURAL NOTES SEE SHEET S001.
2. FOR TYPICAL FRAMING DETAILS SEE SHEET SEE SHEET S601.



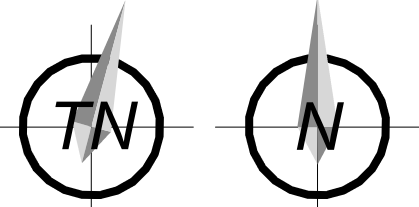
5 FRAMING ABOVE ROOF PLAN AT ENTRY

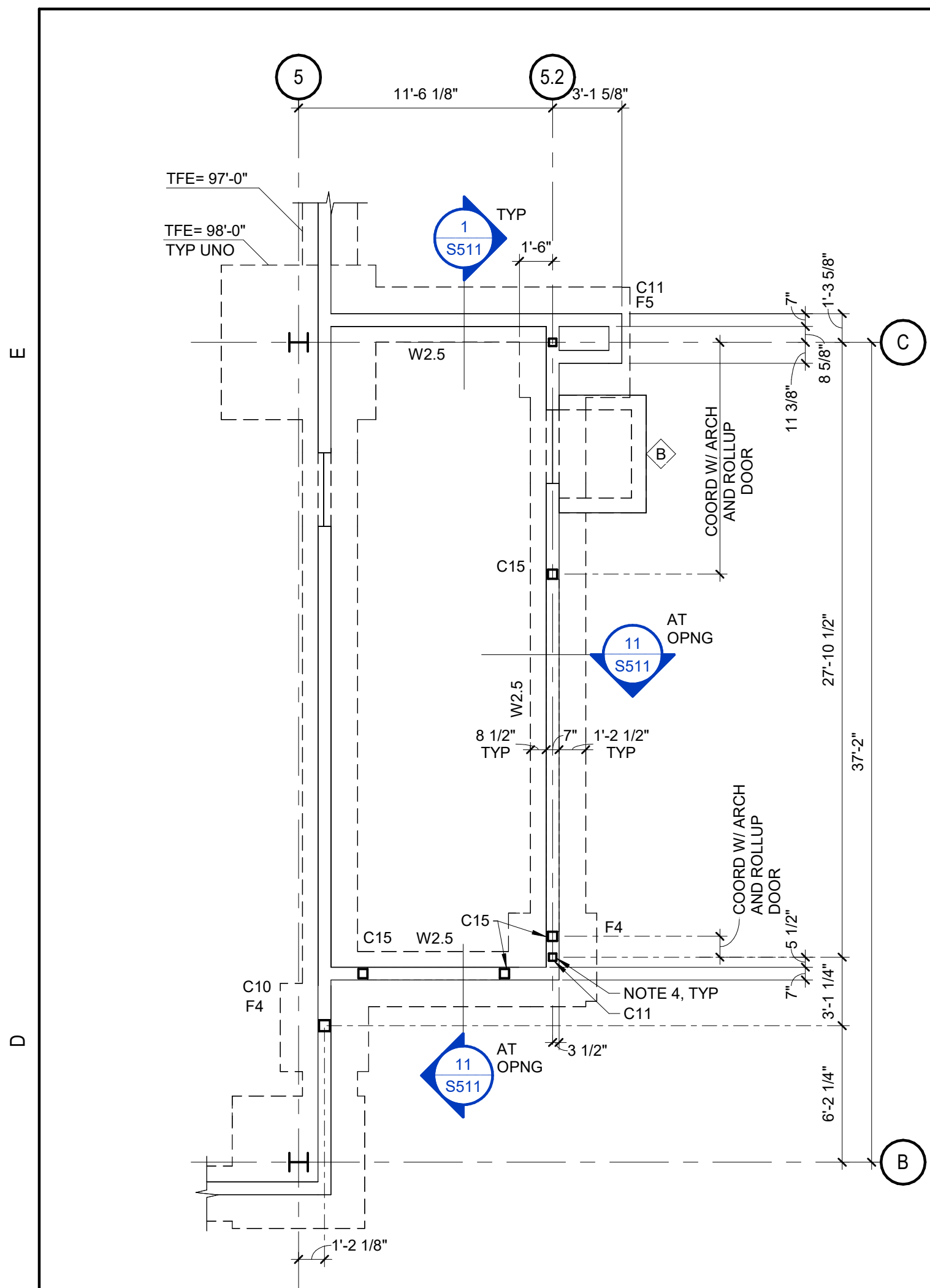
3/16" = 1'-0"

NOTES:

1. FOR GENERAL STRUCTURAL NOTES SEE SHEET S001.
2. FOR TYPICAL ROOF FRAMING DETAILS SEE SHEET SEE S611.

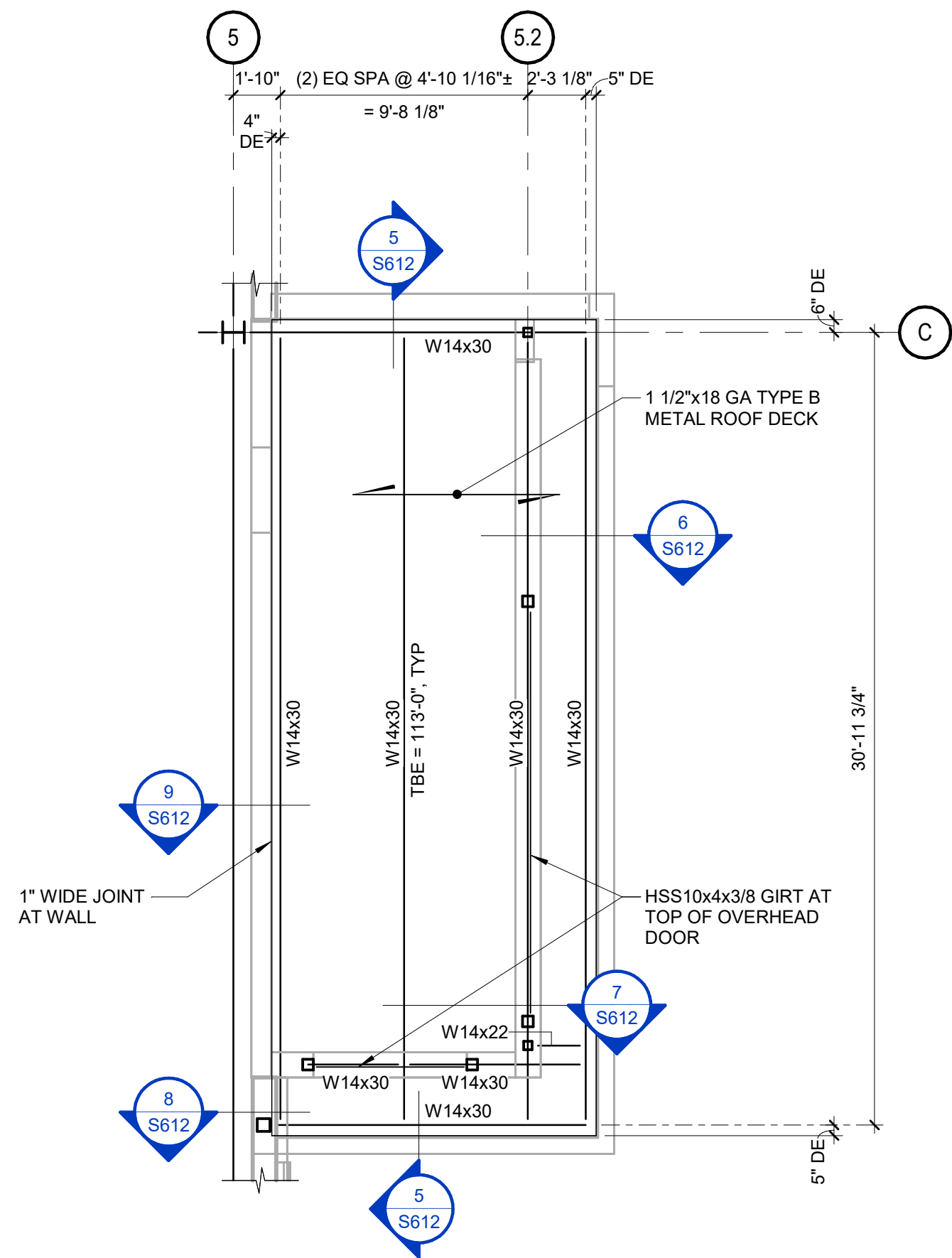
SHEET S103 FOR ADDITIONAL NOTES.





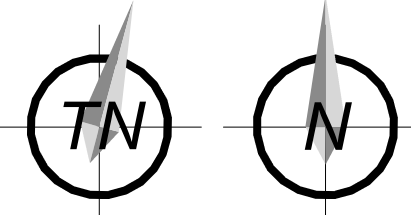
1 FOUNDATION PLAN AT BISTRO

- 3/16" = 1'-0"
- NOTES:
- FOR GENERAL STRUCTURAL NOTES SEE SHEET S001.
 - FOR TYPICAL CONCRETE DETAILS SEE SHEET S501 & S502.
 - SEE SHEET S101 FOR ADDITIONAL NOTES.
 - PROVIDE TEMPORARY BLOCKOUT IN FOUNDATION WALL TO INSTALL COLUMN AND BASE CONNECTION.

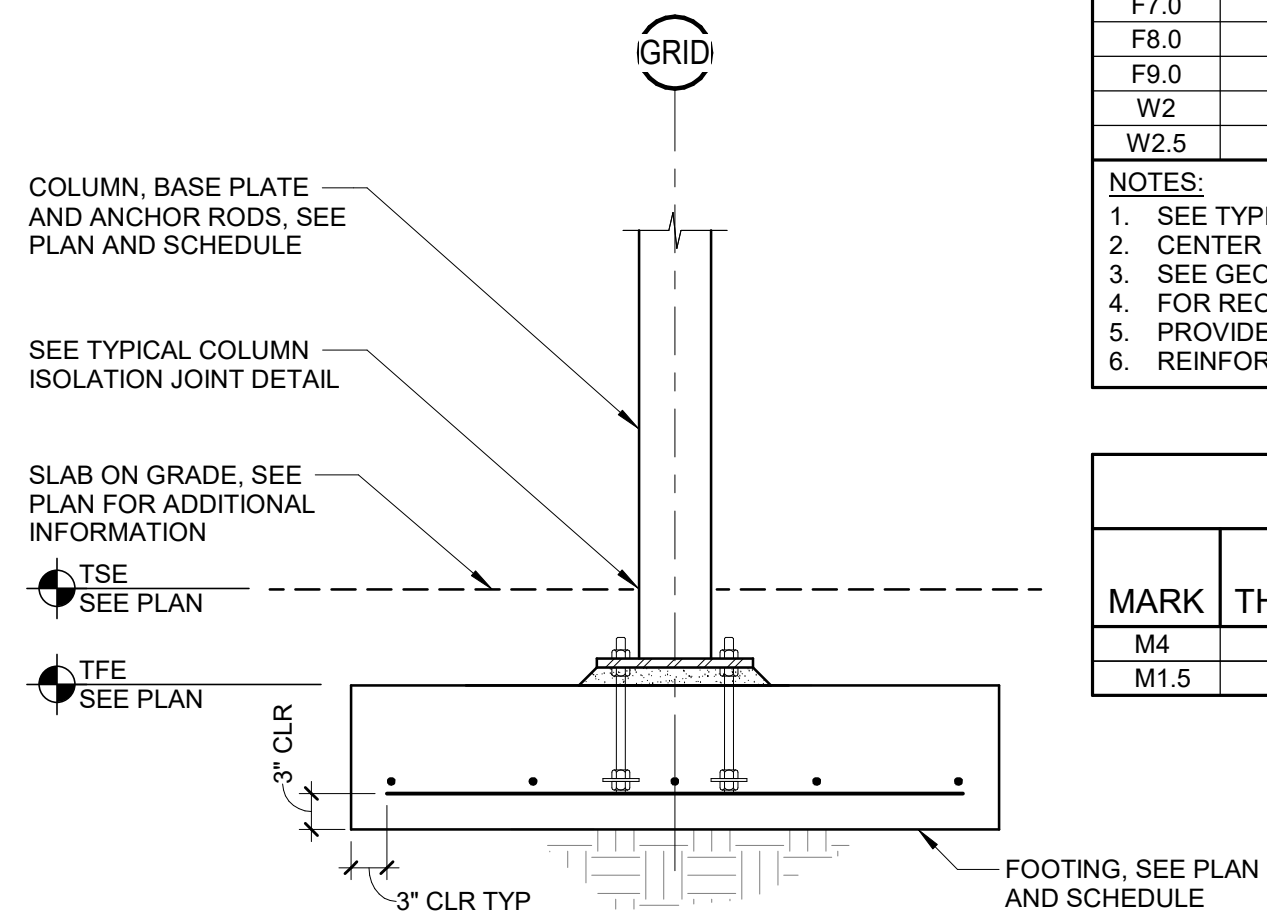


2 ROOF FRAMING PLAN AT BISTRO

- 3/16" = 1'-0"
- NOTES:
- FOR GENERAL STRUCTURAL NOTES SEE SHEET S001.
 - FOR TYPICAL ROOF FRAMING DETAILS SEE SHEET SEE S611. SHEET S103 FOR ADDITIONAL NOTES.



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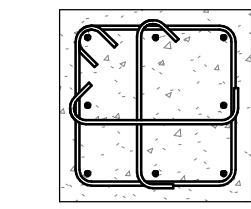
1
S501
3/4" = 1'-0"

TYPICAL FOOTING DETAIL

FOOTING SCHEDULE						
MARK	WIDTH	LENGTH	THICKNESS	REINF BARS		COMMENTS
				BOTTOM	TOP	
F3.0	3'-0"	3'-0"	12"	(4) #4 EW	-	
F4.0	4'-0"	4'-0"	14"	(8) #4 EW	-	
F5.0	5'-0"	5'-0"	18"	(8) #6 EW	-	
F6.0	6'-0"	6'-0"	20"	(8) #6 EW	-	
F7.0	7'-0"	7'-0"	24"	(10) #6 EW	-	
F8.0	8'-0"	8'-0"	26"	(14) #6 EW	-	
F9.0	9'-0"	9'-0"	30"	(10) #6 EW	-	
W2	2'-0"	CONT	12"	(3) #6 CONT	(3) #7 CONT	
W2.5	2'-6"	CONT	18"	(3) #7 CONT	(3) #7 CONT	

NOTES:
1. SEE TYPICAL FOOTING DETAILS FOR ADDITIONAL INFORMATION.
2. CENTER FOOTING ON COLUMN, PIER, OR WALL UNLESS NOTED OTHERWISE.
3. SEE GEOTECHNICAL REPORT FOR SUBGRADE REQUIREMENTS.
4. FOR RECTANGULAR FOOTINGS, LONGITUDINAL BARS ARE TO BE THE BOTTOM LAYER OF REINFORCING.
5. PROVIDE CORNER BARS AT CORNERS OF WALL FOOTINGS, SEE 6/S502.
6. REINFORCEMENT OF CONTINUOUS FOOTINGS SHALL BE CONTINUOUS THROUGH SPREAD/SPOT FOOTINGS.

MAT FOUNDATION SCHEDULE				
MARK	THICKNESS	DIMENSION	REINFORCING	COMMENTS
M4	4'-0"	SEE PLAN	SEE DETAIL 14/S502	
M1.5	1'-6"	SEE PLAN	#6 @ 12" OC EW, TOP & BOTTOM	



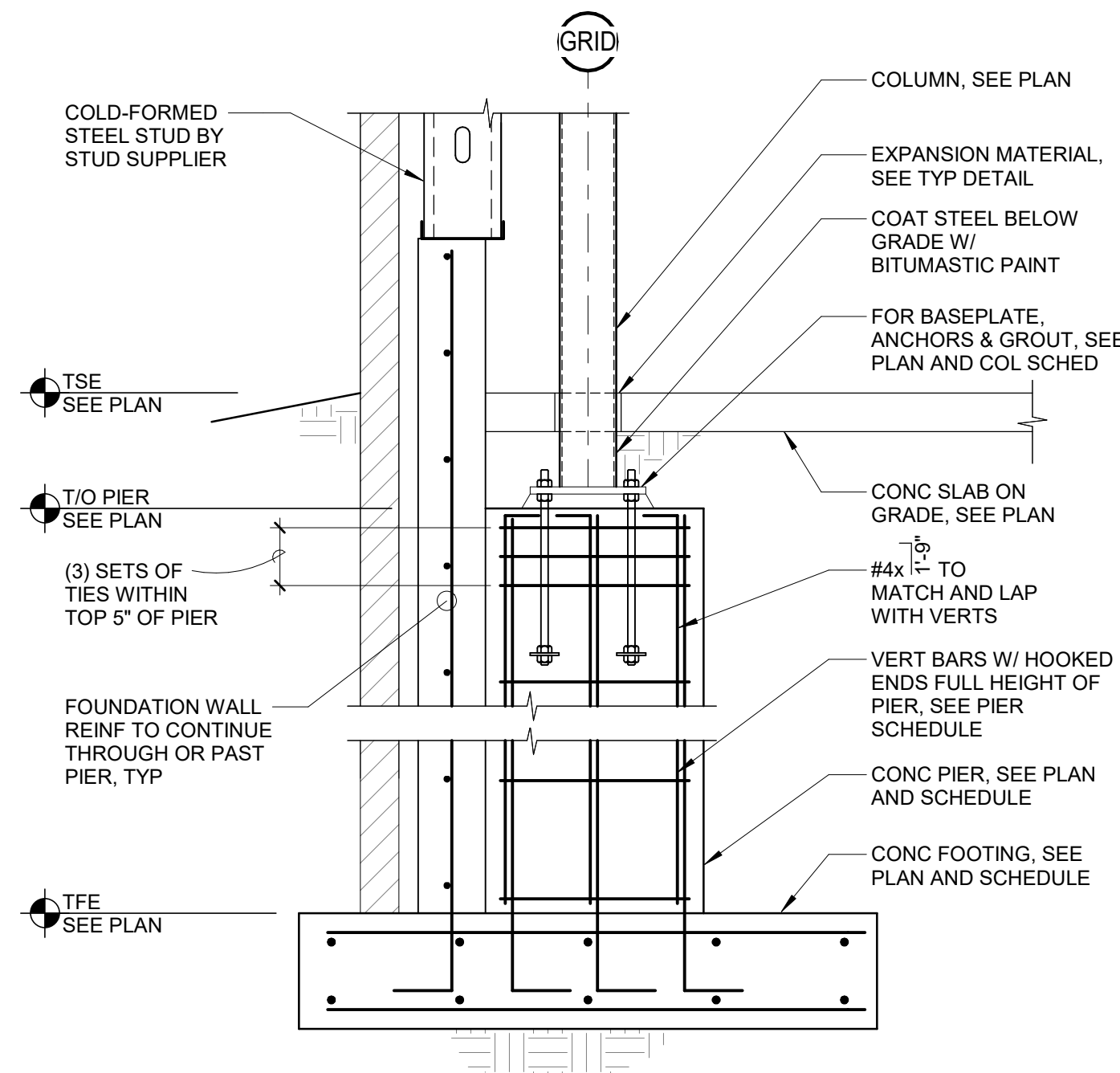
- NOTES:
1. IF CLEAR SPACING BETWEEN VERTICAL BARS IS GREATER THAN 8", PROVIDE ADDITIONAL CROSS TIES, SHOWN DASHED.
2. BARS ALONG EACH SIDE OF PIERS SHALL BE EQUALLY SPACED BETWEEN CORNER BARS.

2
S501
1/2" = 1'-0"

PIER SCHEDULE AND BAR/TIE LAYOUT DETAIL

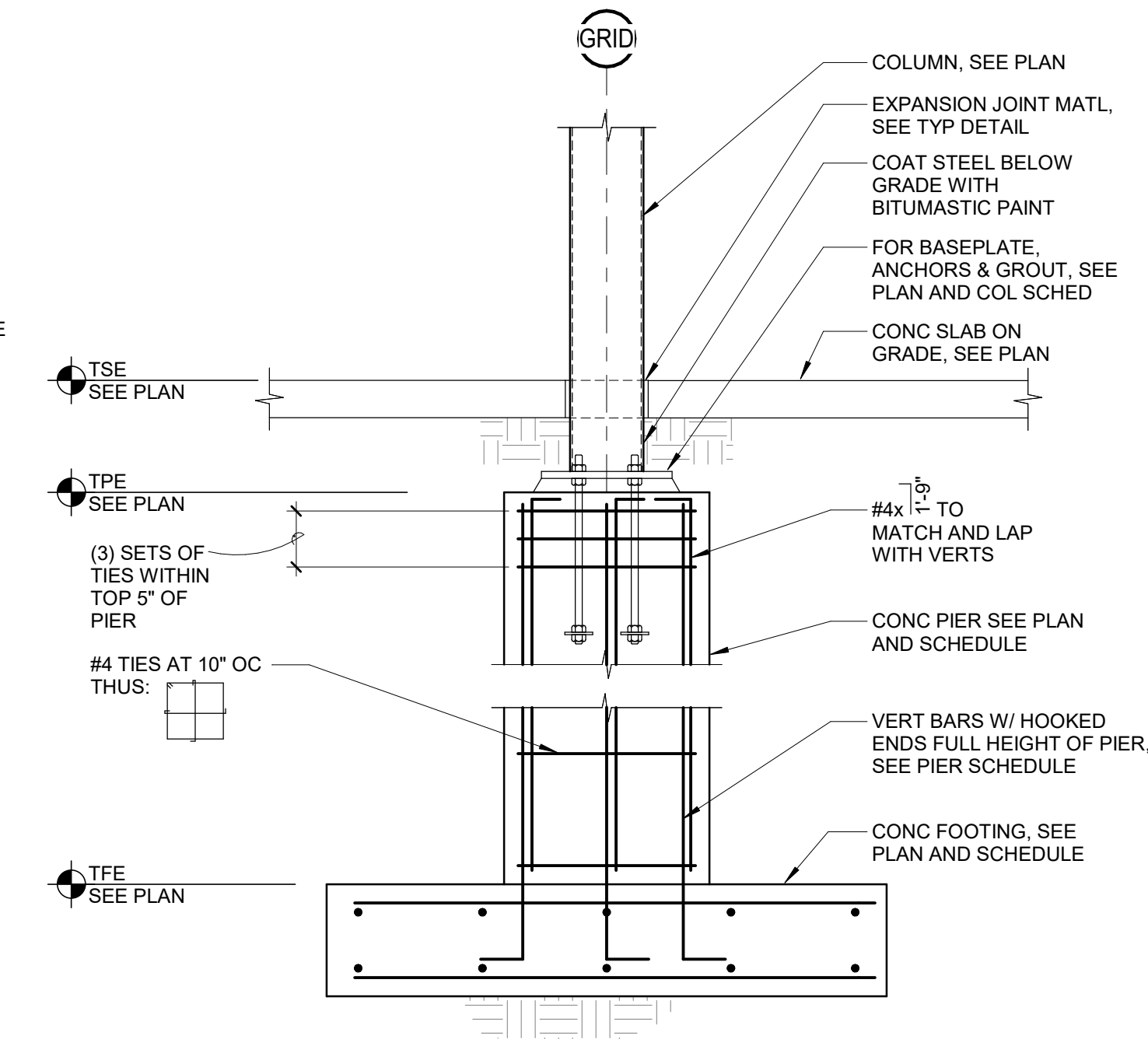
PIER SCHEDULE					
MARK	SIZE	REINFORCING		PIER TYPE	COMMENTS
		VERT	TIES		
P1	2'-0x2'-0"	(8) #8	#4 @ 12" OC	8A	
P2	2'-4x2'-4"	(8) #8	#4 @ 12" OC	8A	

NOTES:
1. SEE TYPICAL COLUMN, PIER AND FOOTING DETAIL FOR ADDITIONAL INFORMATION.
2. SEE TYPICAL CONCRETE PIER BAR AND TIE LAYOUT FOR REINFORCING CONFIGURATIONS.
3. PROVIDE STANDARD 90 DEGREE HOOK AT VERTICAL (VERT) BARS TO FOOTING.
4. CONTRACTORS OPTION PROVIDE HOOKED DOWELS LAP SPliced TO VERTICAL REINFORCING MATCH SIZE AND LOCATION OF VERTICAL REINFORCING.



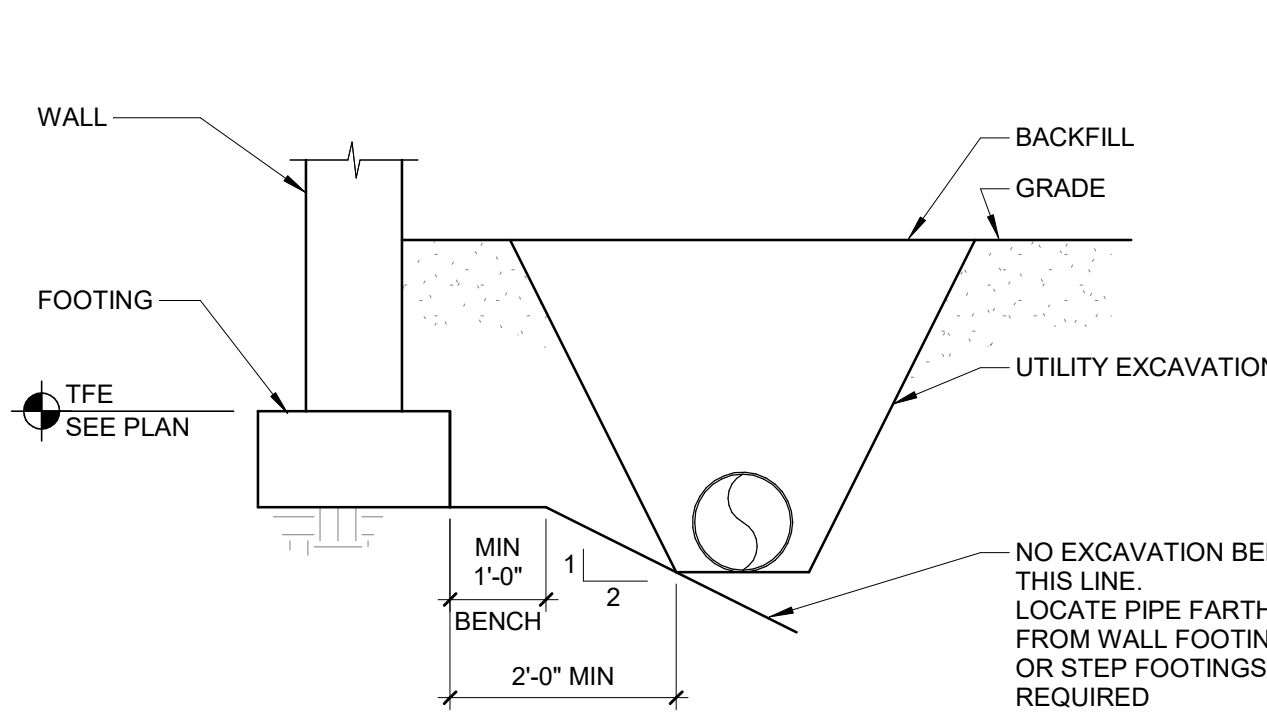
3
S501
3/4" = 1'-0"

TYP PIER DETAIL @ EXTERIOR WALLS



4
S501
3/4" = 1'-0"

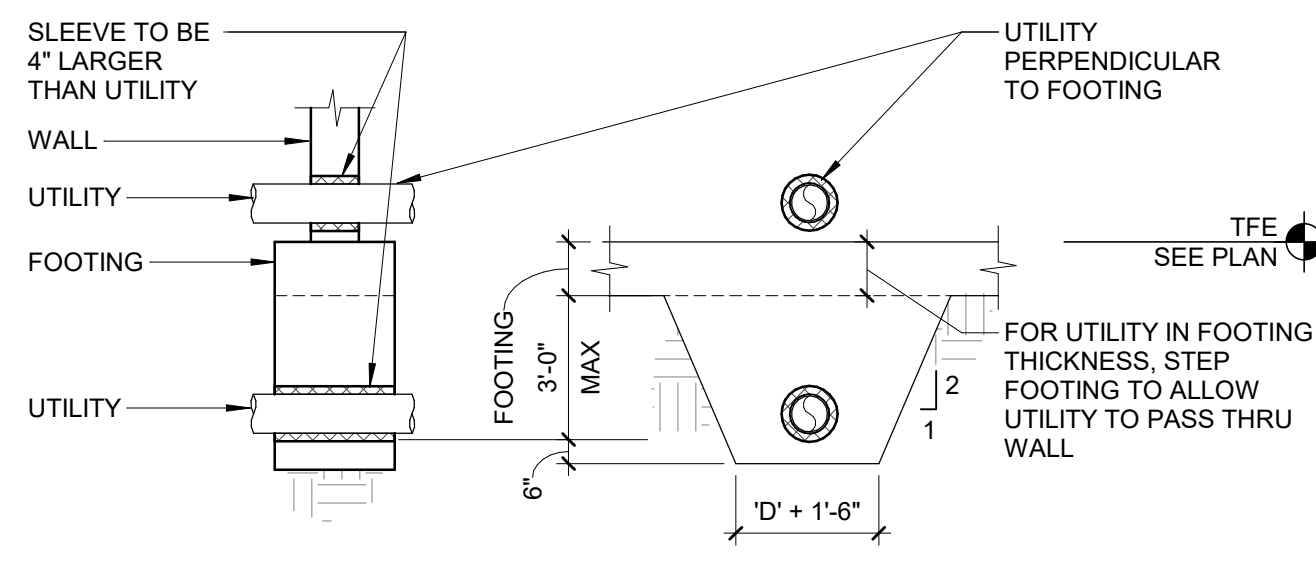
TYP PIER DETAIL @ BLDG INTERIOR



- NOTES:
1. CONTRACTOR SHALL BE RESPONSIBLE FOR SHORING, SHEATHING OR OTHERWISE MAINTAINING THE SIDES OF THE EXCAVATION FROM CAVE-IN UNTIL ALL BACKFILL IS COMPLETED.

5
S501
NO SCALE

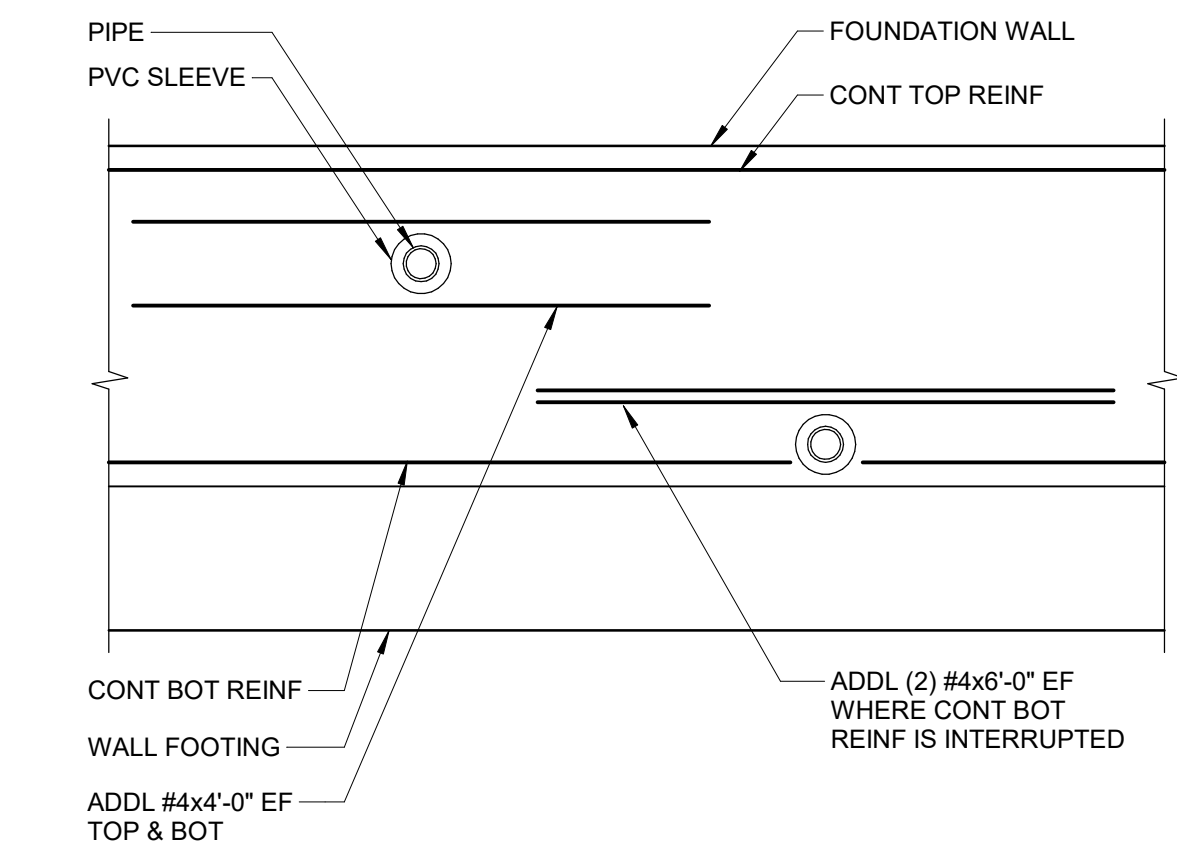
TYPICAL FOOTING PARALLEL TO UNDERGROUND UTILITIES DETAIL



- NOTES:
1. FOR PIPES 3'-0" OR LESS BELOW FOOTING PROVIDE SLEEVE AND CONCRETE AS SHOWN.
2. FOR PIPES MORE THAN 3'-0" BELOW FOOTING BUT LESS THAN 8'-0", USE STEPPED FOOTING DETAIL TO GET WITHIN 3'-0" MAX.
3. FOR PIPES 8'-0" OR MORE BELOW FOOTING, CONCRETE FILL IS NOT REQUIRED. PLACE UTILITY IN TRENCH WITH SOIL COMPACTED PER SPEC REQUIREMENTS.
4. WRAP PIPE AT WALL / FOOTING PENETRATION WITH MINIMUM 2" COMPRESSIBLE MATERIAL BETWEEN SLEEVE AND UTILITY.
5. AT CONTRACTOR'S OPTION, FILL PIPING TRENCH BELOW FOOTING W/ LEAN CONCRETE AND PROVIDE SLEEVE AND COMPRESSIBLE MATERIAL AS NOTED ABOVE.

6
S501
NO SCALE

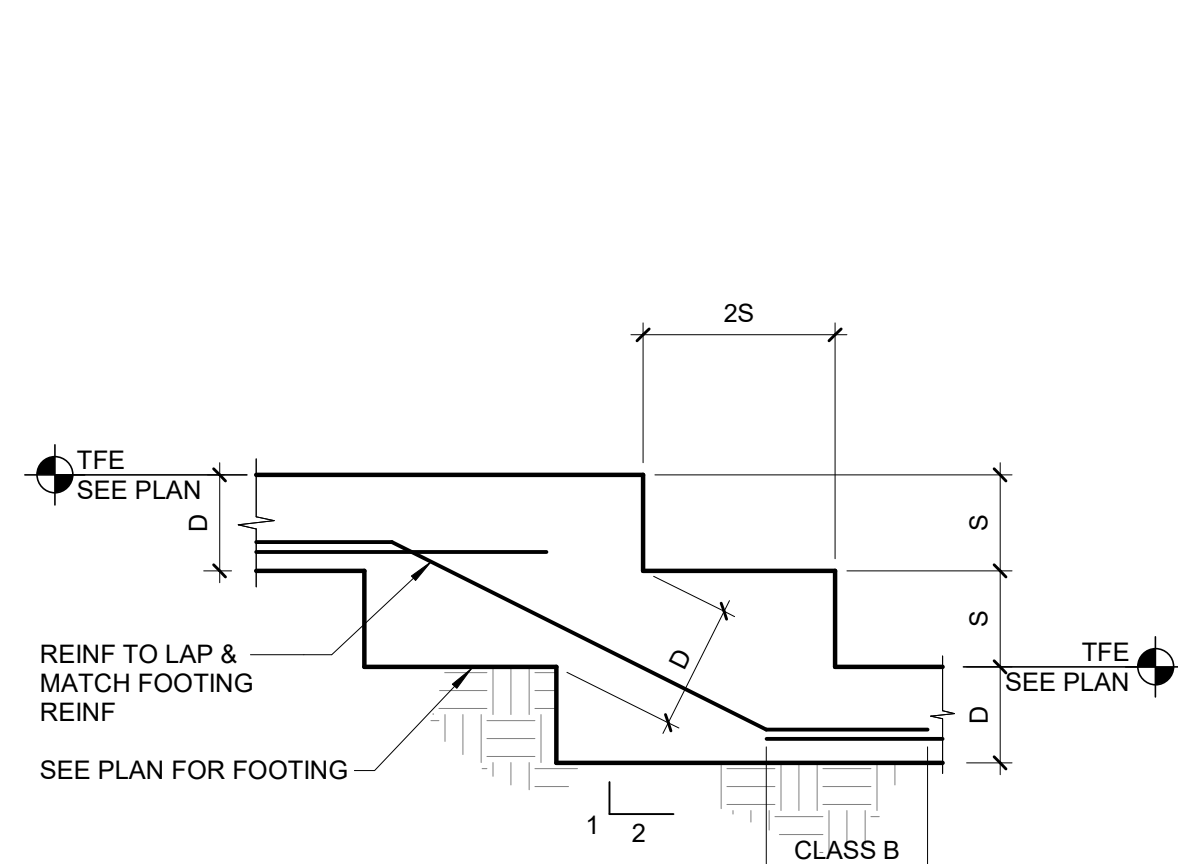
TYPICAL FOOTING PERPENDICULAR TO UNDERGROUND UTILITIES DETAIL



- NOTES:
1. SEE MEIP AND SITE DRAWINGS FOR UTILITY AND MECHANICAL PIPE LOCATIONS AND ELEVATIONS.
2. TYPICAL FOOTING AND WALL REINF NOT SHOWN FOR CLARITY.
3. SLEEVES TO BE 2" LARGER IN DIA THAN OD OF PIPES.
4. CONTACT STRUCTURAL ENGINEER FOR SLEEVES GREATER THAN 12" DIA.

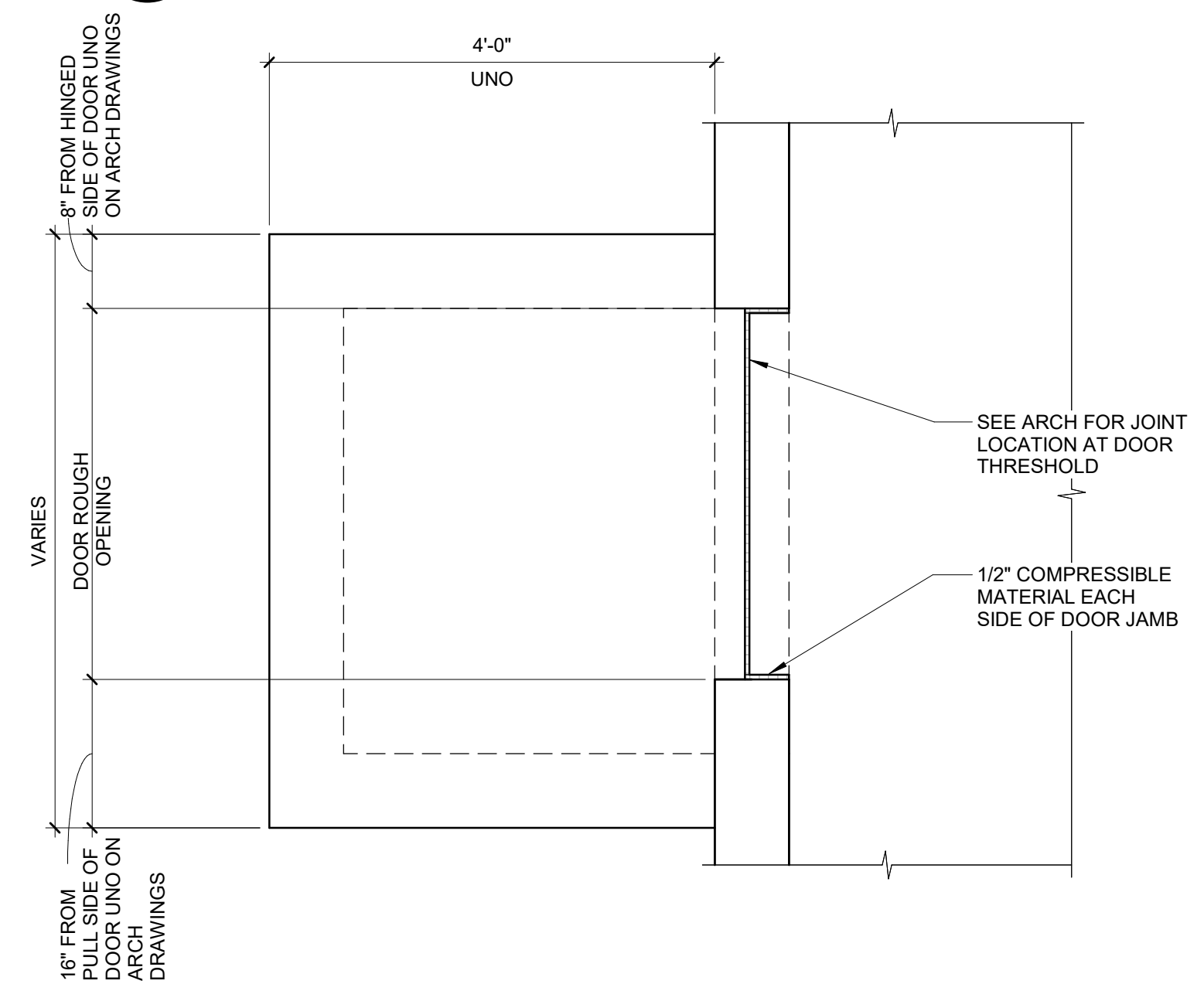
7
S501
NO SCALE

TYP PIPE SLEEVES AT FDN WALLS



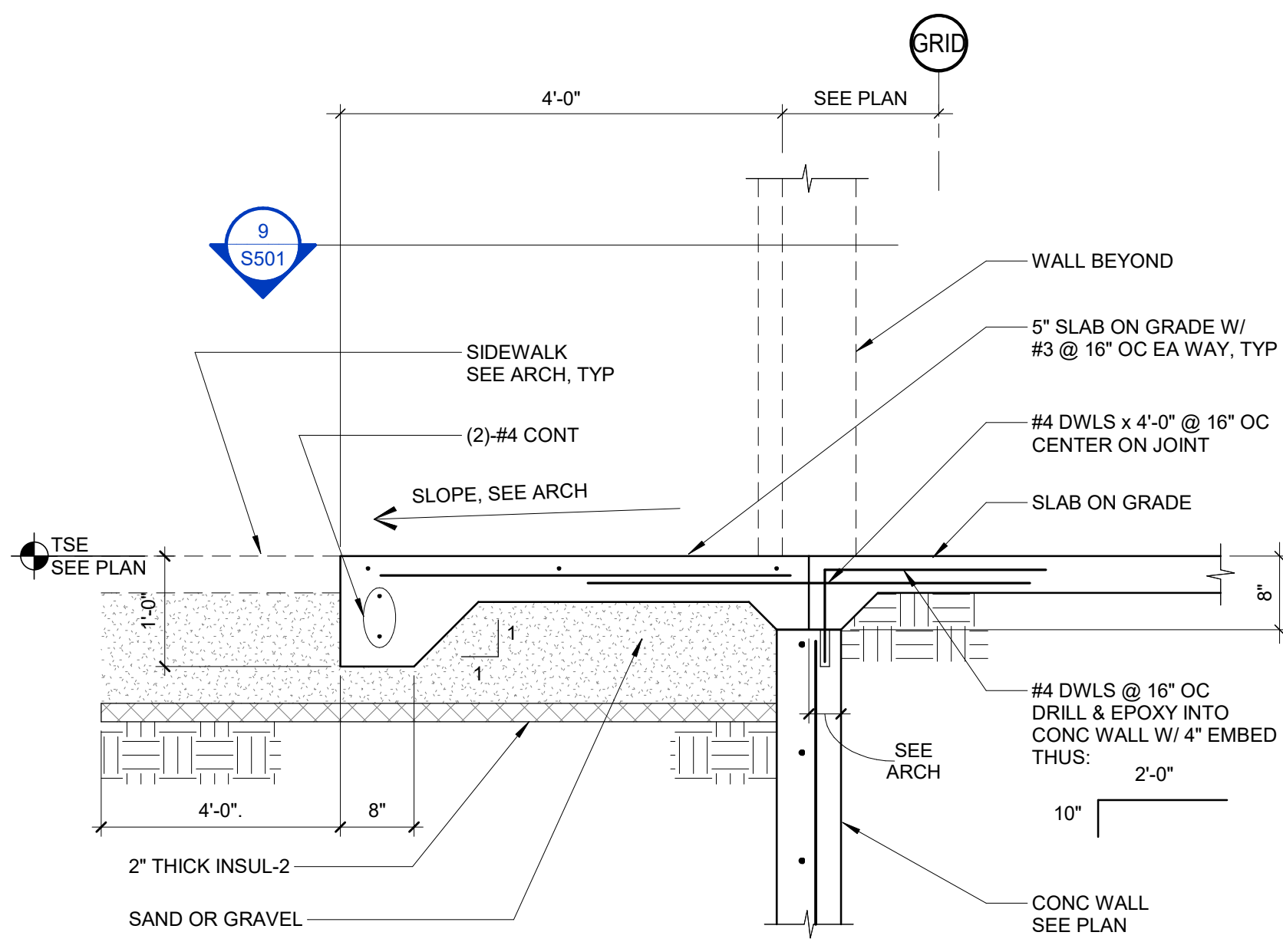
8
S501
1/2" = 1'-0"

TYP STEPPED FOOTING DETAIL AT CONC OR CMU WALL

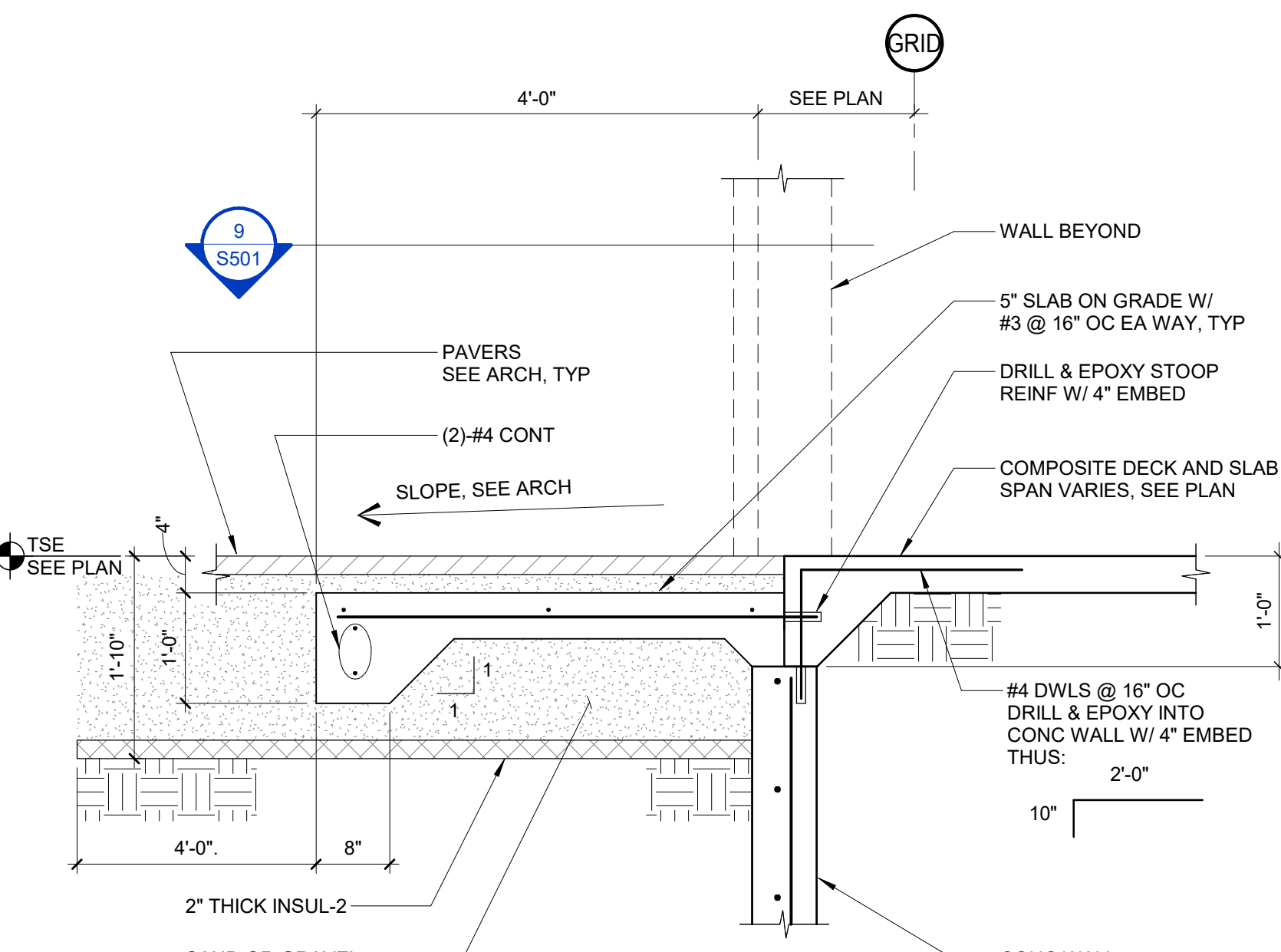


9
S501
3/4" = 1'-0"

PLAN DETAIL AT EXTERIOR DOORWAYS



TYPICAL STOOP DETAIL AT SLAB ON GRADE



TYPICAL STOOP DETAIL W/ PAVERS AT SLAB ON GRADE

10
S501
3/4" = 1'-0"

TYPICAL STOOP DETAILS

LIFE TIME
HEALTHY WAY OF LIFE
2900 CORPORATE PLACE CHANHASSEN, MN 55317
CONSULTANT
LIFE TIME
PROPERTY DEVELOPMENT

VAA
VAA, LLC
2200 Seneca Lane N., Suite 200
Plymouth, MN 55441
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www.vaaorg.com

PROJECT INFORMATION

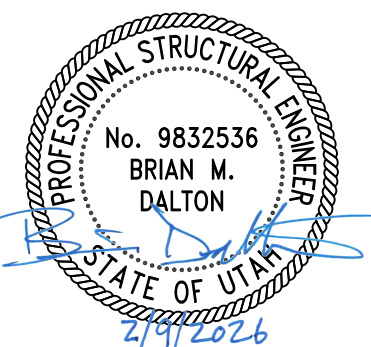
Holladay, UT
1975 East Rodeo Drive
Holladay, Utah

DRAWING LOG

REV DATE DESCRIPTION

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ISSUE DATE: 02/09/26

SHEET NO.

S501

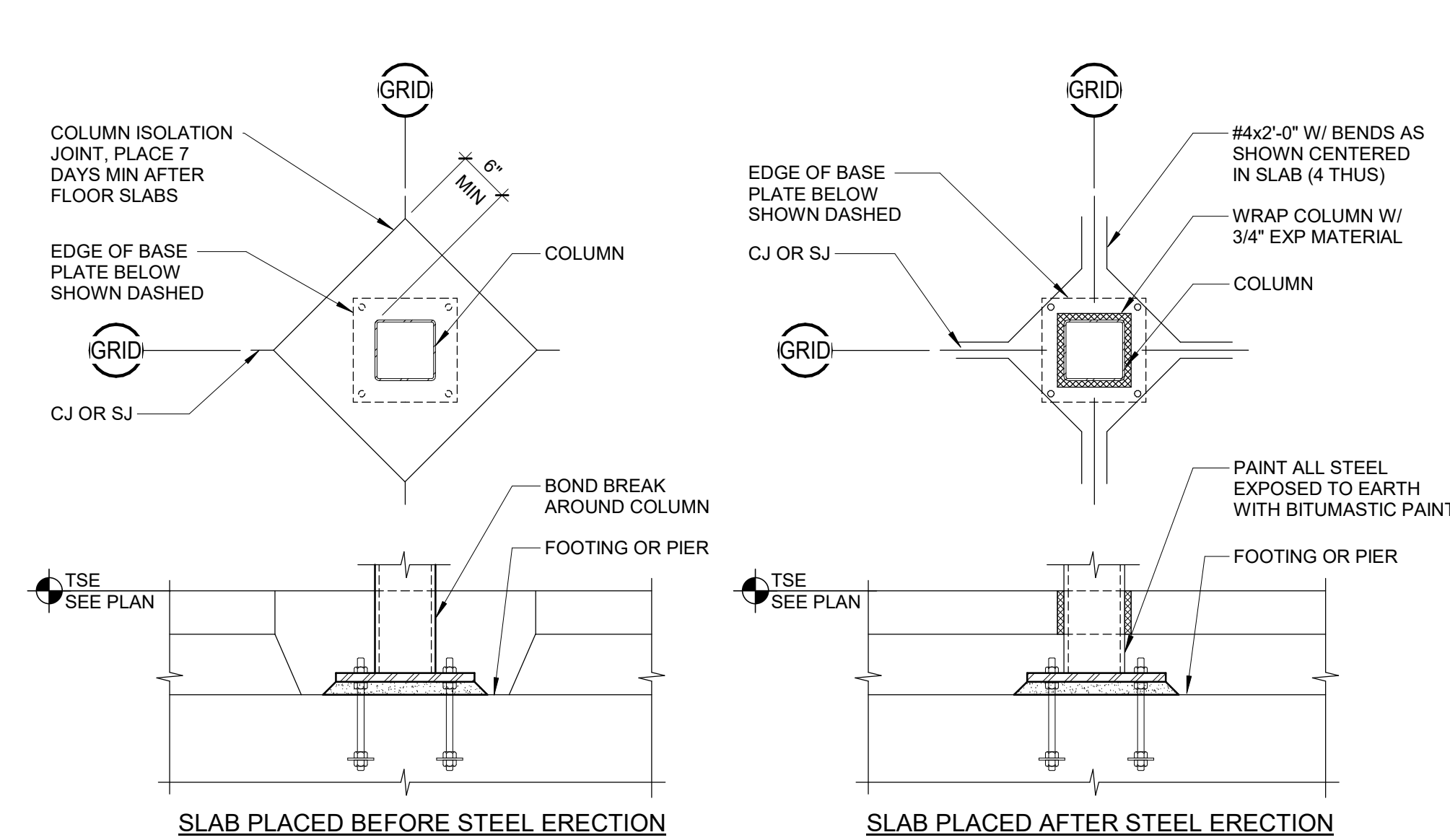
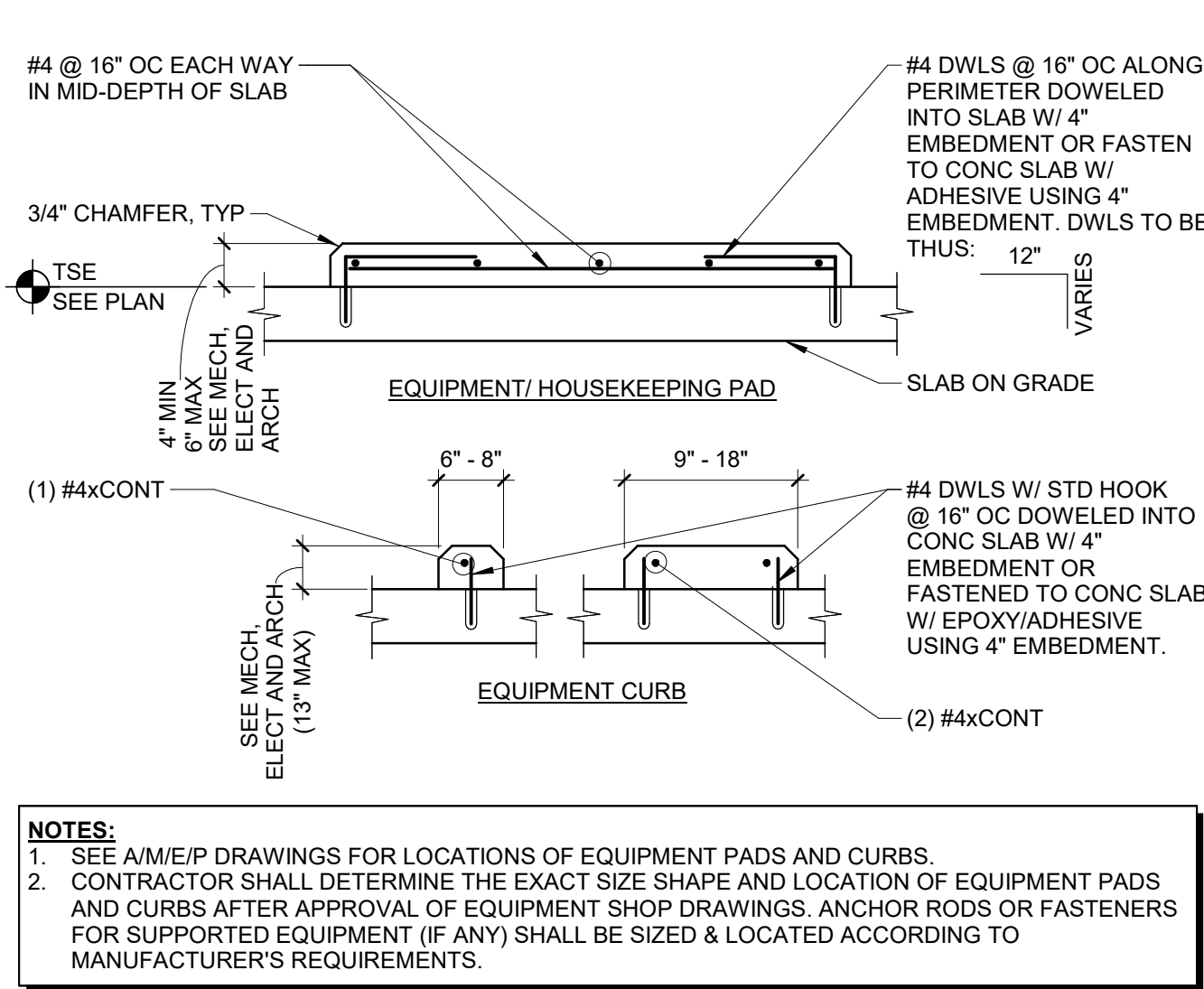
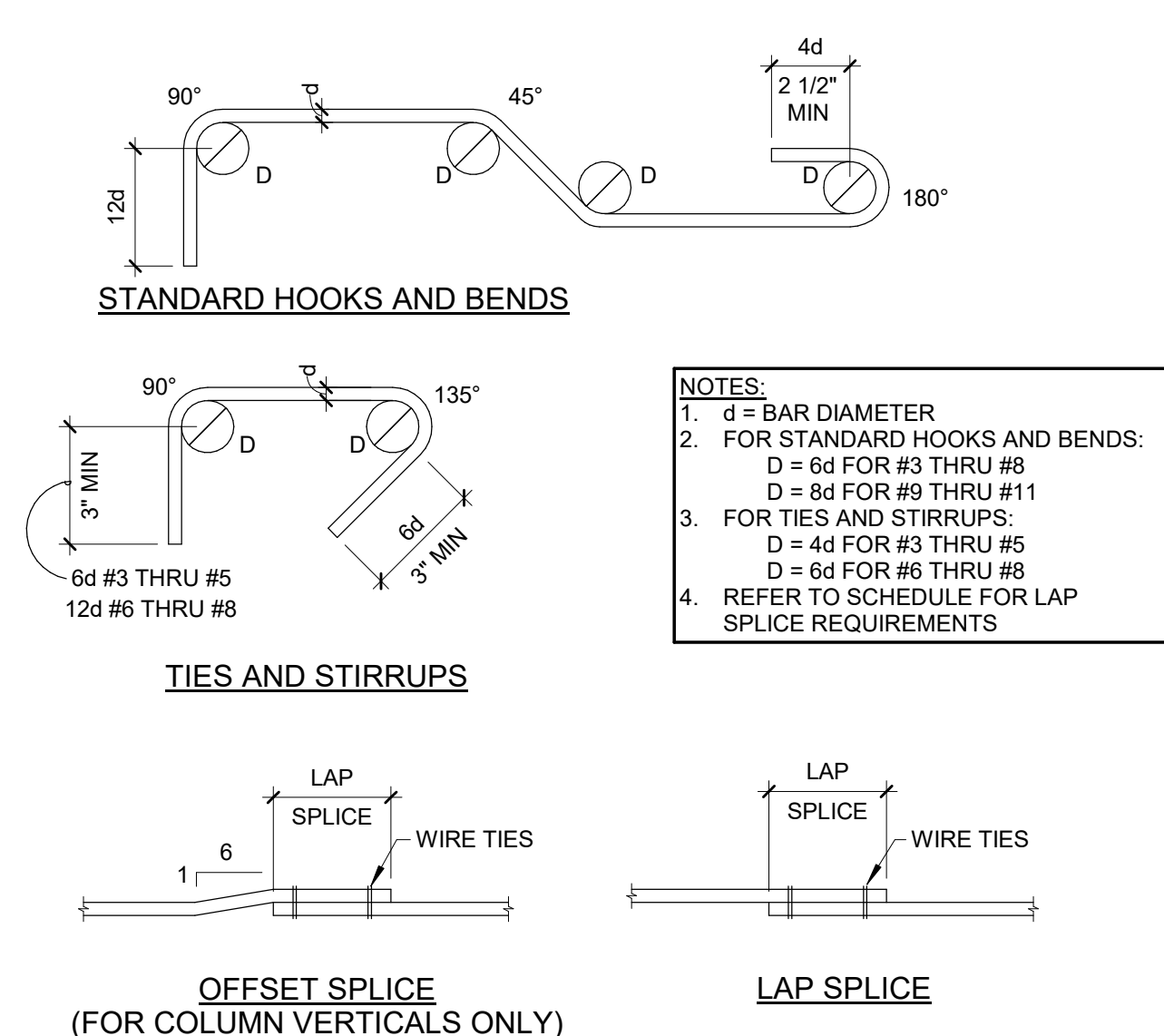
PUBLICATIONS
ISSUE FOR PERMIT/BID

TYPICAL FOUNDATION DETAILS

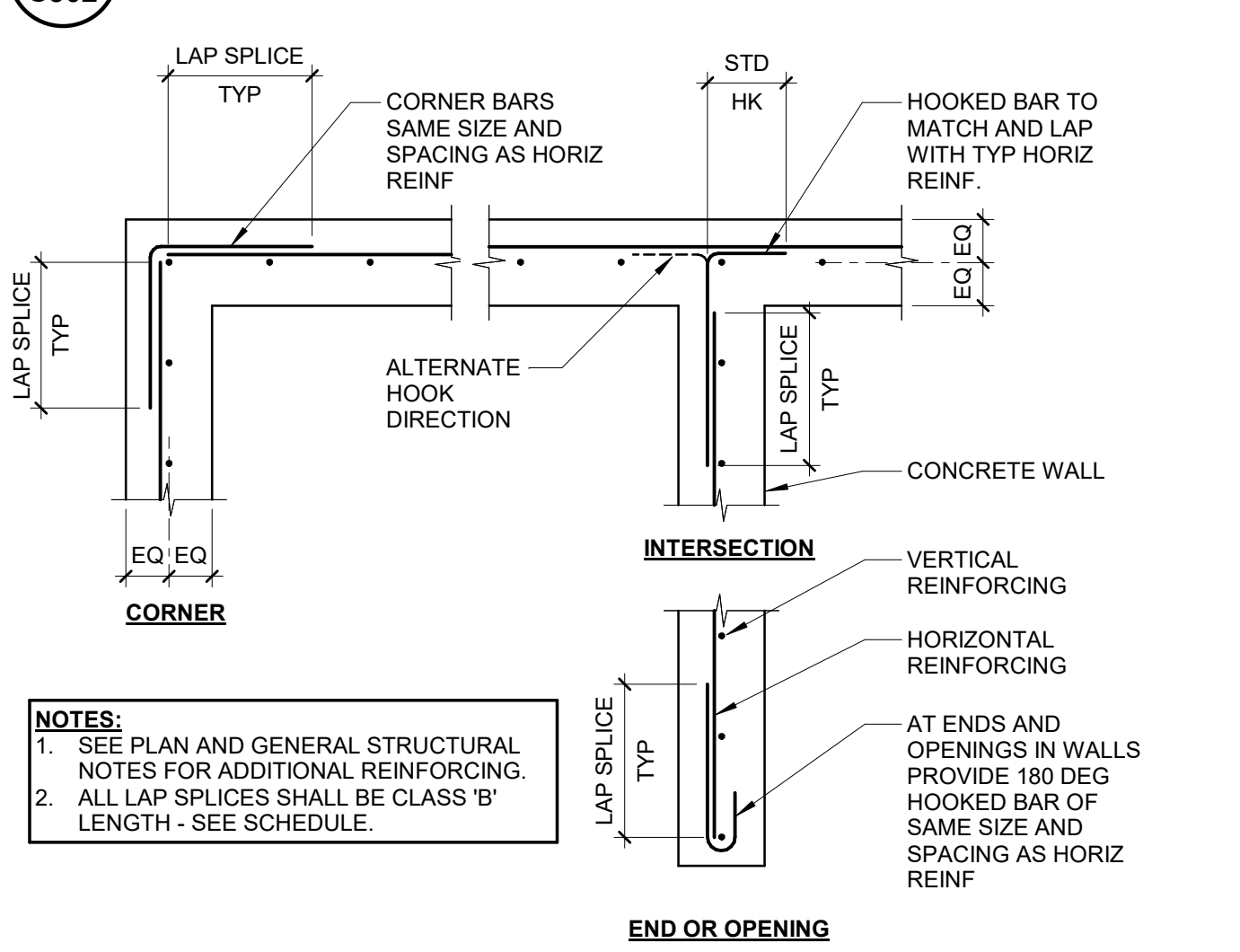
CONCRETE REINFORCING LAP SPlice AND EMBEDMENT OF STANDARD HOOKS SCHEDULE							
BAR SIZE	f'c = 3000 psi	f'c = 4000 psi	f'c = 5000 psi	f'c = 5000 psi	f'c = 5000 psi	f'c = 6000 psi	f'c = 6000 psi
	SPlice LENGTH	EMBED L _{SH}	EMBED L _{SH}	SPlice LENGTH	EMBED L _{SH}	SPlice LENGTH	EMBED L _{SH}
#3	22"	9"	19"	8"	17"	16"	6"
#4	29"	11"	25"	10"	23"	9"	8"
#5	36"	14"	31"	12"	28"	11"	10"
#6	43"	17"	37"	15"	34"	13"	12"
#7	63"	20"	54"	17"	49"	15"	14"
#8	72"	22"	62"	19"	56"	17"	16"
#9	81"	25"	70"	22"	63"	20"	18"
#10	91"	28"	79"	25"	71"	22"	20"
#11	101"	31"	87"	27"	78"	24"	22"

NOTES:
1. SCHEDULE APPLIES TO UNCOATED, GRADE 60 REINFORCING BARS IN NORMAL WEIGHT CONCRETE.
2. FOR EPOXY COATED BAR, MULTIPLY THE LAP SPlice LENGTHS BY 1.5 AND EMBEDS BY 1.2.
3. FOR LIGHT WEIGHT CONCRETE, MULTIPLY THE ABOVE VALUES BY 1.33.
4. FOR TOP BAR SPlices WHERE 12" OR MORE OF FRESH CONCRETE IS CAST BELOW THE SPlice, MULTIPLY THE LENGTHS SHOWN BY 1.3.
5. LAP SPlice LENGTHS APPLY TO REINFORCING BARS WITH:
A. CLEAR SPACING OF BARS GREATER OR EQUAL TO (2) x BAR DIAMETER (D_b)
B. CLEAR COVER GREATER THAN D_b
C. FOR ALL OTHER CASES, MULTIPLY THE LENGTHS BY 1.5.

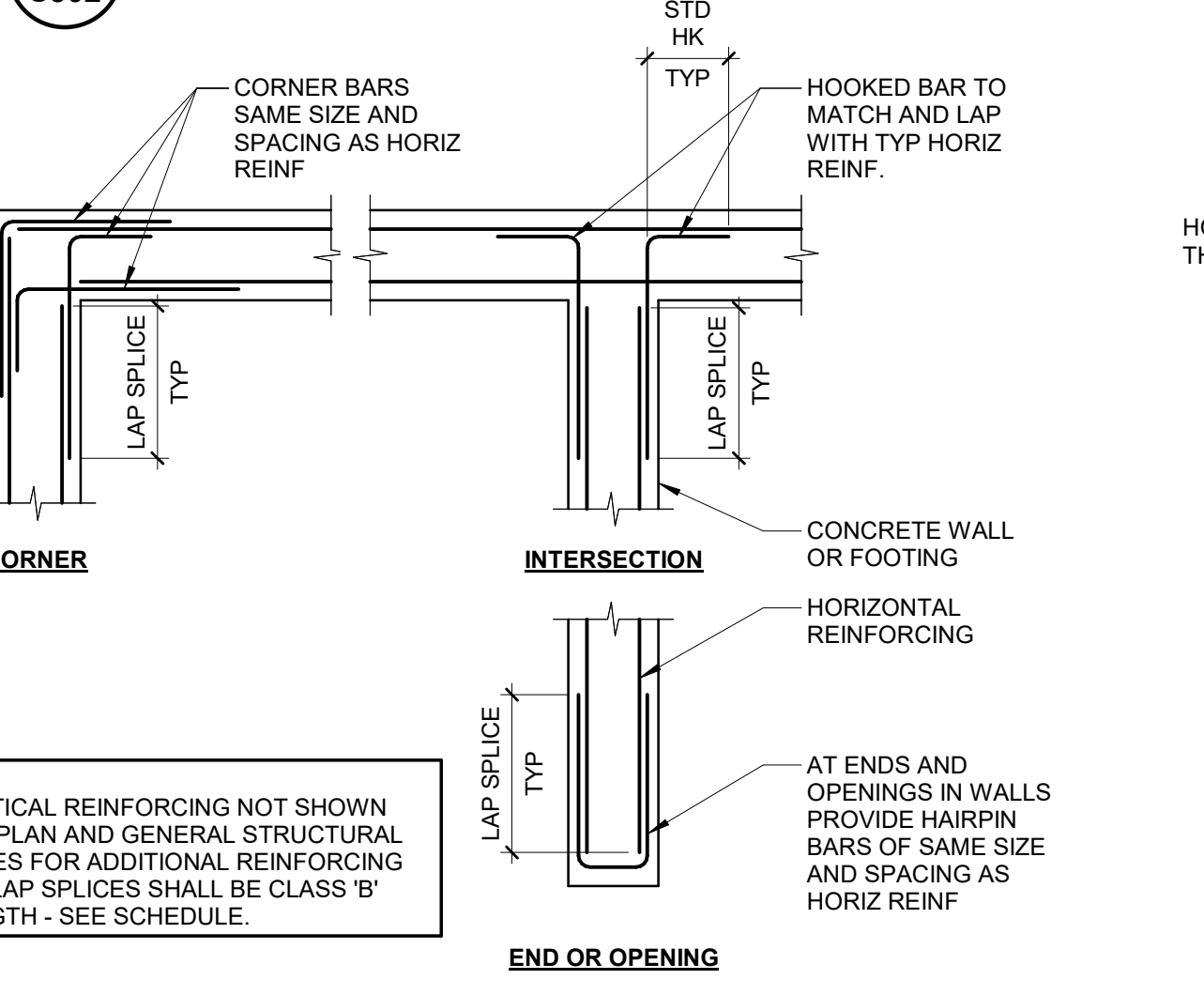
STD 90 DEG HK
CONCRETE SURFACE



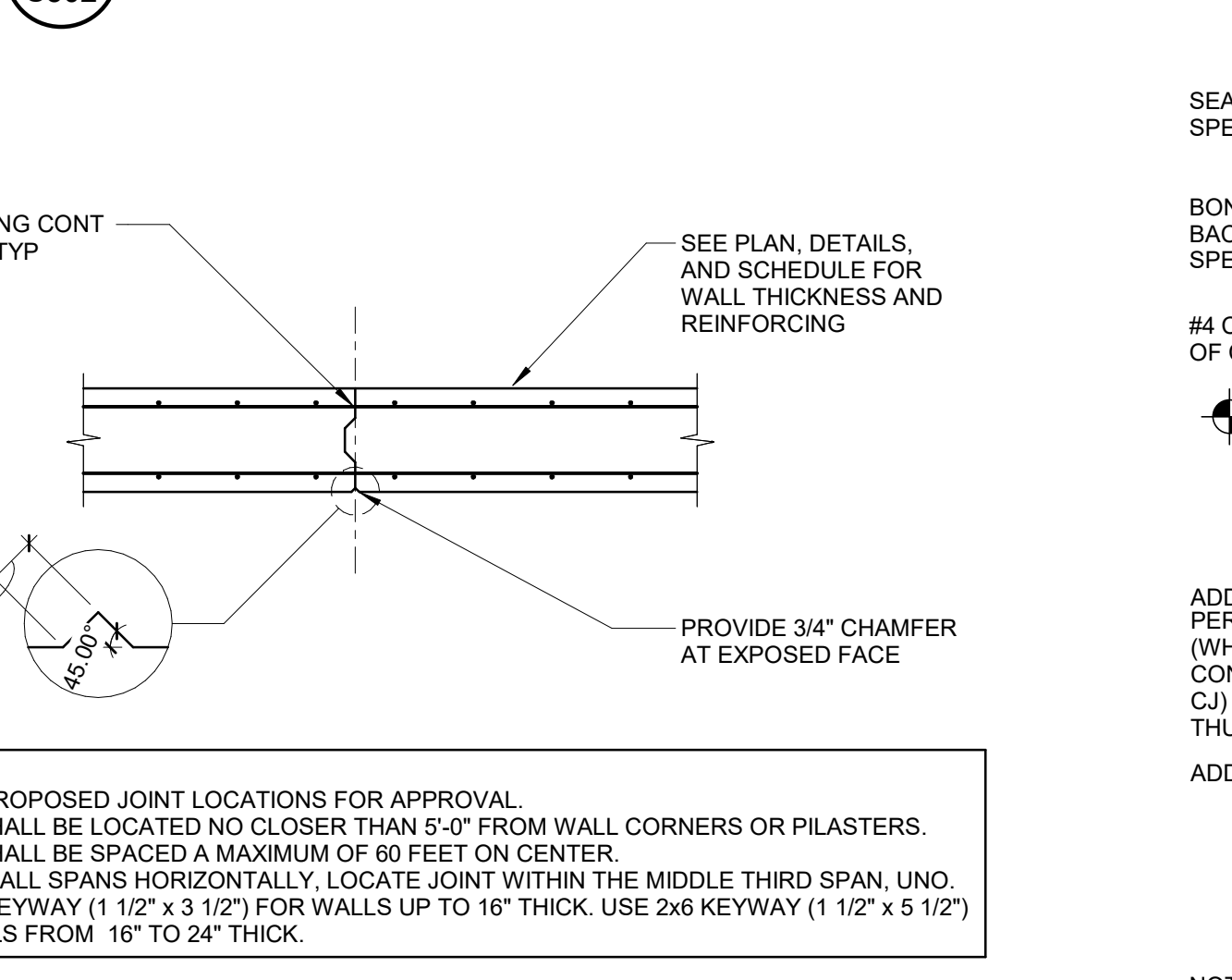
1 CONCRETE REINFORCING SCHEDULE



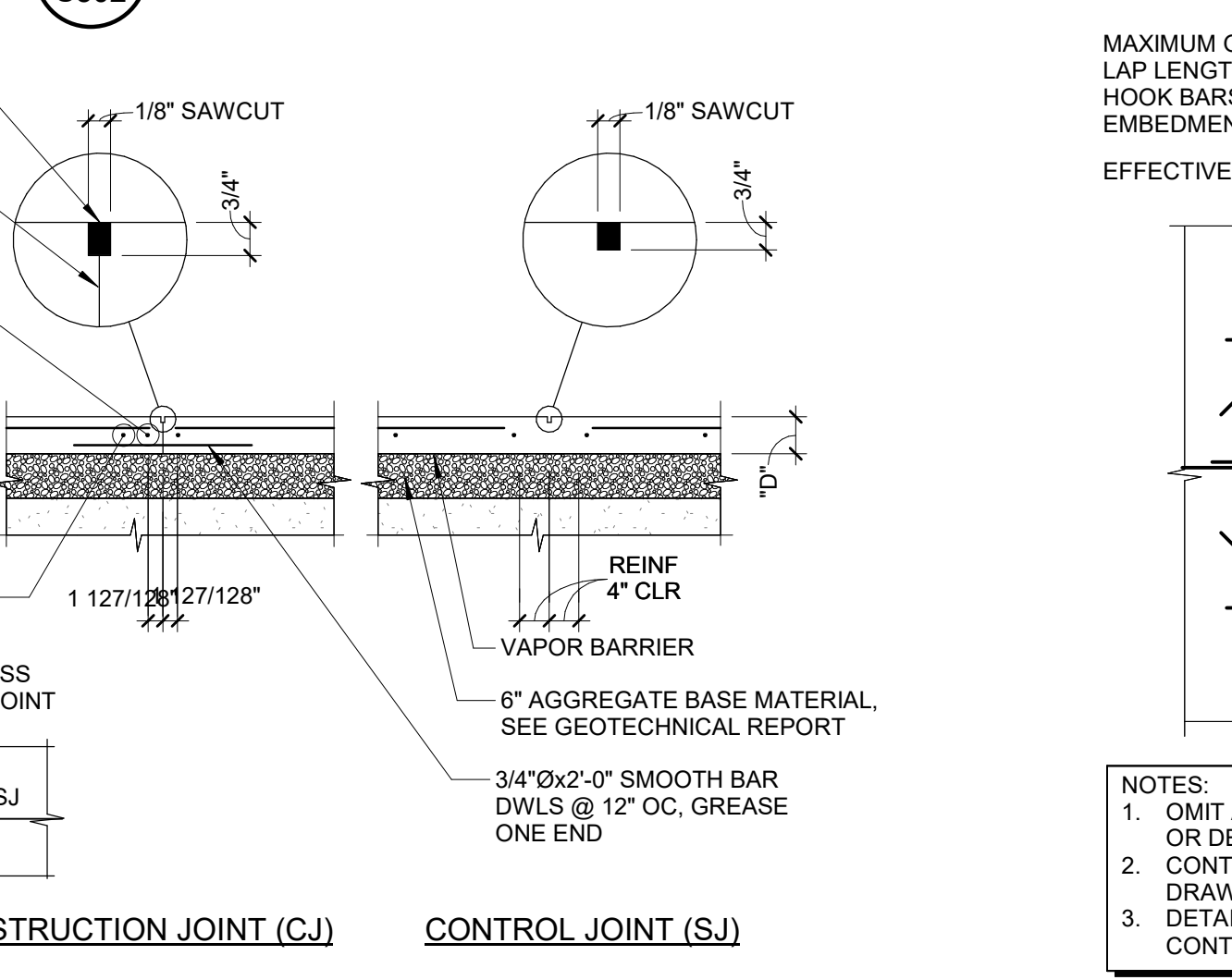
2 TYPICAL BAR BENDING DETAIL



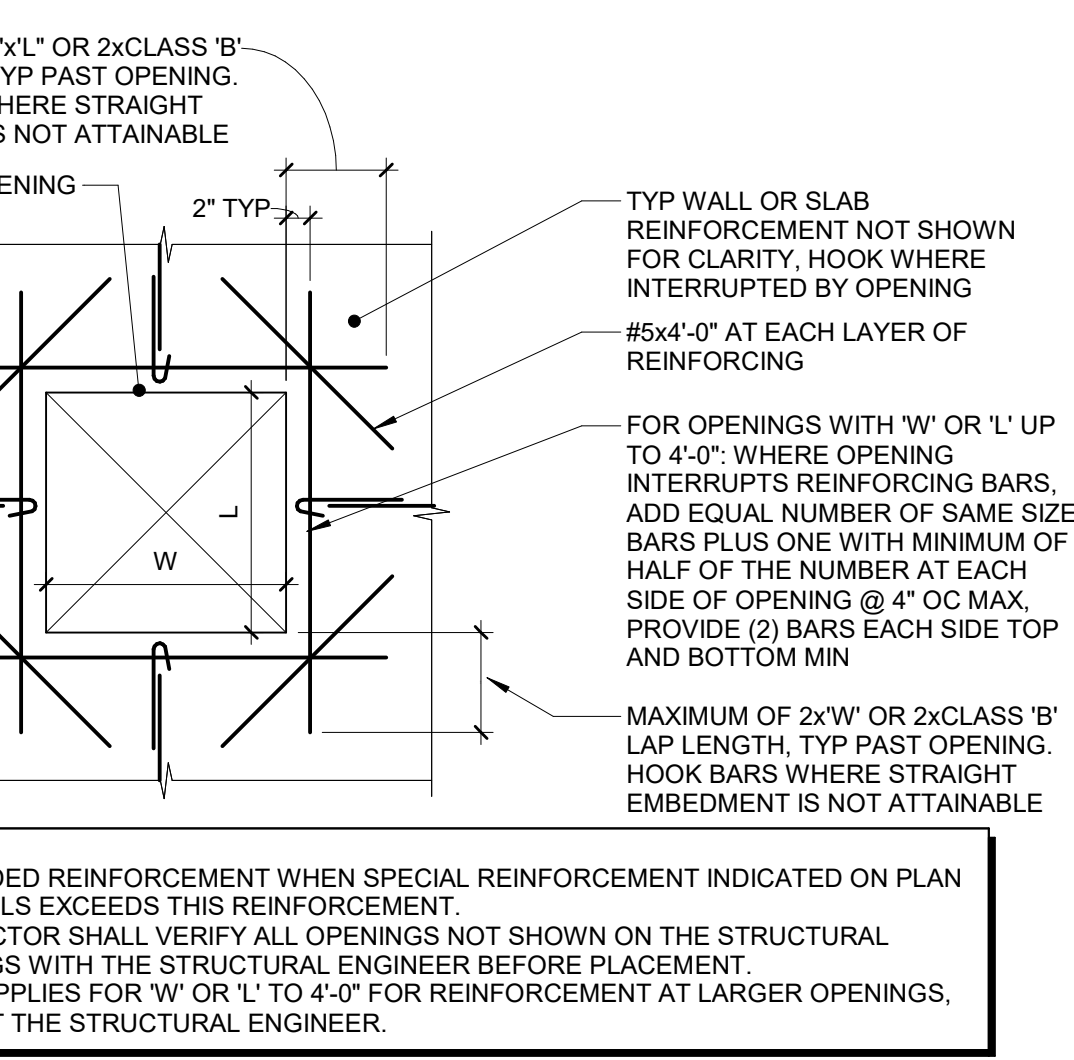
3 INTERIOR EQUIPMENT PAD OR CURB



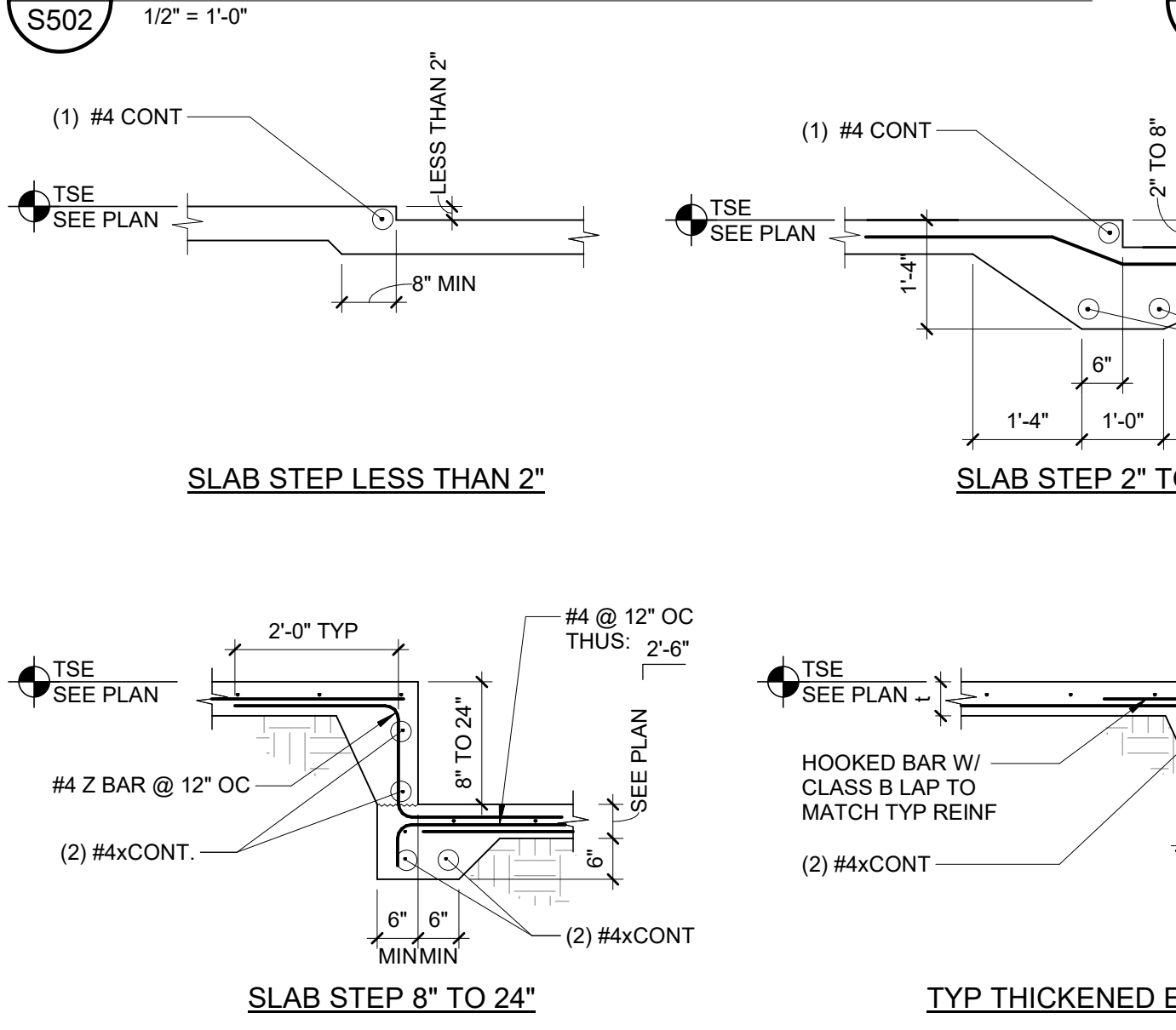
4 TYPICAL COLUMN ISOLATION JOINT



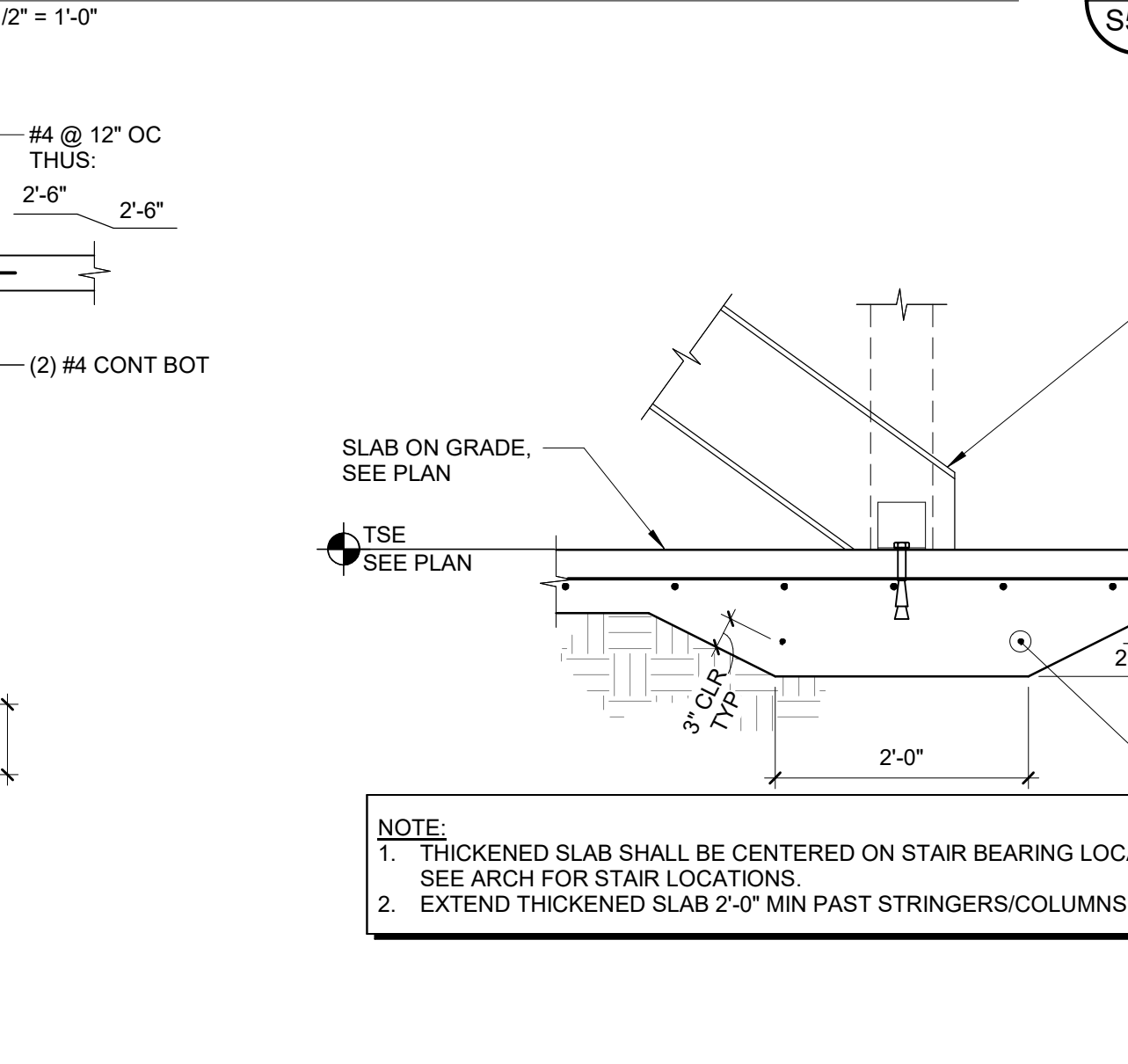
5 TYP CONC WALL AND STRUCTURAL SLAB OPENING REINF DETAIL



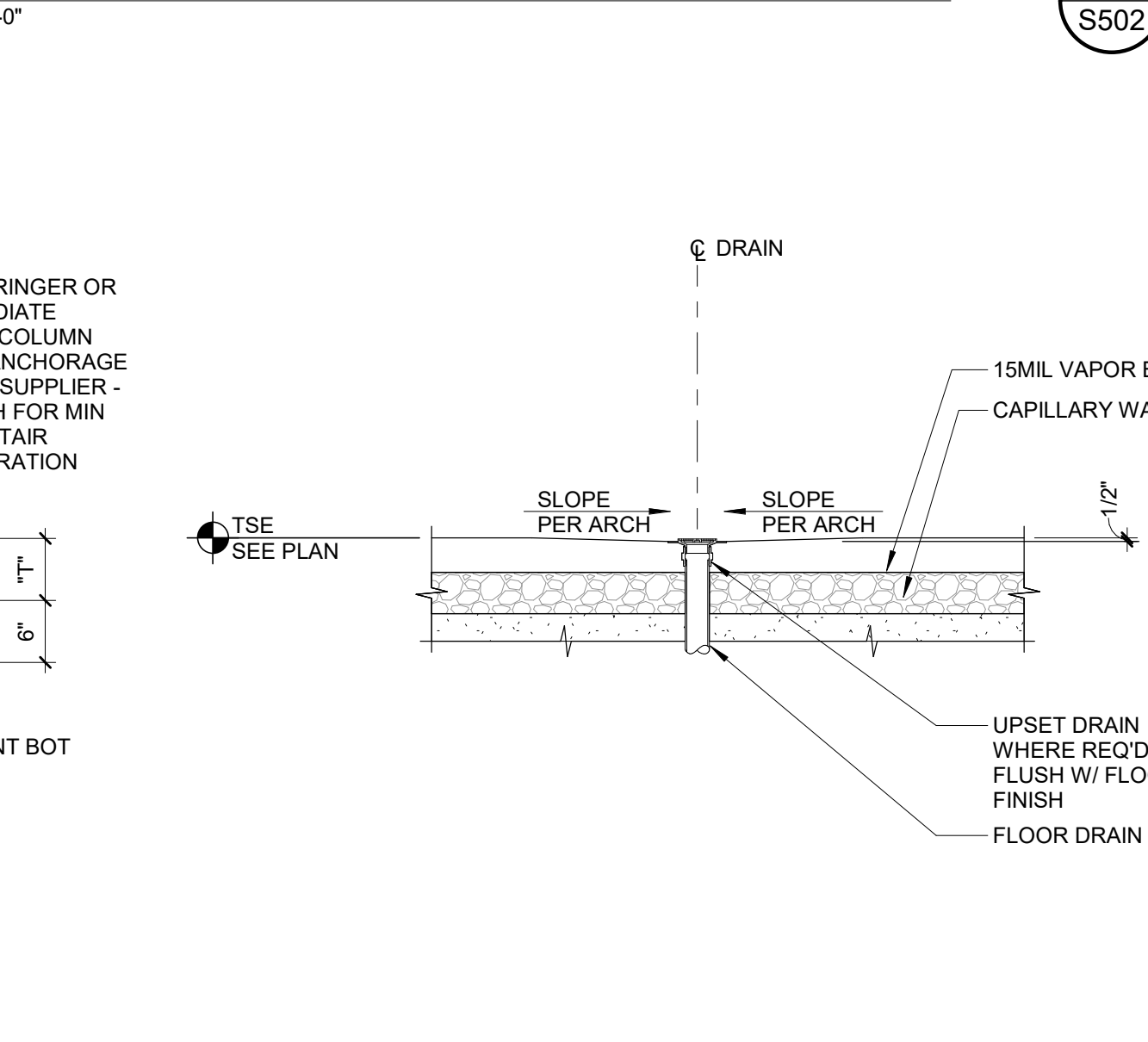
5 TYPICAL CORNER BAR DETAIL - SINGLE CURTAIN



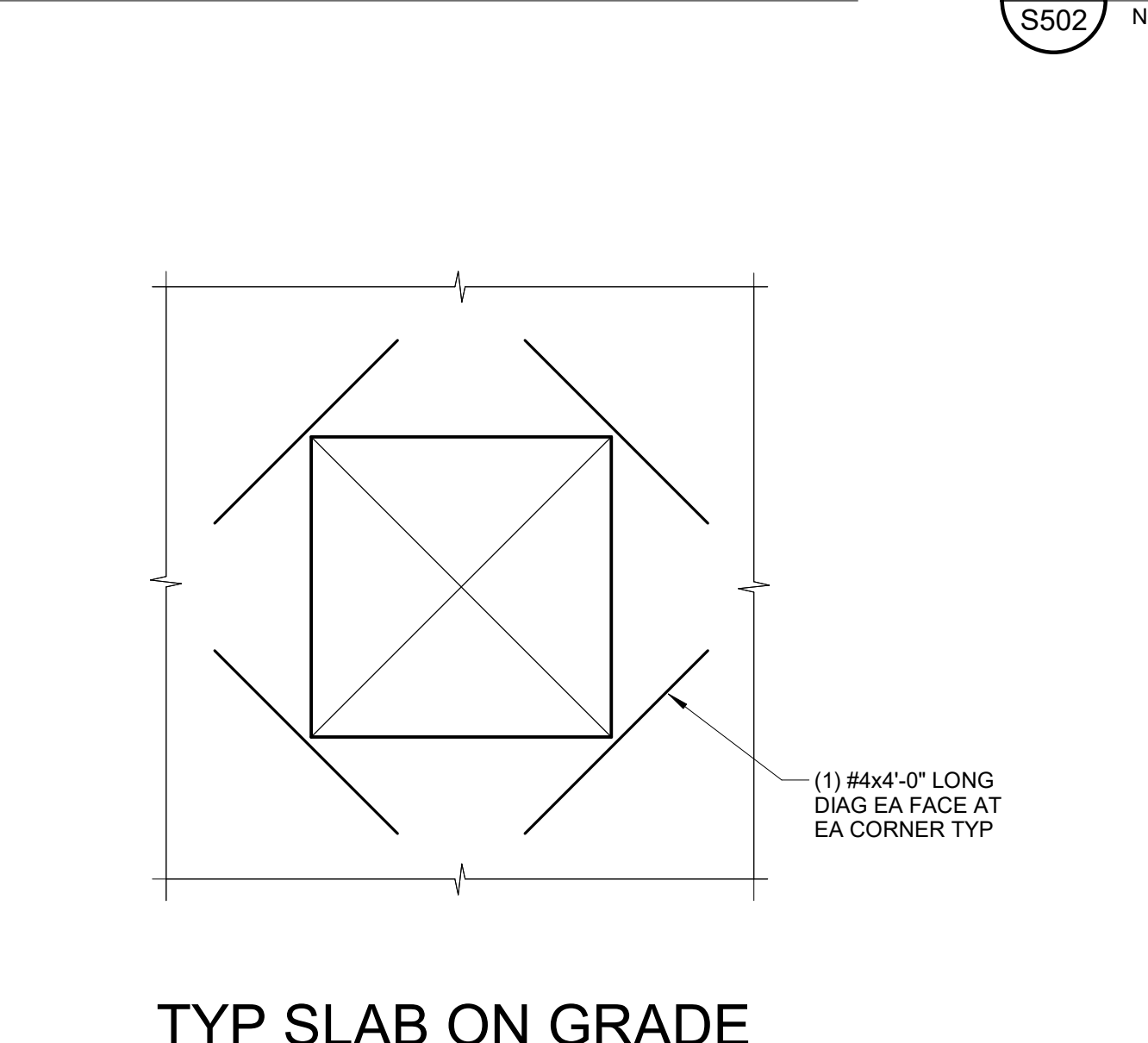
6 TYPICAL CORNER BAR DETAIL - DOUBLE CURTAIN



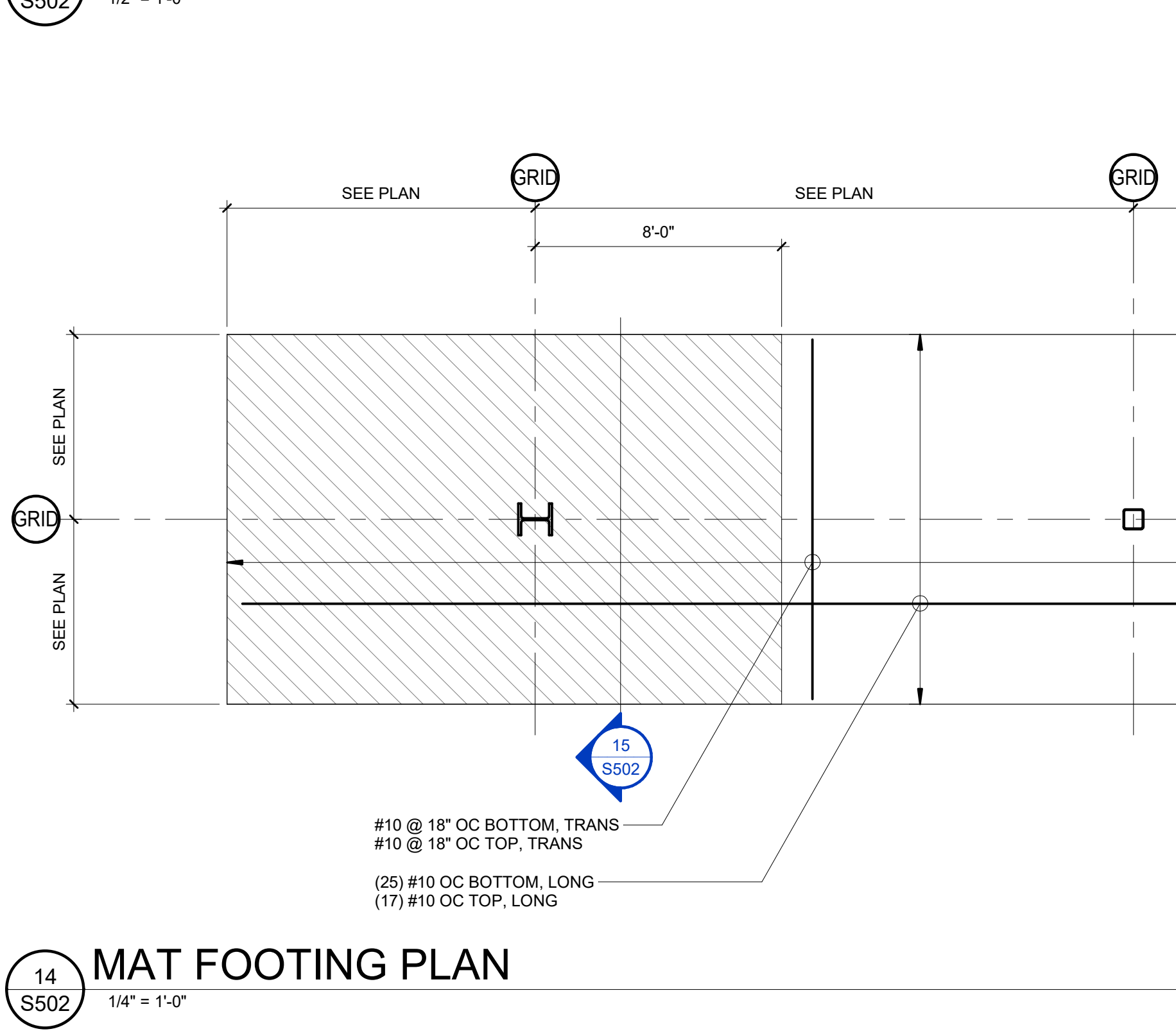
7 TYP VERTICAL CONSTRUCTION JOINTS FOR CONCRETE WALLS



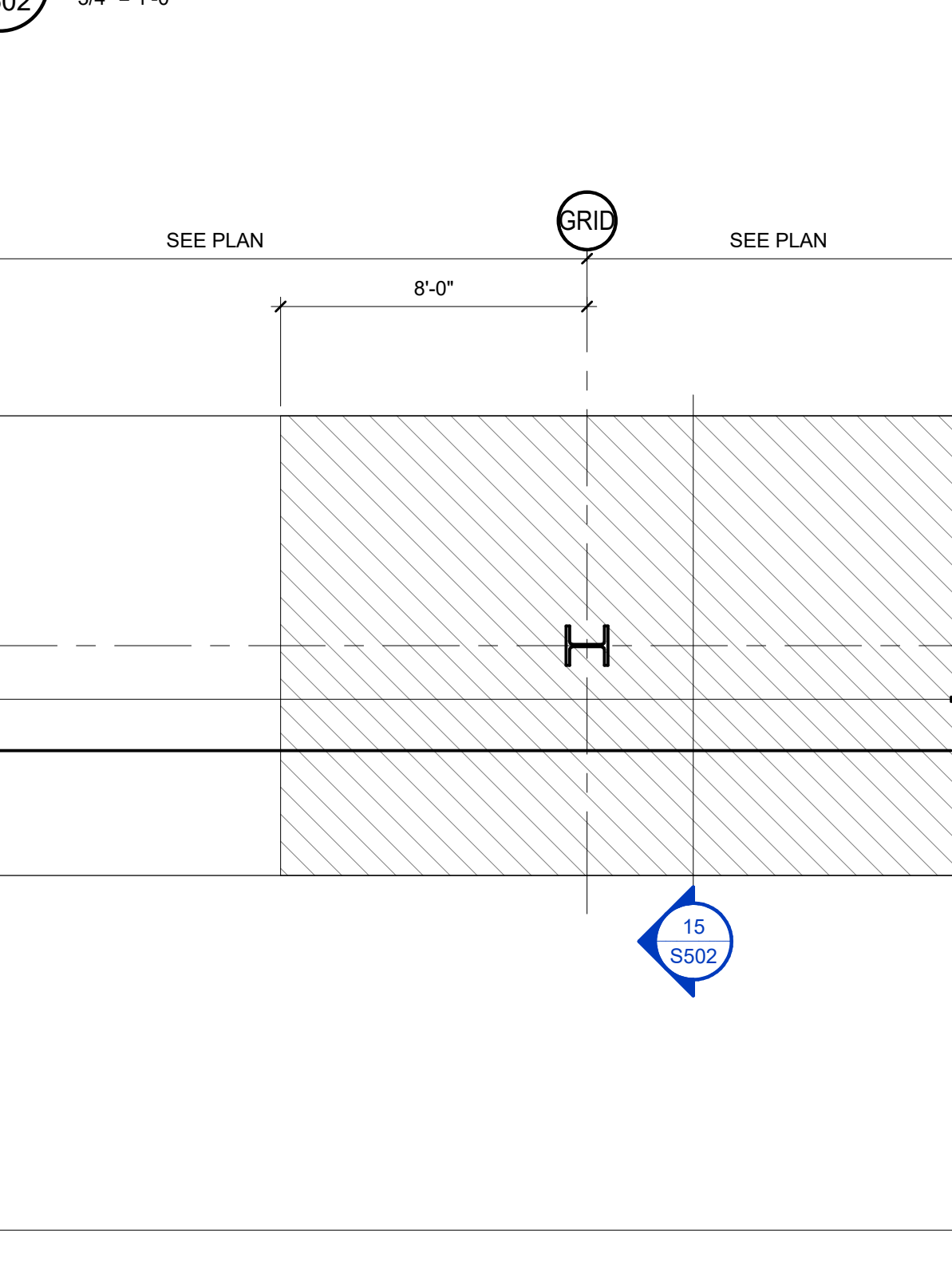
8 TYP SLAB JOINT DETAIL



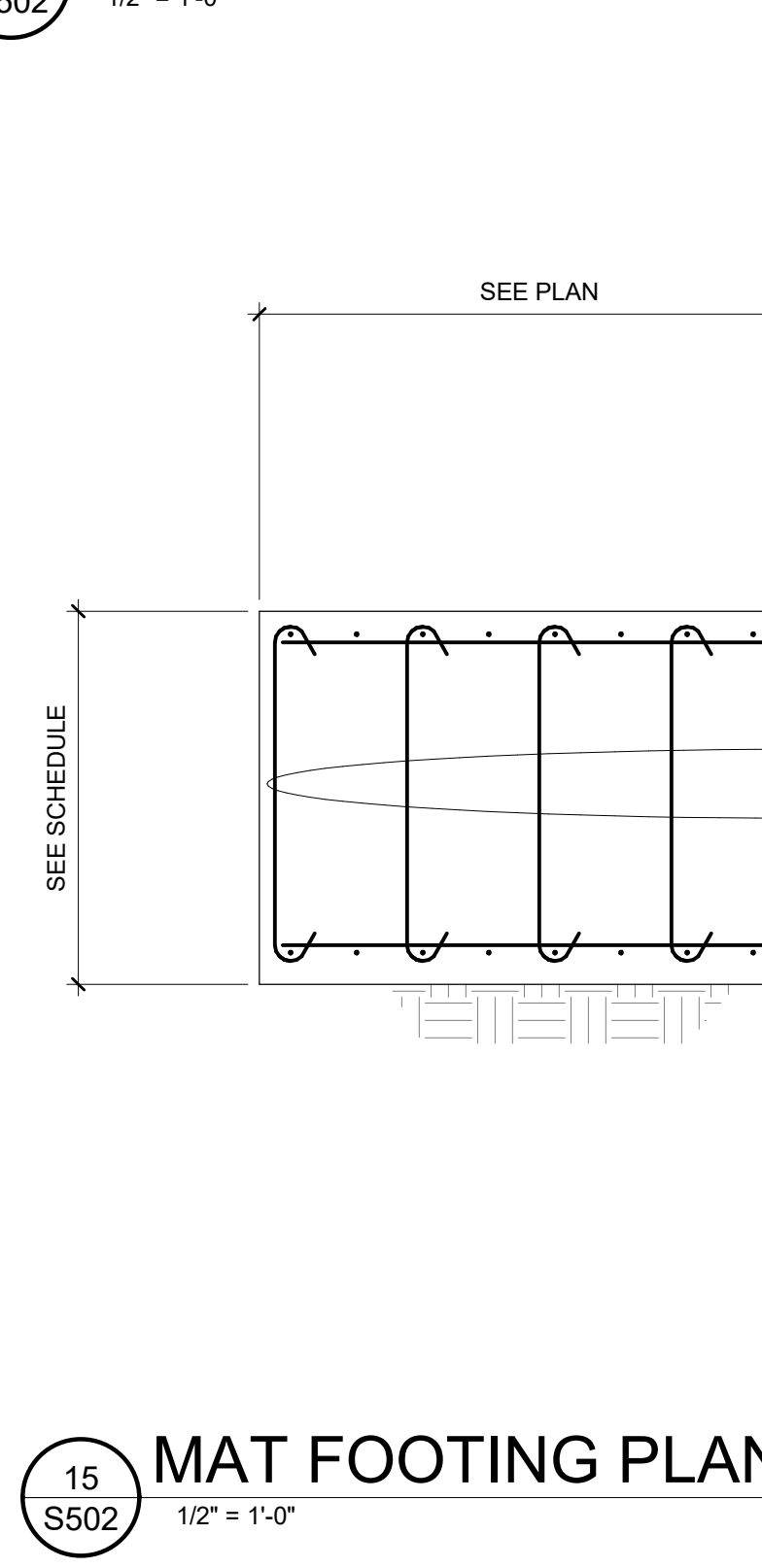
10 TYP SLAB ON GRADE STEPS AND THICKENED EDGE



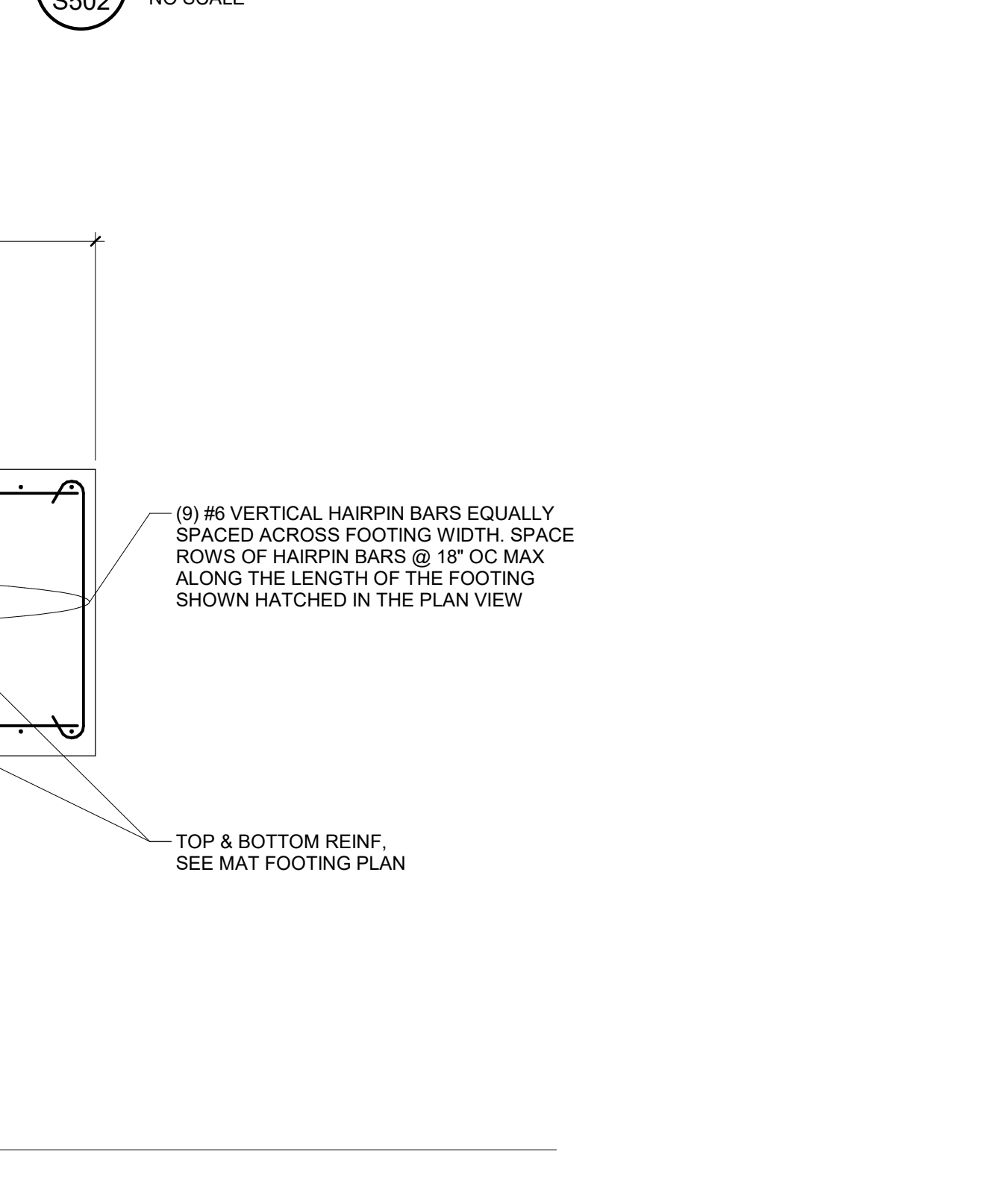
11 THICKENED SLAB AT STEEL STAIR



12 FLOOR DRAIN DETAIL



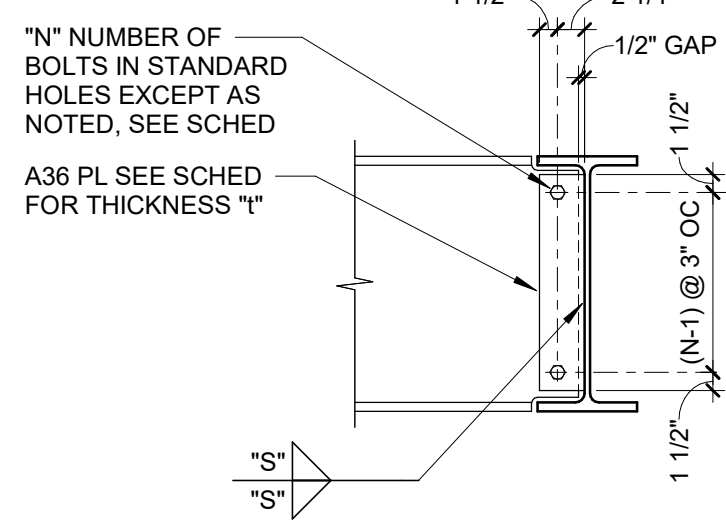
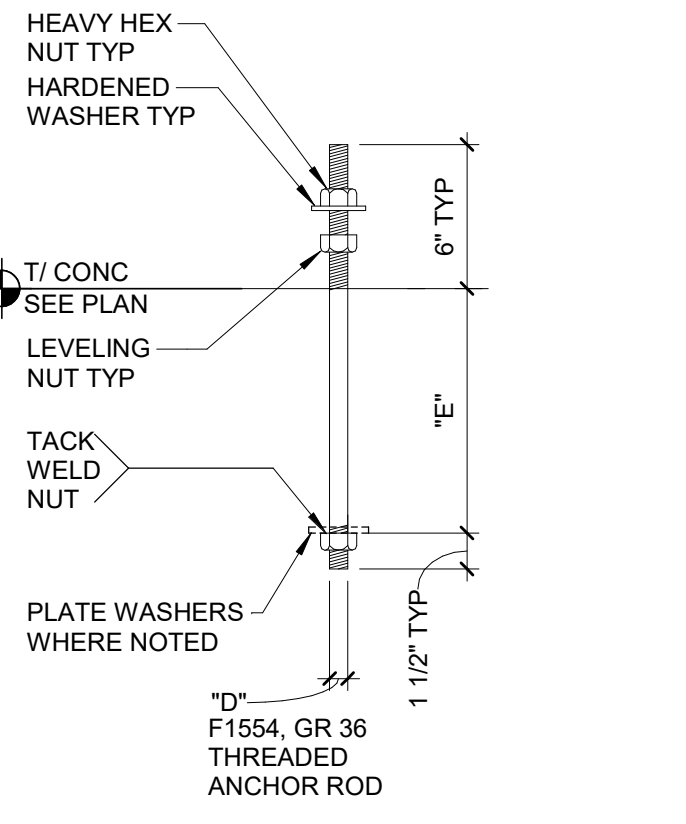
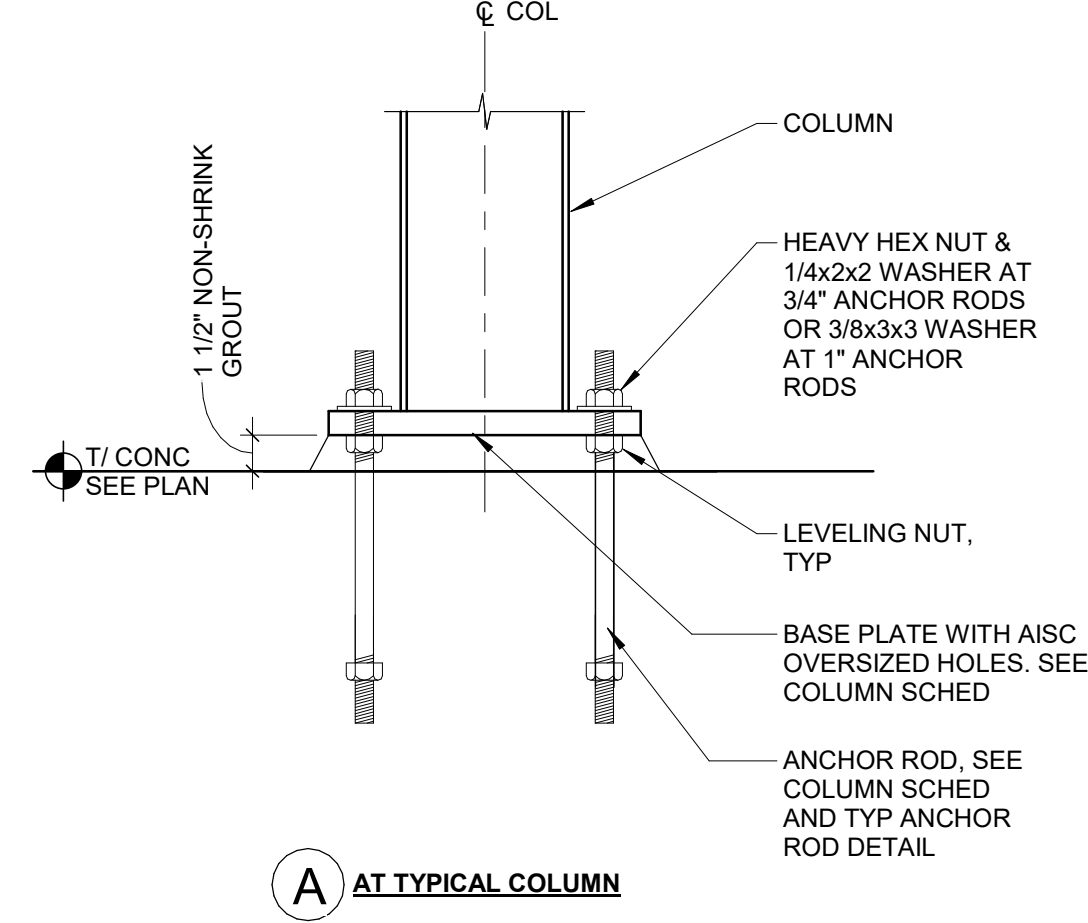
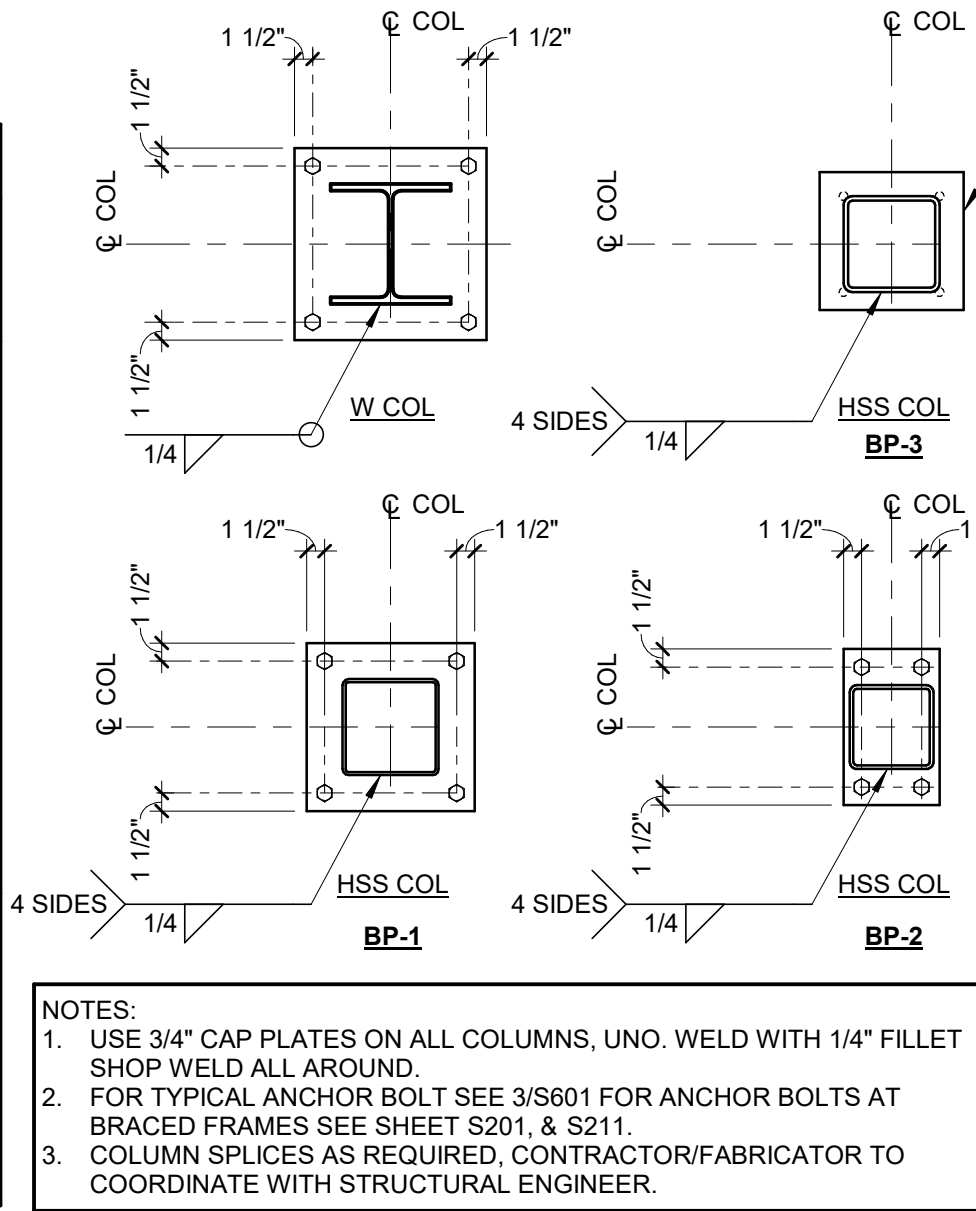
13 TYP SLAB ON GRADE OPENING REINF DETAIL



E
D
C
B
A

COLUMN AND BASE PLATE SCHEDULE						
MARK	SIZE	BASE PLATE TYPE	BASE PLATE SIZE 'T'x'W'x'L'	ANCHOR ROD DIAMETER 'D'	EMBED LENGTH 'E'	COMMENTS
C1	W10x45	BP-1	1 1/2x16x16	3/4"	0'-8"	
C2	W10x49	BP-1	1 1/2x16x16	3/4"	0'-8"	
C3	W10x54	BP-1	1 1/2x16x16	3/4"	0'-8"	
C4	W10x69	BP-1	1 3/4x16x16	3/4"	0'-8"	
C5	W10x77	BP-1	1 3/4x16x16	3/4"	0'-8"	
C6	W12x120					SEE DETAIL 7/S211
C7	HSS6x6x5/8					SEE DETAIL 7/S511
C8	HSS9x9x1/2	BP-1	1x15x15	3/4"	1'-4"	
C9	HSS8x8x5/8	BP-1	1x14x14	3/4"	0'-8"	
C10	HSS6x6x1/2	BP-2	1x7x12	3/4"	0'-8"	
C11	HSS4x4x3/8	BP-1	3/4x10x10	3/4"	0'-8"	
C12	HSS7x7x3/8	BP-2	1x6x13	3/4"	0'-8"	
C13	HSS7x5x3/8	BP-2	1x6x13	3/4"	0'-8"	
C14	HSS3x3x1/2	BP-1	1x14x14	3/4"	0'-8"	
C15	HSS5x5x3/8	BP-1	3/4x11x11	3/4"	0'-8"	
C16	HSS8x8x3/8	BP-3	-	-	-	SEE BP-3 DETAIL

NOTES:
1. SEE THIS SHEET FOR BASE PLATE DETAILS, TYPICAL COLUMN SETTING DETAIL AND TYPICAL ANCHOR ROD DETAIL.
2. MILL COLUMN BASE AND/OR BASE PLATE AS NECESSARY FOR FULL CONTACT PER AISC 360 SECTION M2.8.
3. BASE PLATE THICKNESS INDICATED IS MINIMUM THICKNESS AFTER MILLING.
4. ANCHOR RODS SHALL NOT BE SET INTO CONCRETE AFTER CONCRETE IS CAST.
5. PROVIDE MAXIMUM HOLE SIZE IN BASE PLATE FOR ANCHOR RODS PER AISC TABLE 14-2, UNLESS NOTED OTHERWISE.
6. SEE COLUMN BASE PLATE DETAILS FOR ADDITIONAL INFORMATION.



CONNECTION SCHEDULE					REMARKS
FRAMING BEAM	NO. OF BOLTS 'N'	WELD SIZE 'S'	SHEAR TAB THICKNESS 'T'		
W8, W10	2	1/4"	3/8"		
W12, W14	3	1/4"	3/8"		
W16	4	1/4"	3/8"		
W18	5	1/4"	3/8"		
W21, W24	6	1/4"	3/8"		PROVIDE SHORT SLOTTED HOLES IN CONN PL
W27	7	1/4"	3/8"		PROVIDE SHORT SLOTTED HOLES IN CONN PL
W30	8	1/4"	3/8"		PROVIDE SHORT SLOTTED HOLES IN CONN PL
W33, W36	9	1/4"	3/8"		PROVIDE SHORT SLOTTED HOLES IN CONN PL
W40	10	1/4"	3/8"		PROVIDE SHORT SLOTTED HOLES IN CONN PL

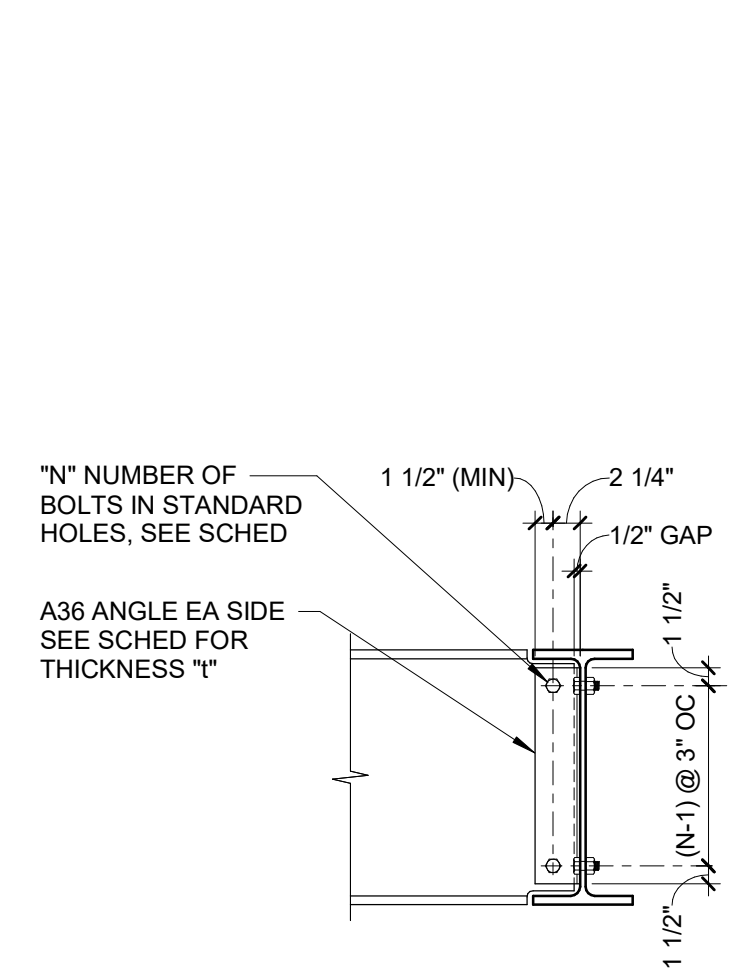
NOTES:
1. FOR CONNECTION WITH SUPPORTING MEMBER WEB THICKNESS GREATER THAN 3/4" USE 3/8" WELD AND 7/16" THICK TAB PLATE.
2. TYPICAL CONNECTION DESIGN IS BASED ON MAXIMUM TOP FLANGE COPE OF 1 3/8" DEEP AND 4" LONG.
3. "N" = NUMBER OF ROWS OF 3/4" DIA A325 BOLTS. BOLTS/CONNECTIONS SHALL BE SLIP CRITICAL WHERE SHORT SLOTTED HOLES ARE USED.
4. WHERE SHORT-SLOTTED HOLES ARE USED, THE SLOTTED HOLES SHALL BE IN THE CONN PLATE.

1 COLUMN AND BASE PLATE SCHEDULE

2 TYP COLUMN SETTING DETAIL

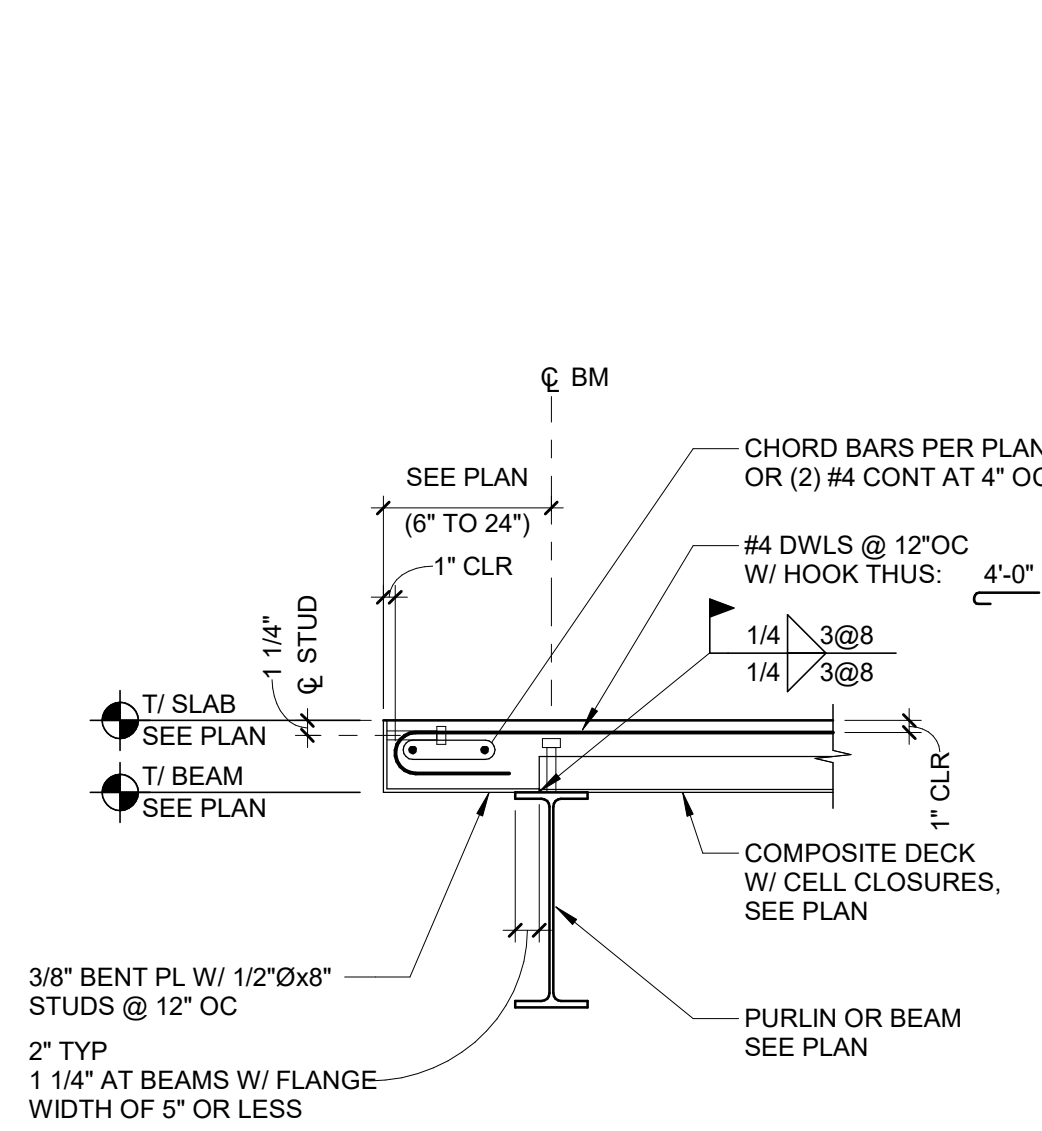
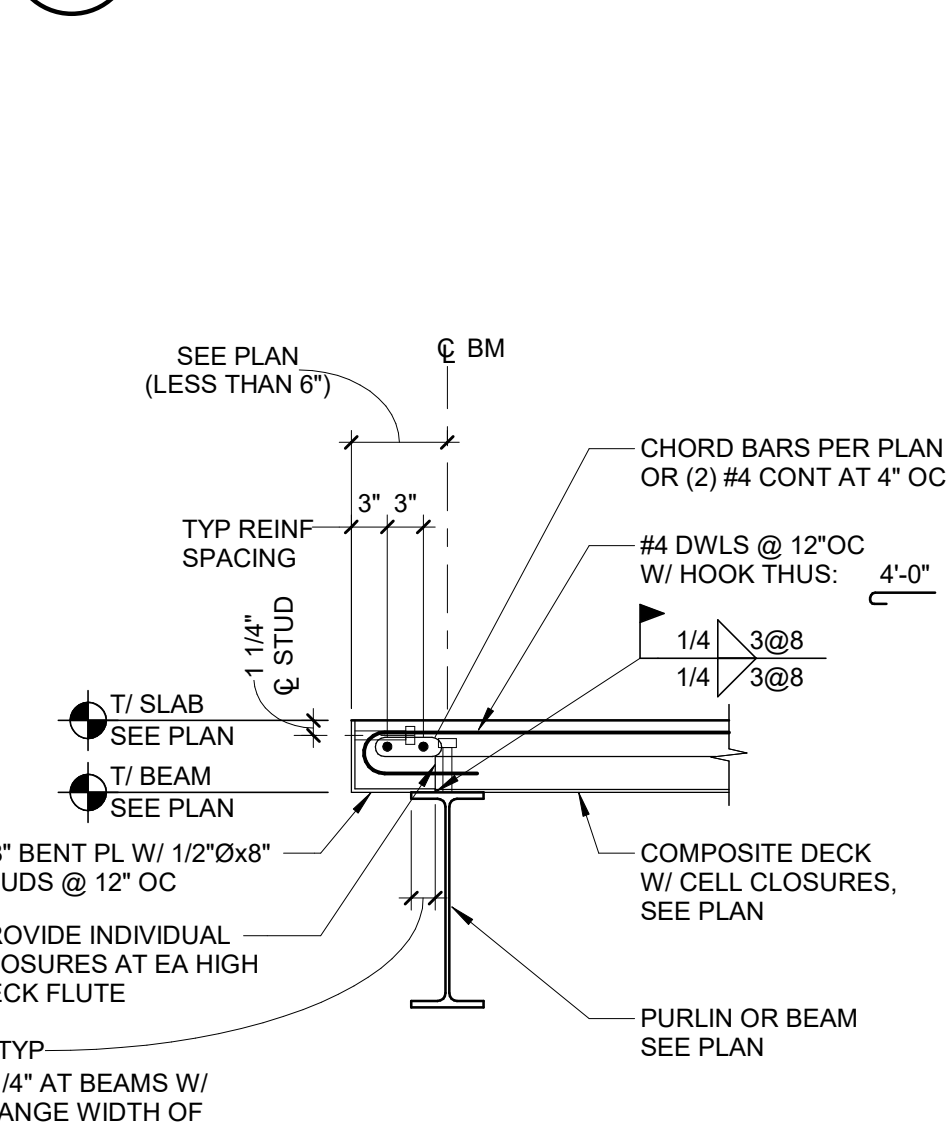
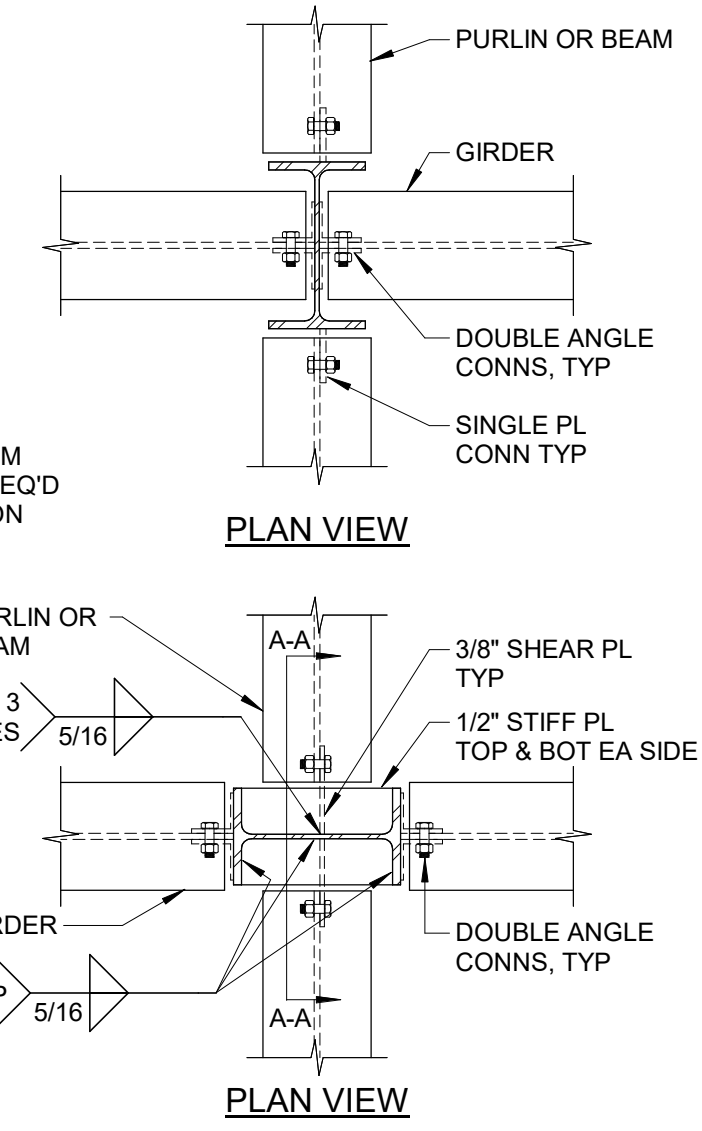
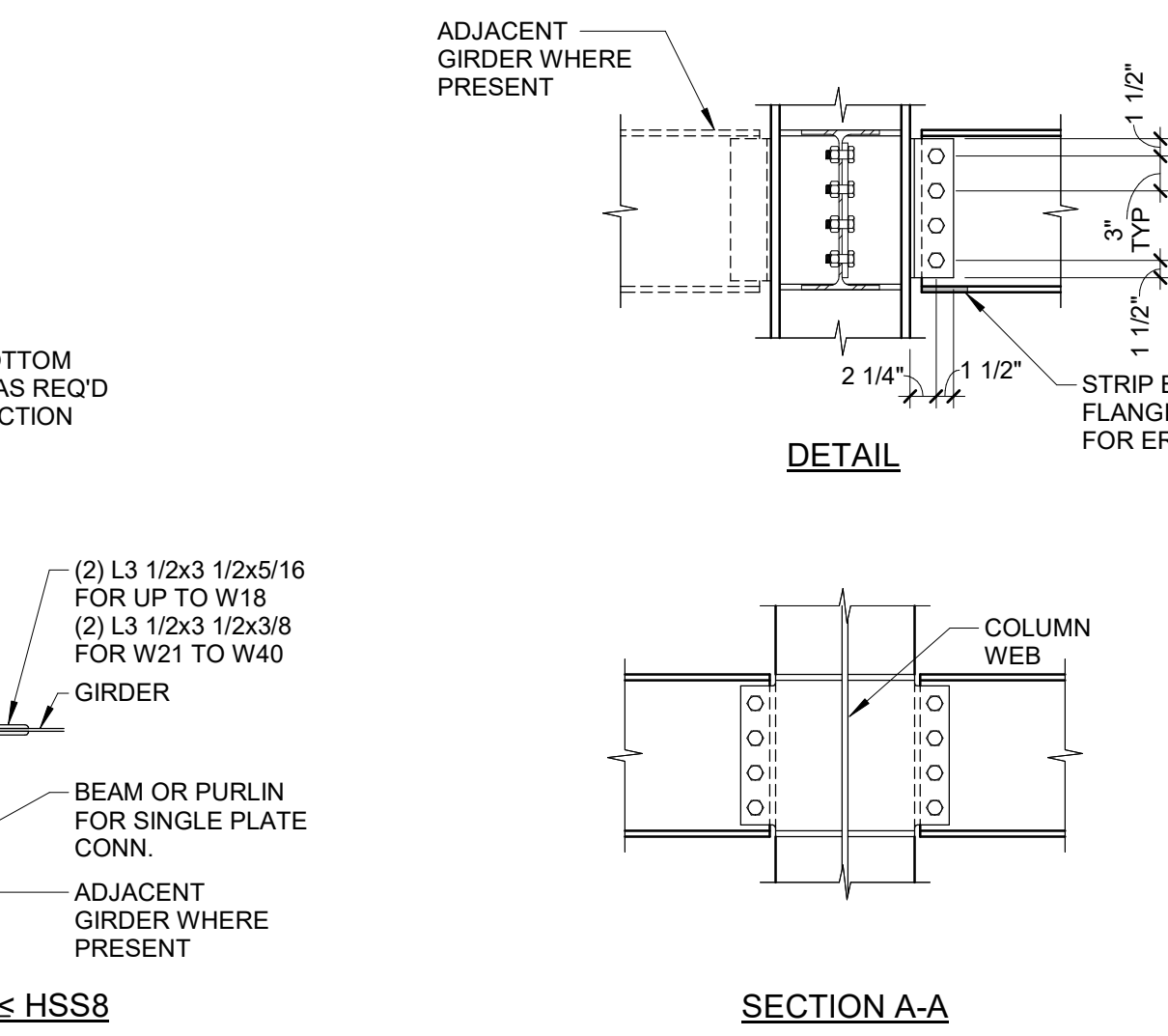
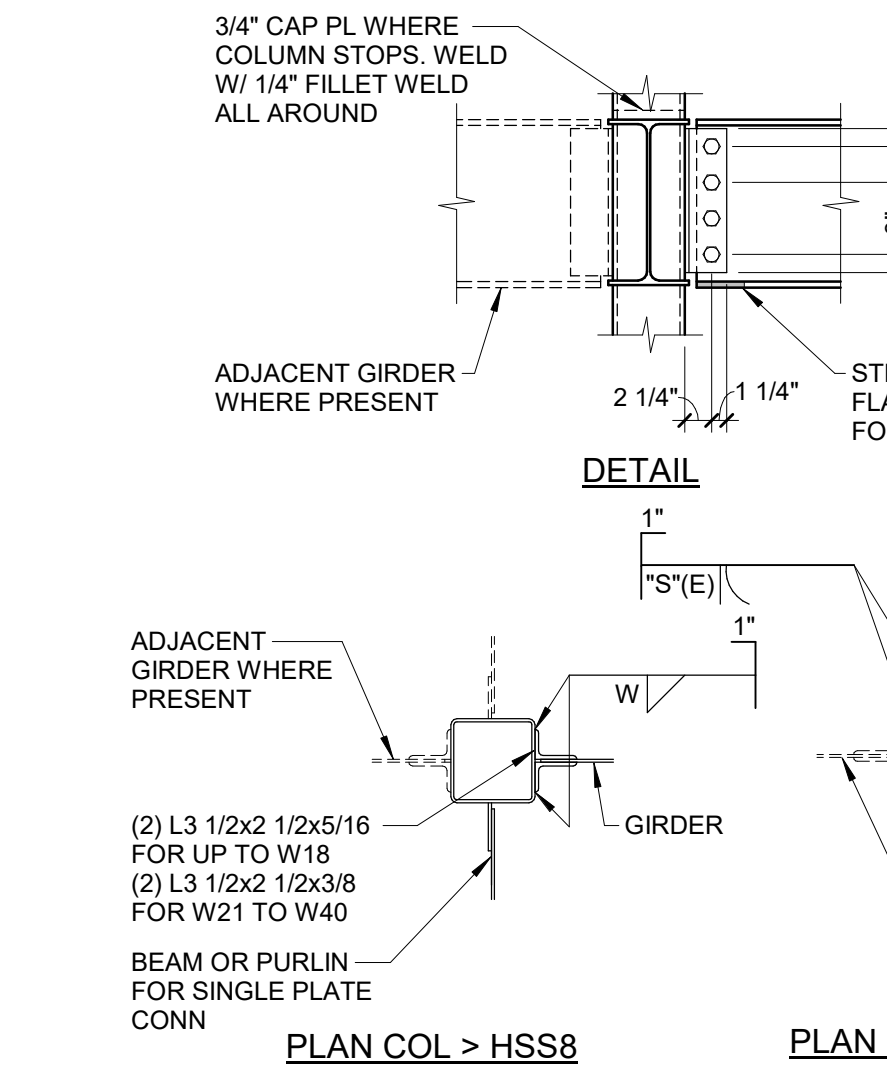
3 TYP ANCHOR ROD DETAIL

4 TYP SINGLE PLATE CONNECTION SCHEDULE



CONNECTION SCHEDULE			
FRAMING BEAM	NO. OF BOLTS 'N'	ANGLE THICKNESS 'T'	REMARKS
W8	2	5/16"	
W10	2	5/16"	
W12	3	5/16"	
W14	3	5/16"	
W16	4	5/16"	
W18	5	5/16"	
W21	6	3/8"	
W24	6	3/8"	
W27	7	3/8"	
W33, W36	9	3/8"	
W40	10	3/8"	

NOTES:
1. TYPICAL CONNECTION DESIGN IS BASED ON MAXIMUM TOP FLANGE COPE OF 1 3/8" DEEP AND 4" LONG.
2. "N" = NUMBER OF ROWS OF 3/4" DIA A325 BOLTS.
3. "T" ON PLAN DENOTES TOTAL NUMBER OF BOLTS REQUIRED WHERE DEEP BEAM FRAMES INTO SHALLOW BEAM.

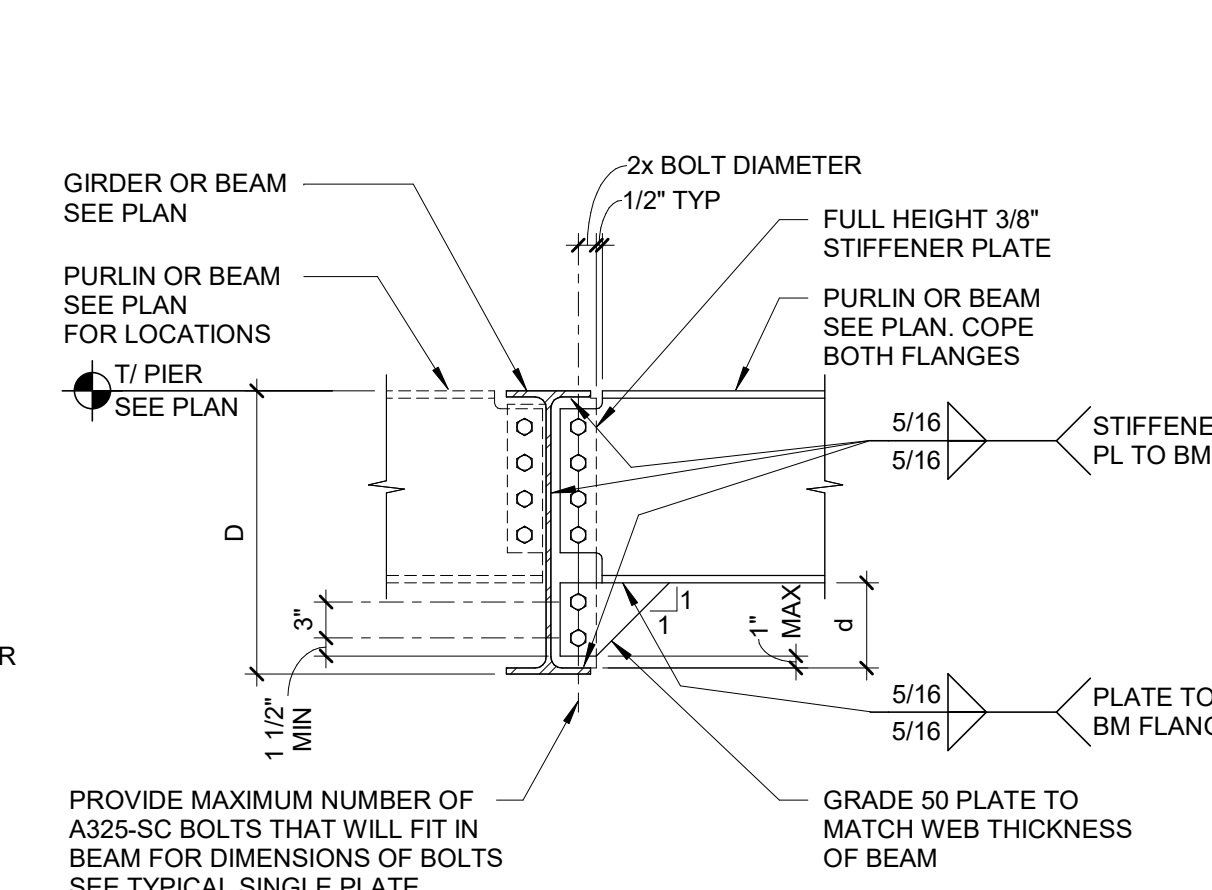
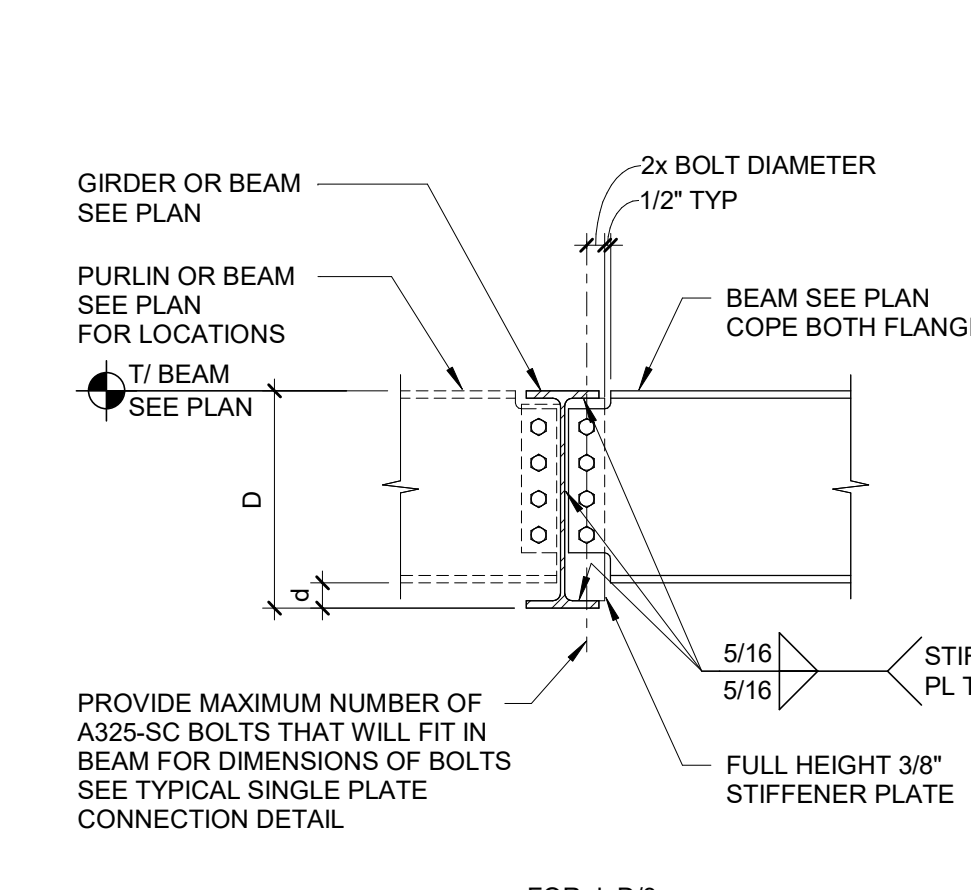
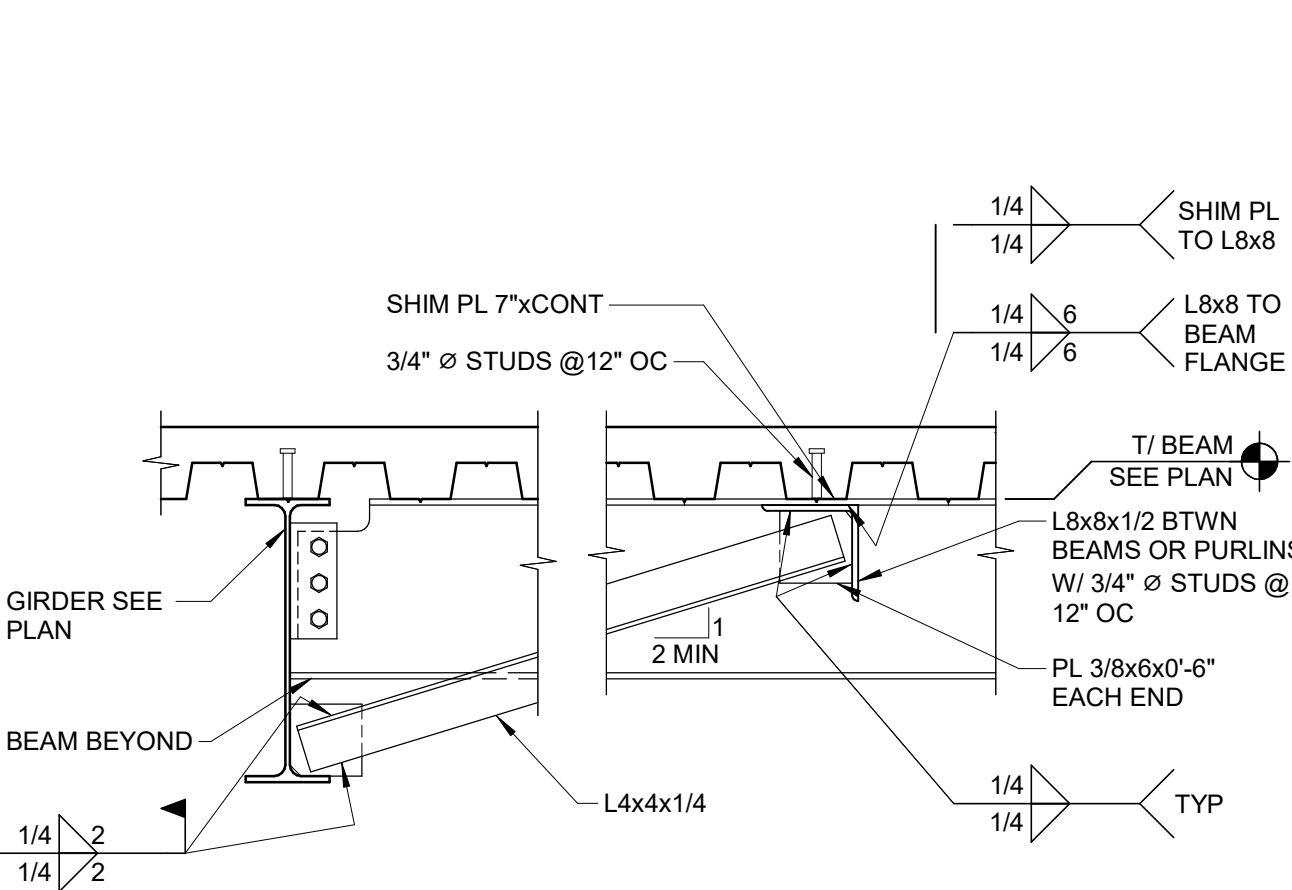
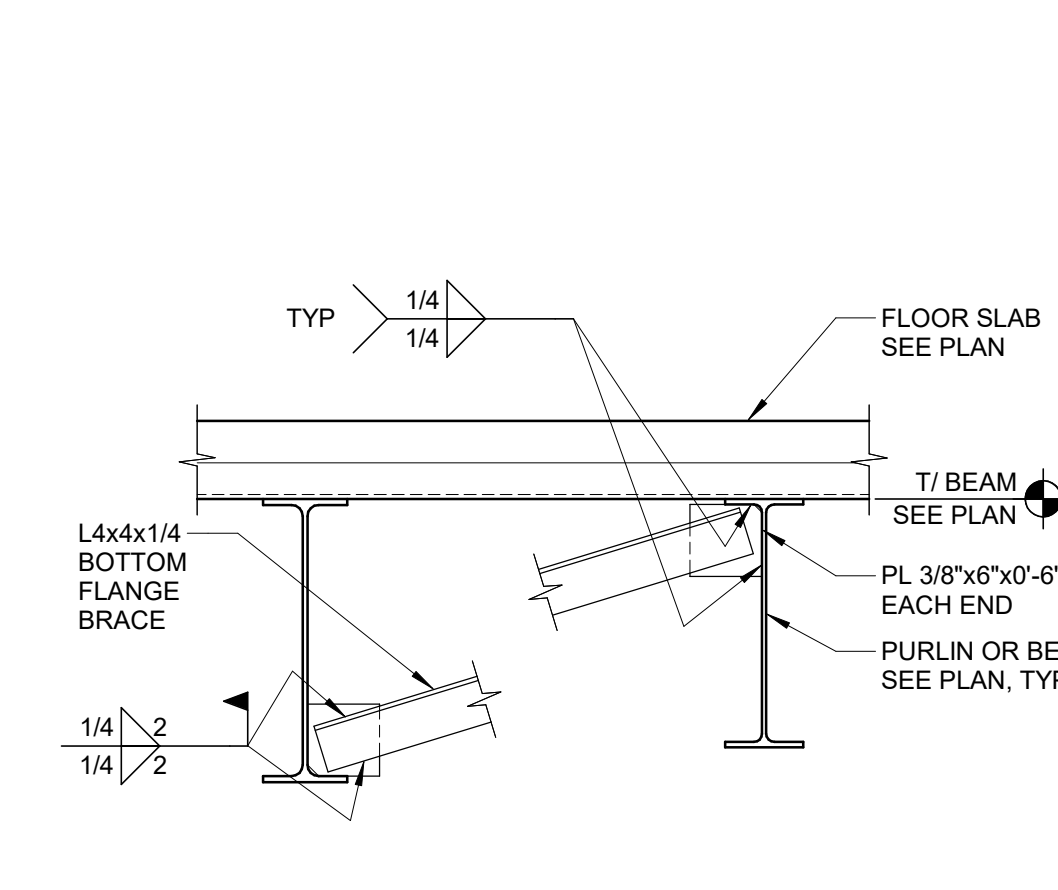


5 TYP DOUBLE ANGLE CONNECTION SCHEDULE

6 BEAM TO HSS COL CONNECTION

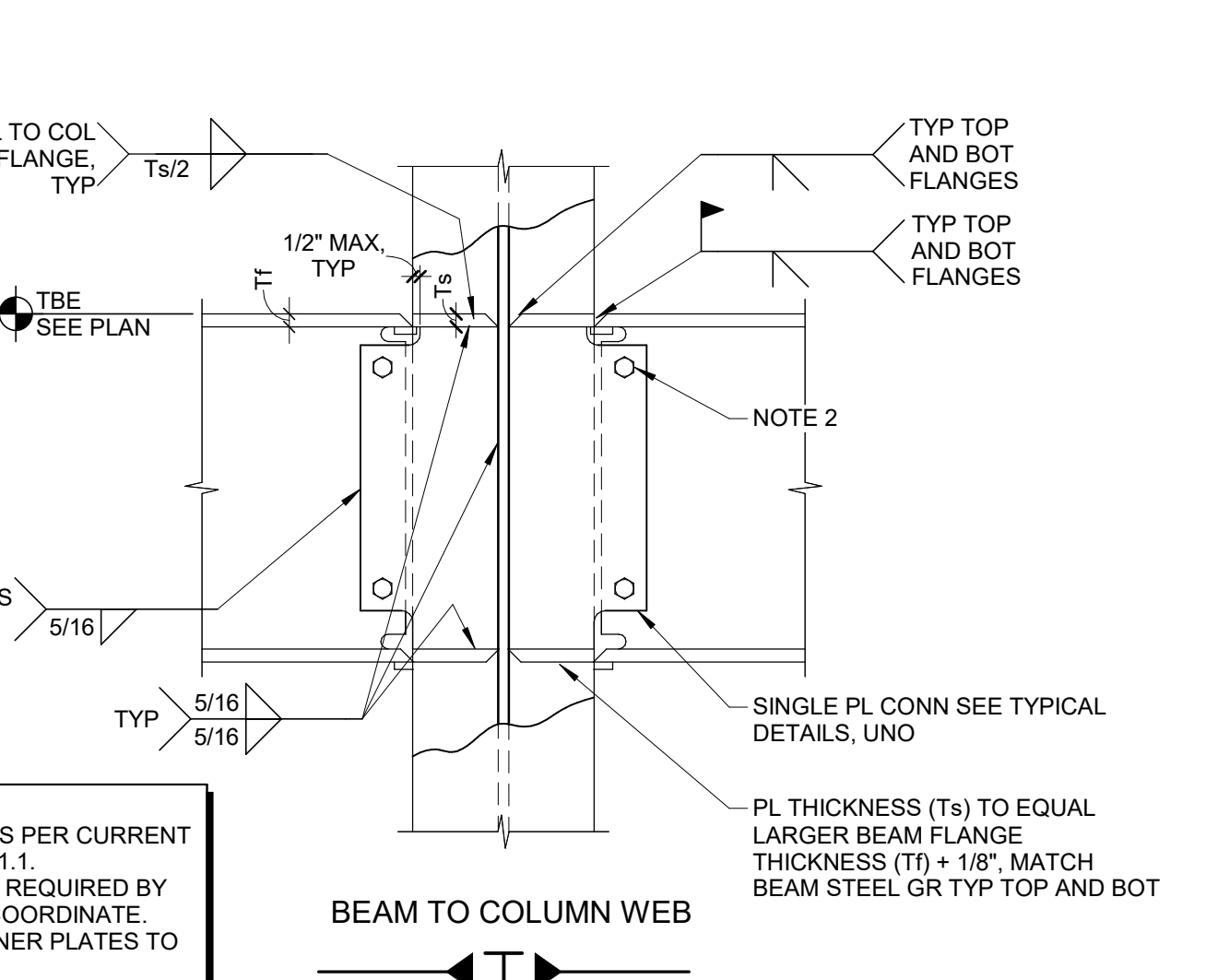
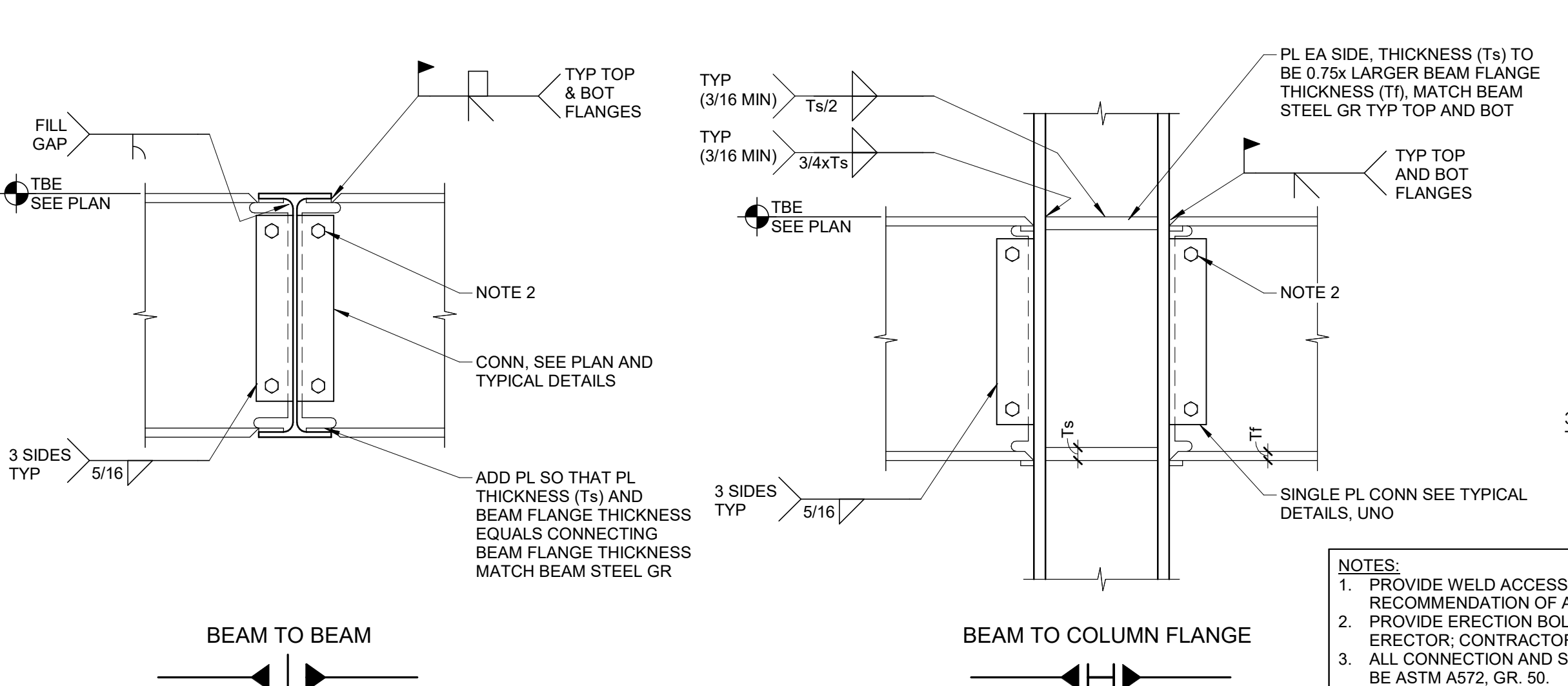
7 BEAM TO W COL CONNECTION

8 TYP COMPOSITE SLAB EDGE DETAIL

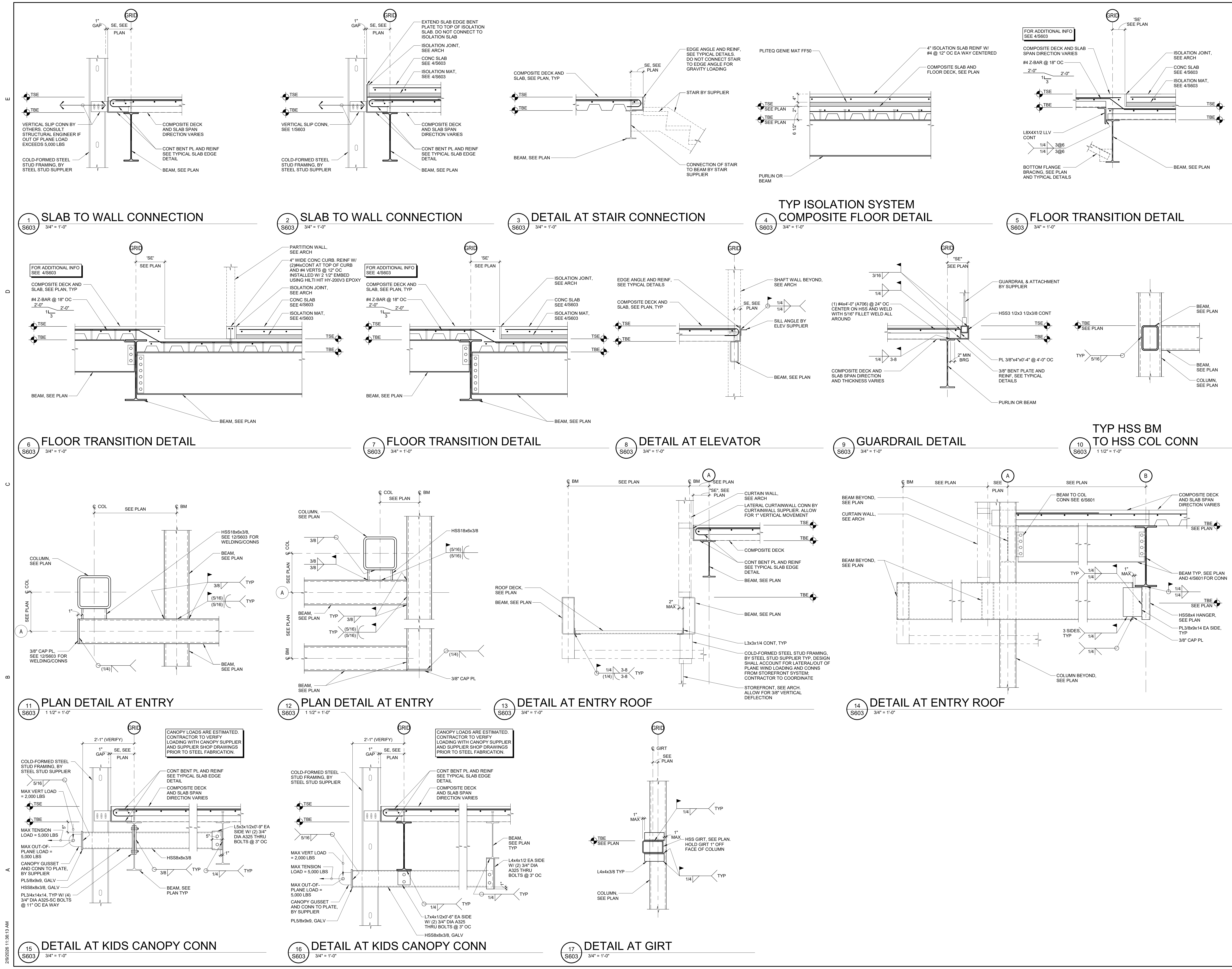


9 TYP BOTTOM FLANGE BEAM BRACE

10 TYP FULL HEIGHT PLATE CONNECTION DETAIL



11 TYPICAL MOMENT CONNECTIONS



LIFETIME
HEALTHY WAY OF LIFE
2900 CORPORATE PLACE CHANHASSEN, MN 55317

CONSULTANT

LIFETIME
PROPERTY DEVELOPMENT

VAA, LLC
2205 Berkeley Lane N, Suite 200
Plymouth, MN 55441

763.558.9100
www.vaaorg.com

PROJECT INFORMATION

Holladay, UT
1975 East Rodeo Drive
Holladay, Utah

DRAWING LOG

REV	DATE	DESCRIPTION

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BRIAN M. DALTON
No. 9832536

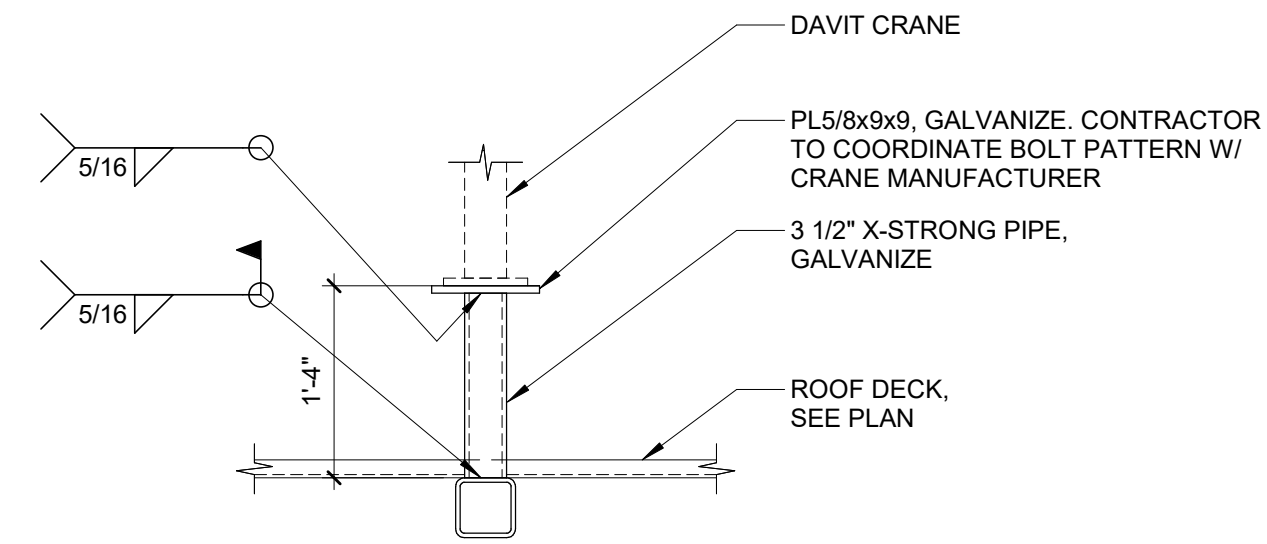
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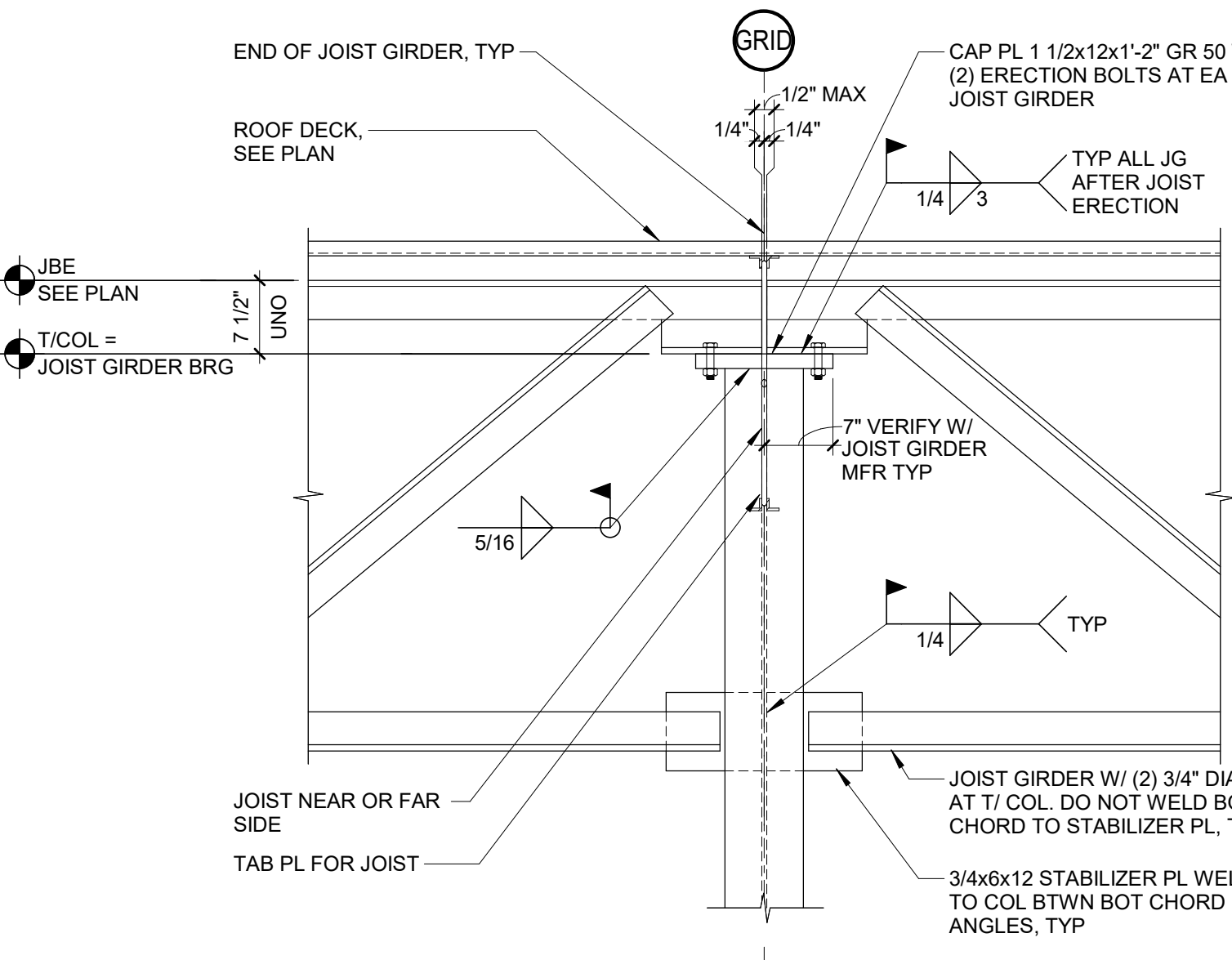
S603

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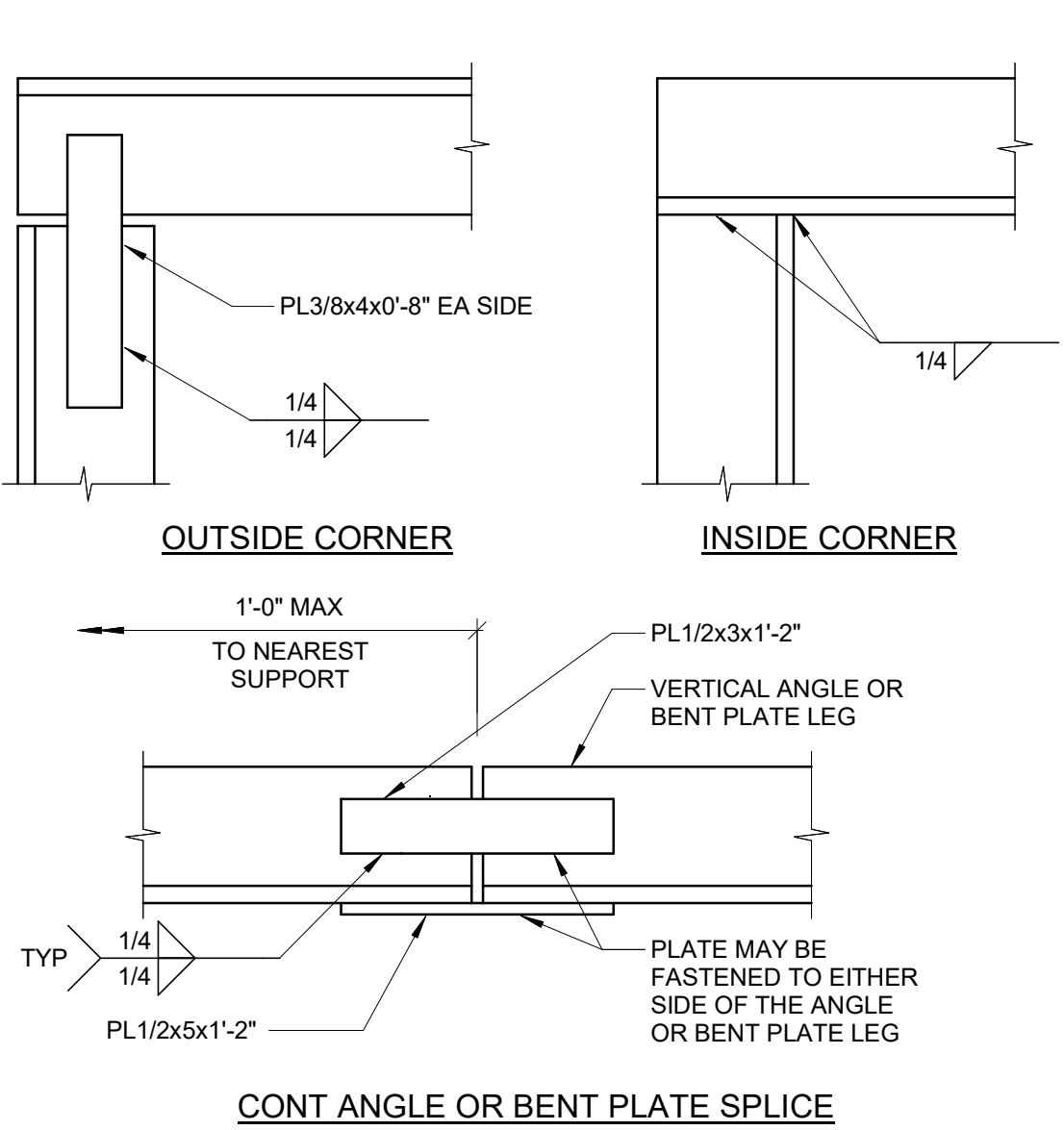
FLOOR FRAMING DETAILS



14
S611
FRAMING AT DAVIT CRANE
3/4" = 1'-0"



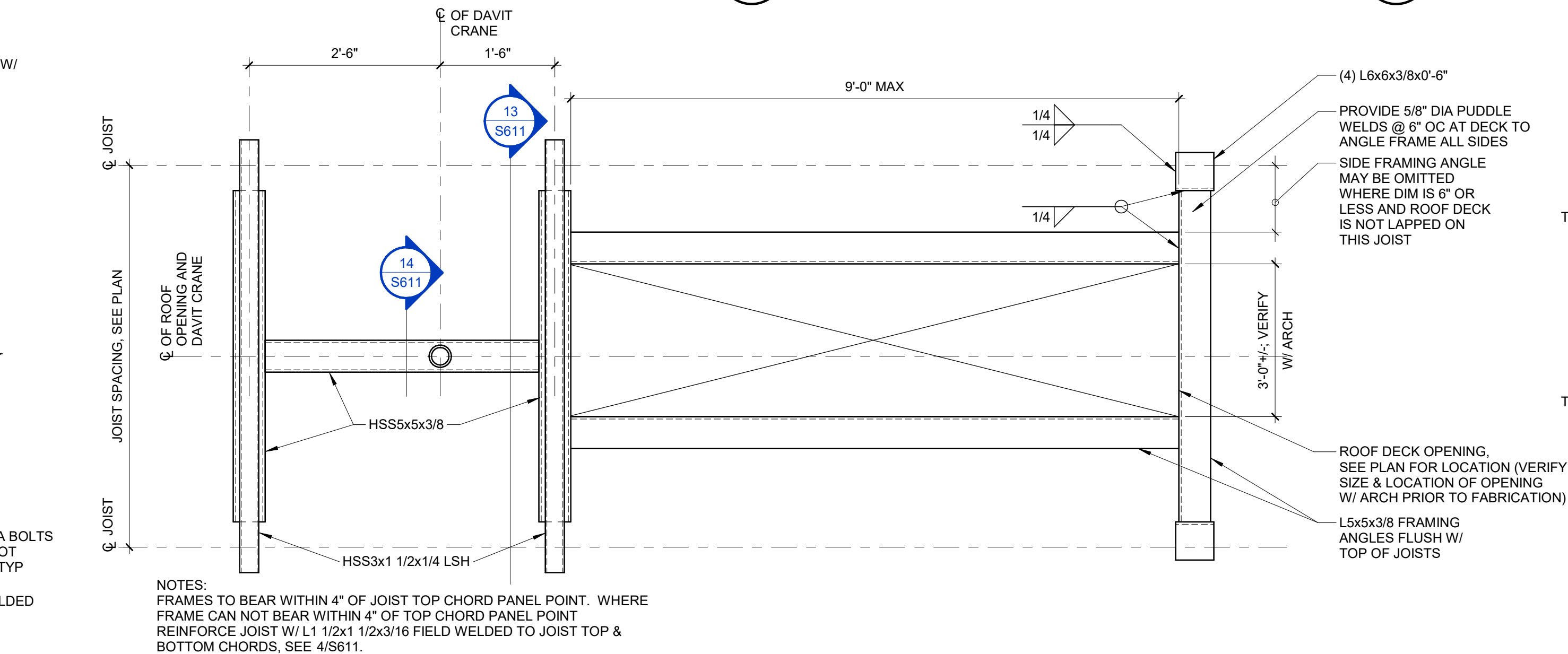
11
S611
JOIST GIRDER TO COLUMN DETAIL
3/4" = 1'-0"



6
S611
TYP ANGLE OR BENT PL SPLICE DETAIL
3/4" = 1'-0"

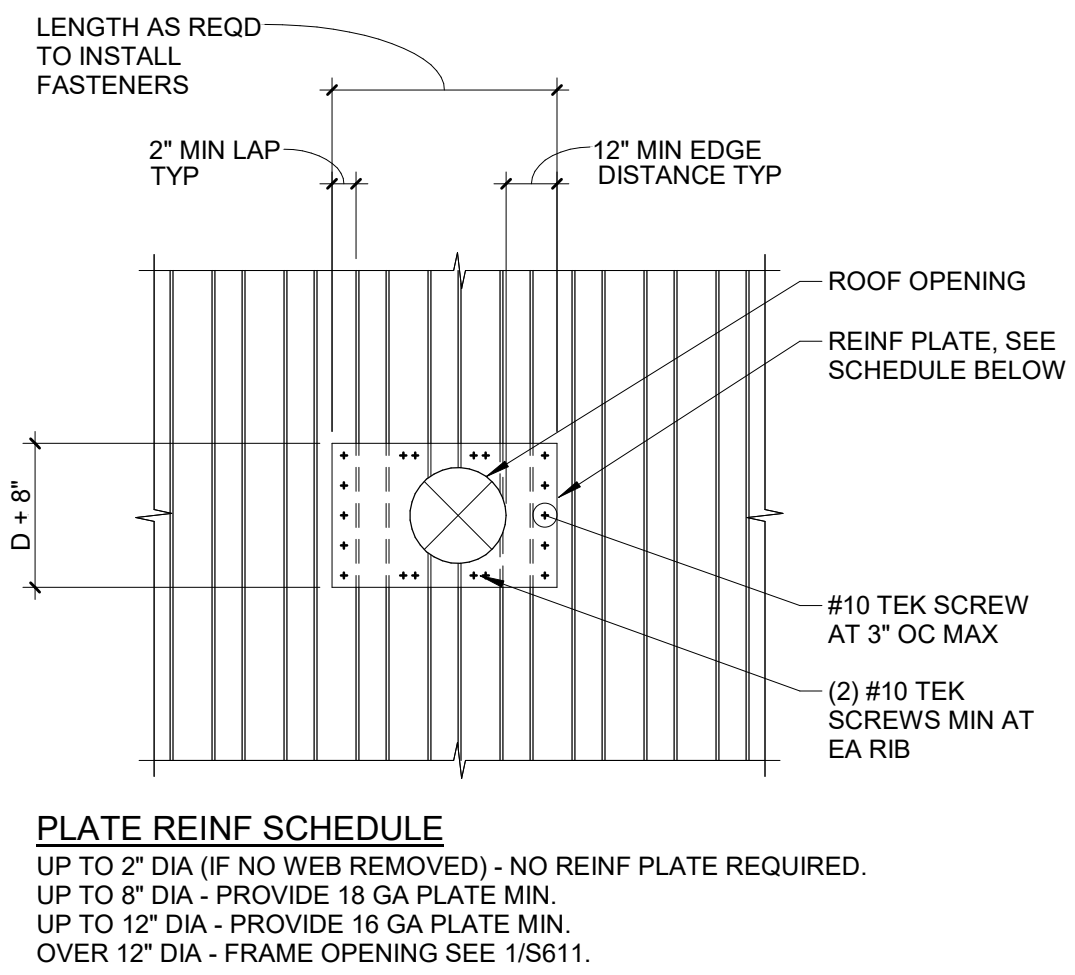
1
S611
TYP 1 1/2 ROOF DECK ATTACHMENT
3/4" = 1'-0"

12
S611
FRAMING AT ROOF HATCH AND DAVIT CRANE
3/4" = 1'-0"

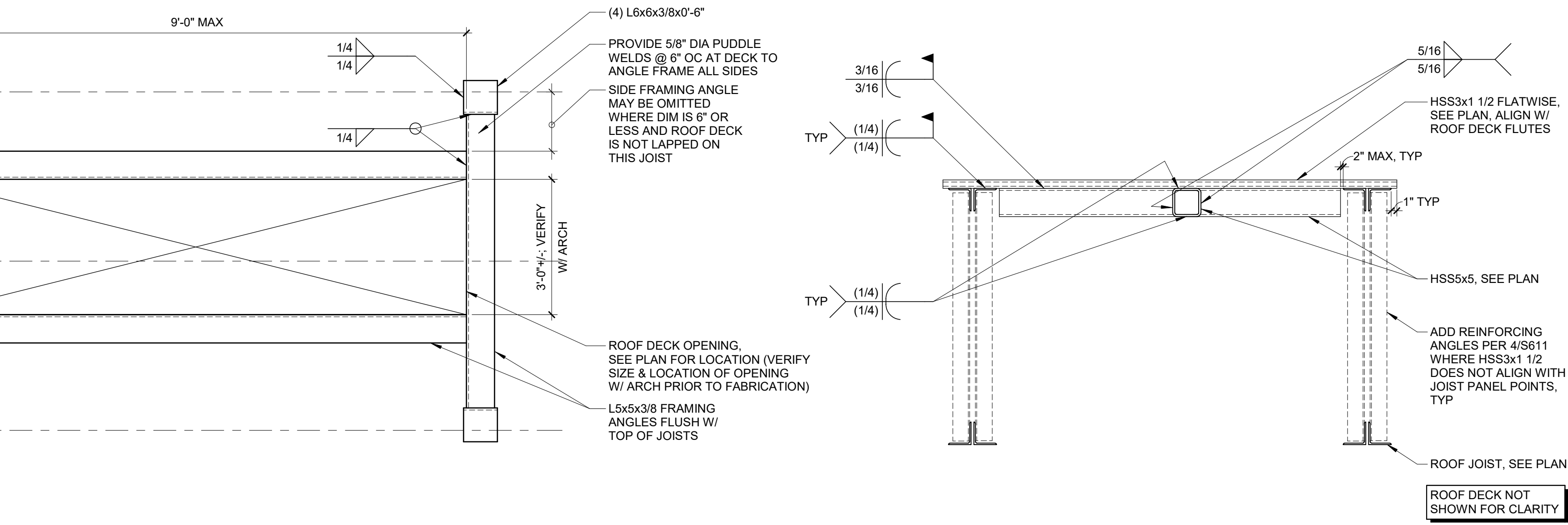


7
S611
TYPICAL BEAM SPLICE DETAIL
1 1/2" = 1'-0"

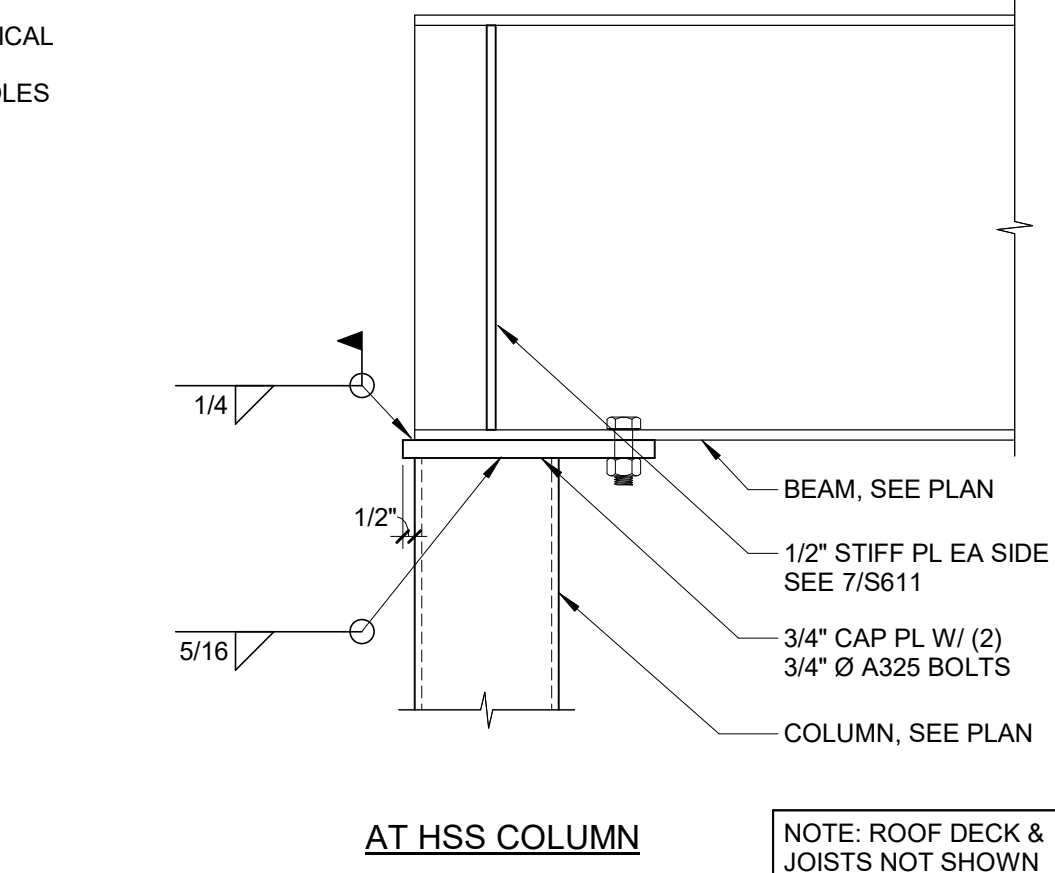
2
S611
TYP DECK REINF FOR ROOF DECK OPENINGS UP TO 12"Ø
3/4" = 1'-0"



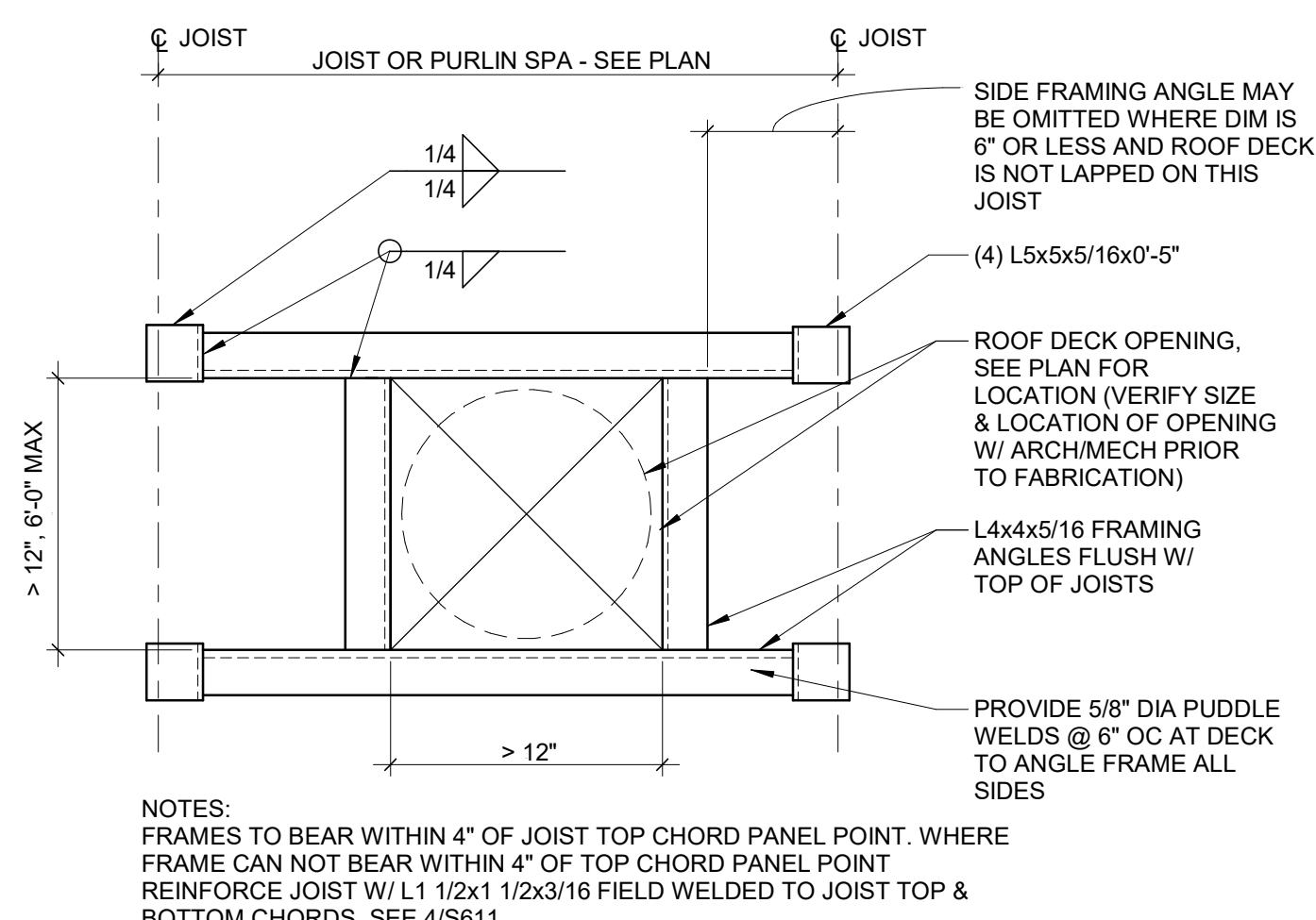
13
S611
FRAMING AT DAVIT CRANE
3/4" = 1'-0"



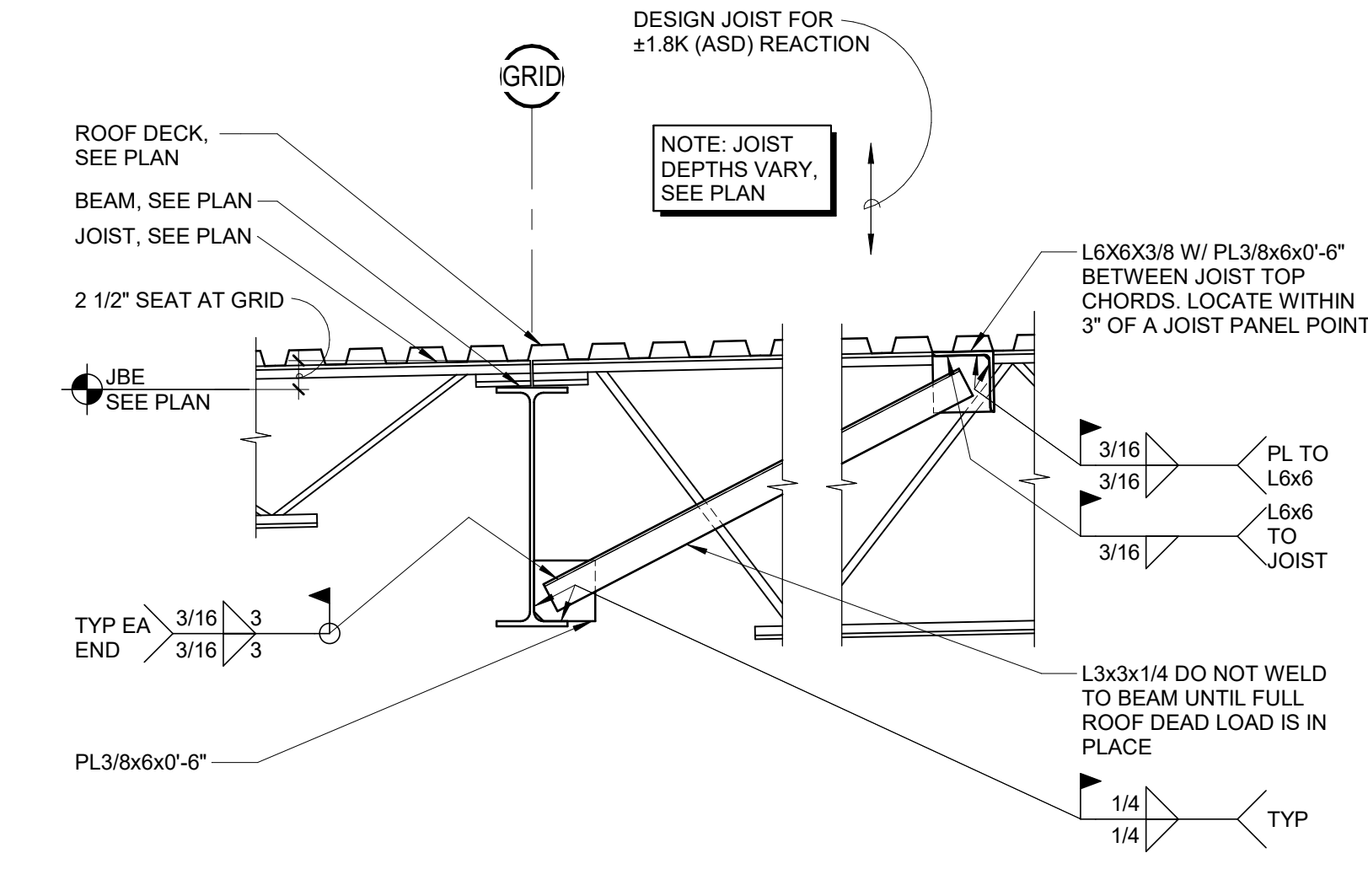
8
S611
COLUMN CAP PL DETAIL
1 1/2" = 1'-0"



3
S611
TYP DECK REINF FOR ROOF DECK OPNG LARGER THAN 12"
3/4" = 1'-0"

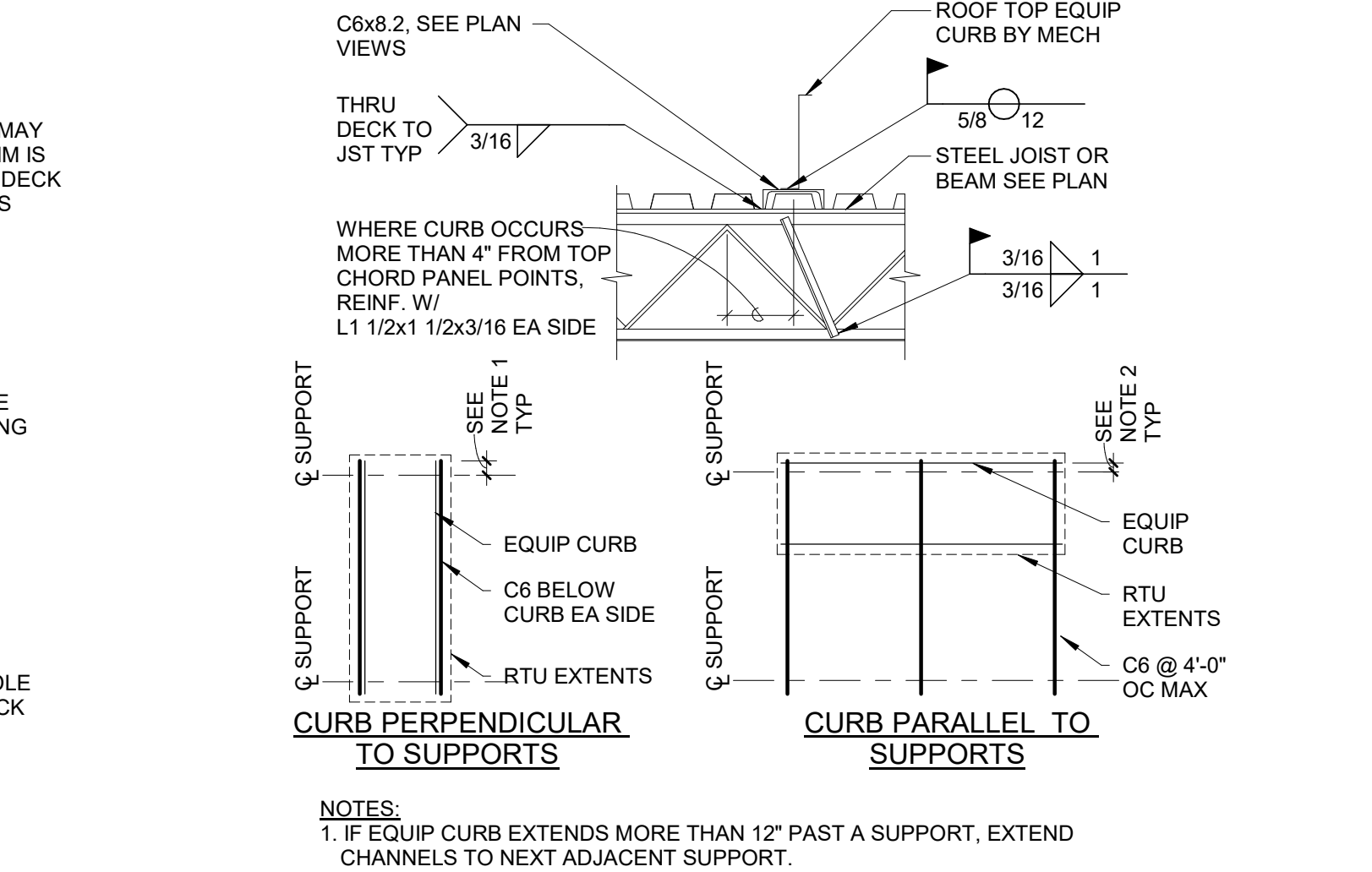


9
S611
TYP BOT FLANGE BM BRACE AT ROOF
3/4" = 1'-0"

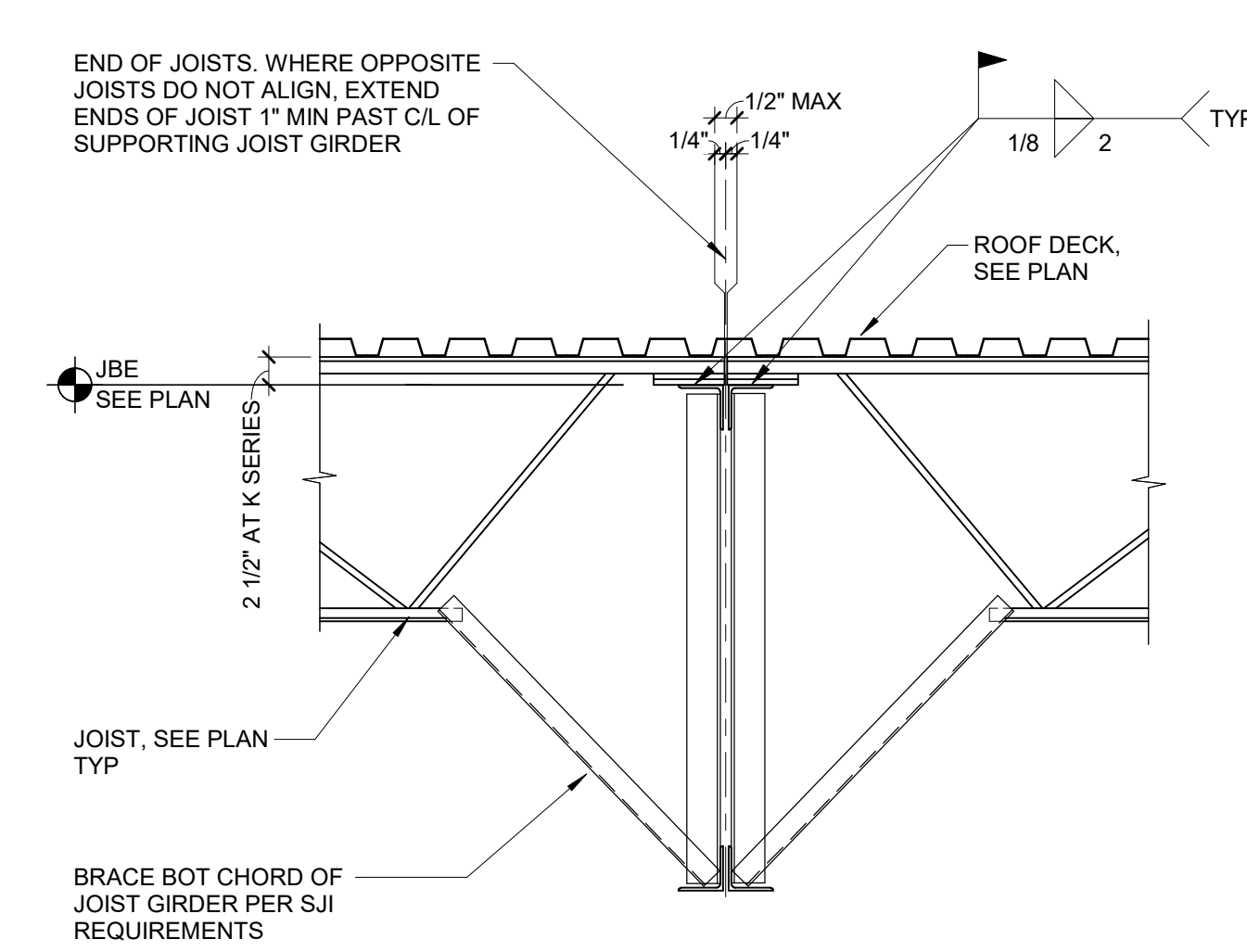


9
S611
TYP BOT FLANGE BM BRACE AT ROOF
3/4" = 1'-0"

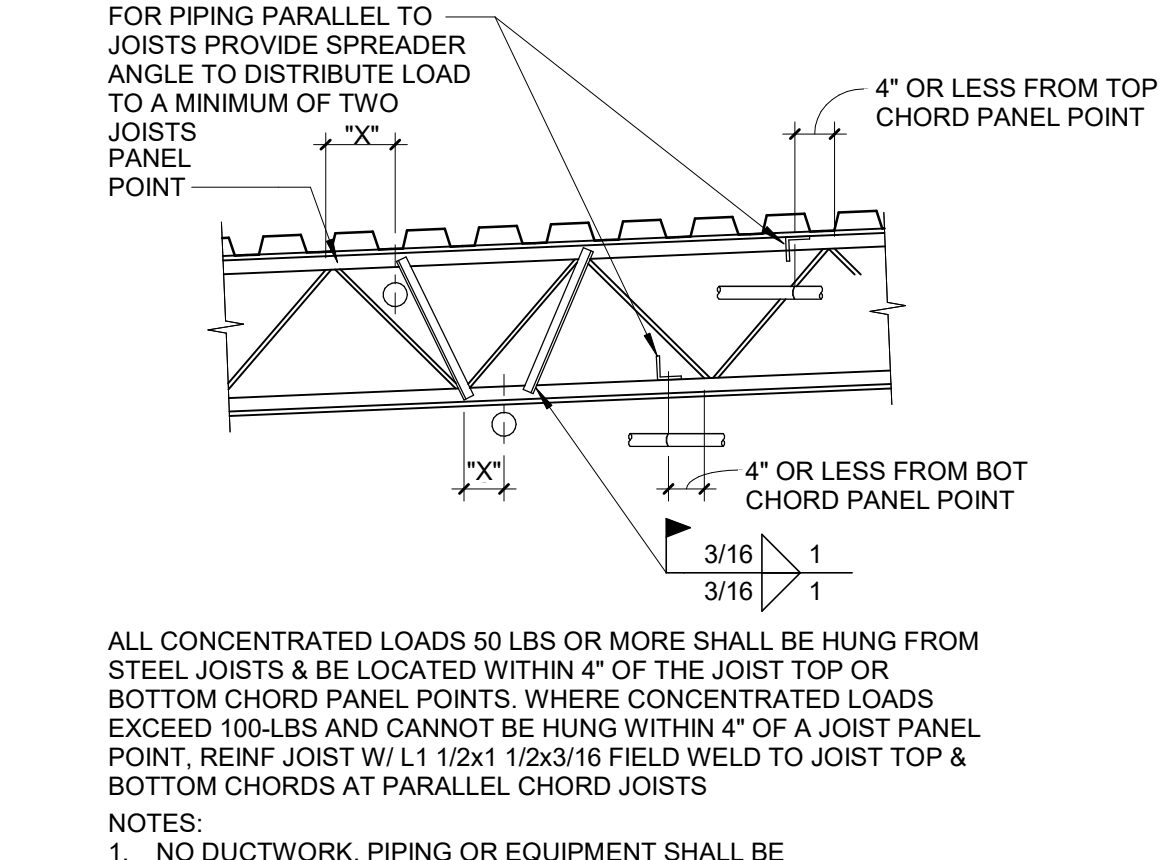
4
S611
TYPICAL EQUIPMENT CURB SUPPORT
NO SCALE



10
S611
TYPICAL ROOF JOIST AT BEAM
3/4" = 1'-0"



5
S611
TYPICAL HANGING PIPE SUPPORT
3/4" = 1'-0"



E

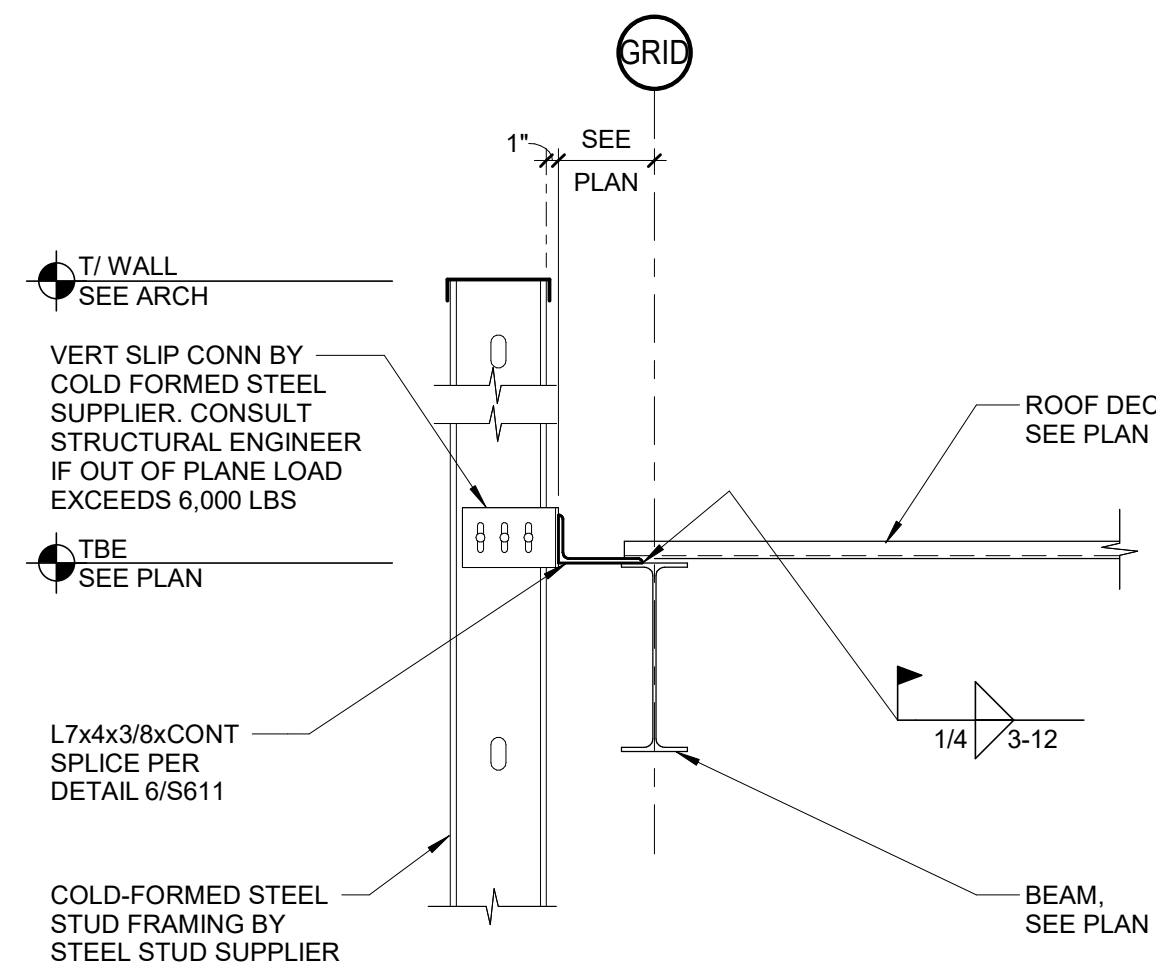
D

C

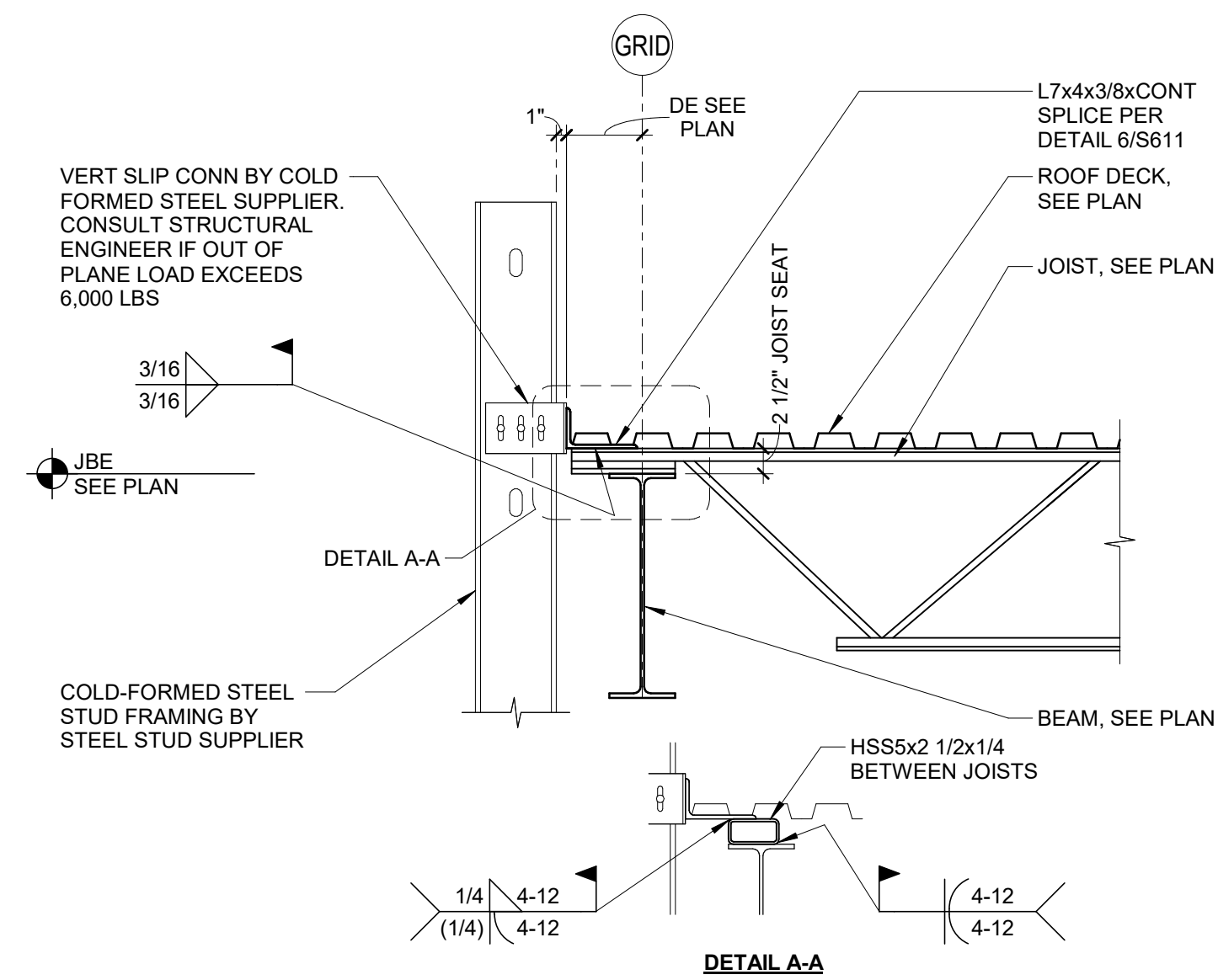
B

A

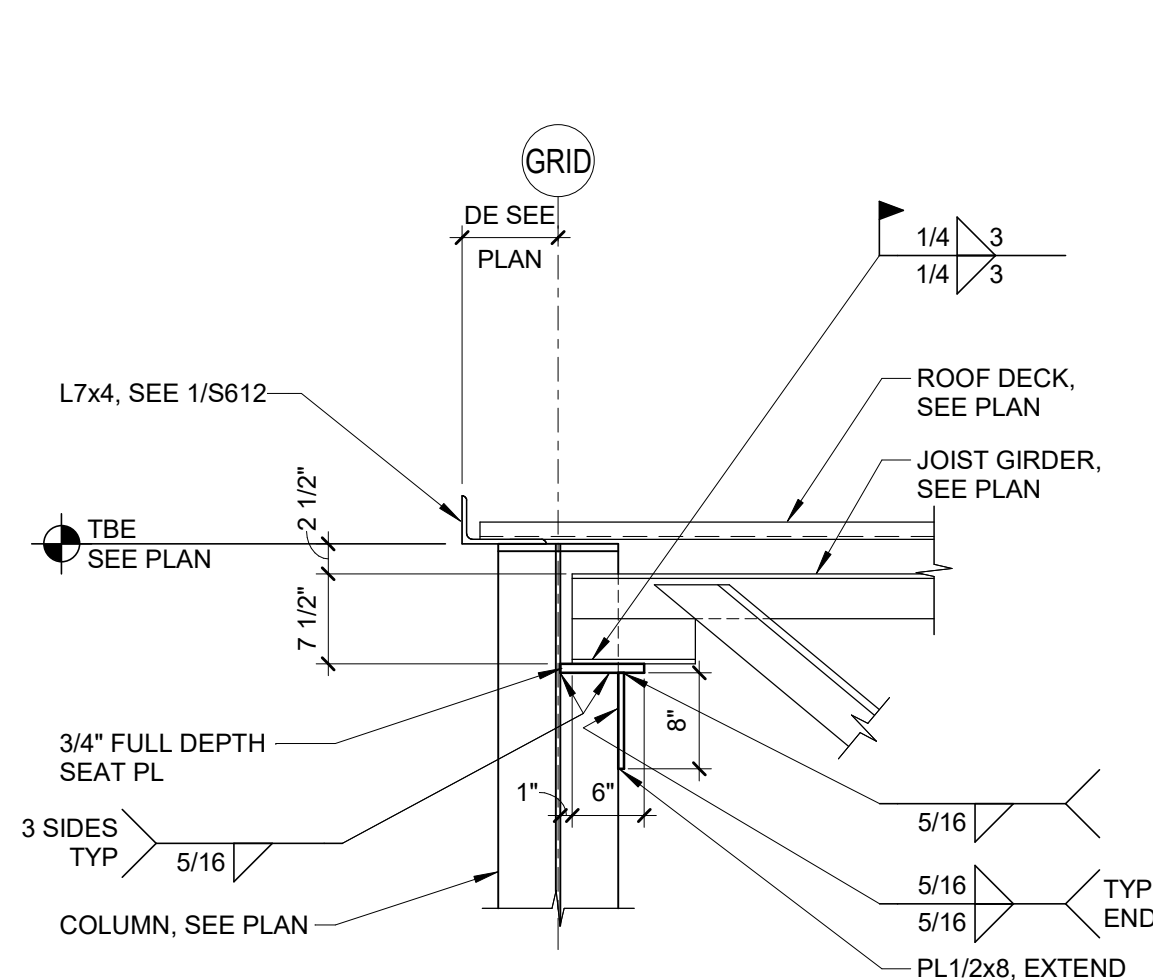
2/9/2026 11:36:14 AM



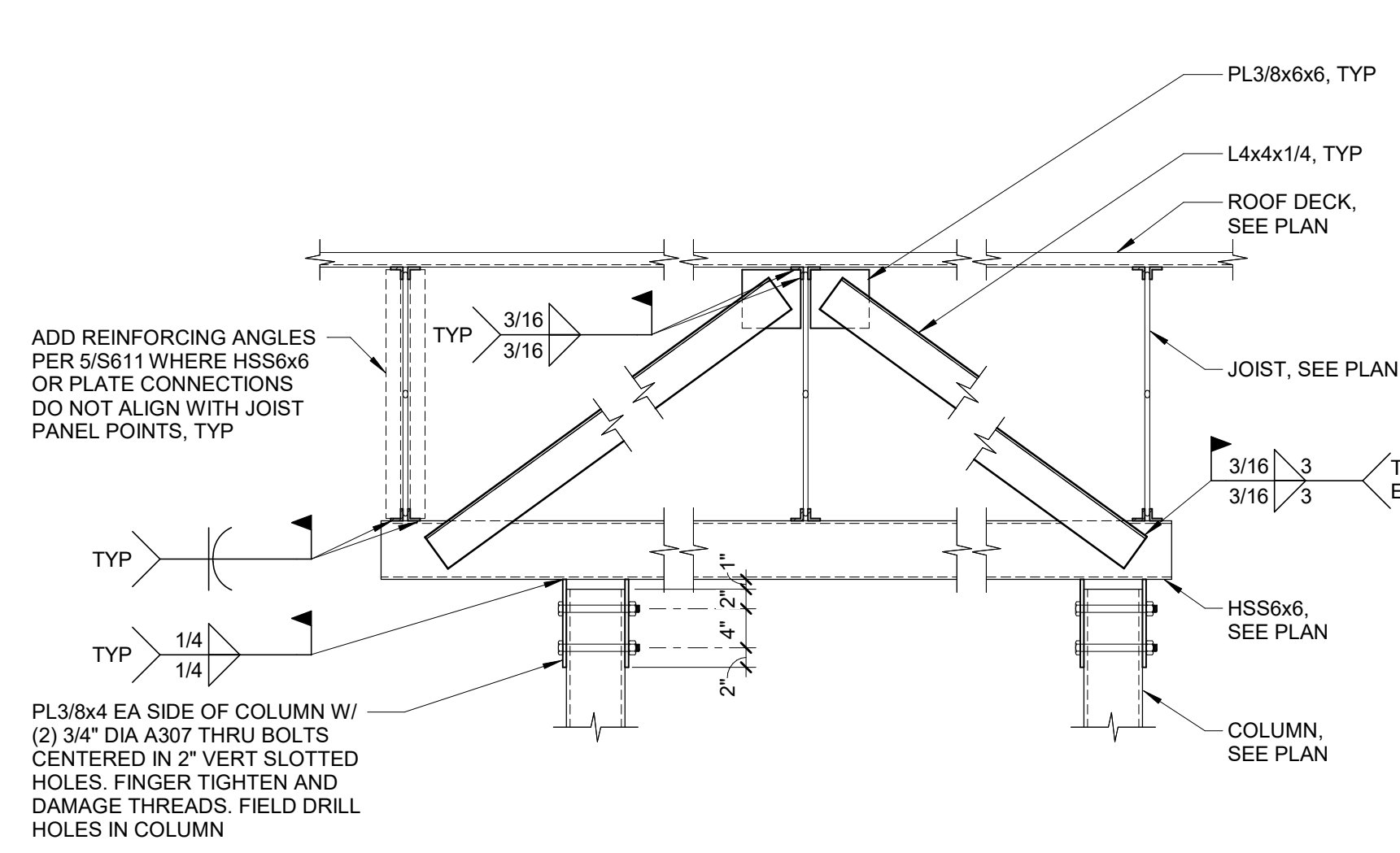
1 DECK TO WALL CONN AT ROOF
S612 3/4" = 1'-0"



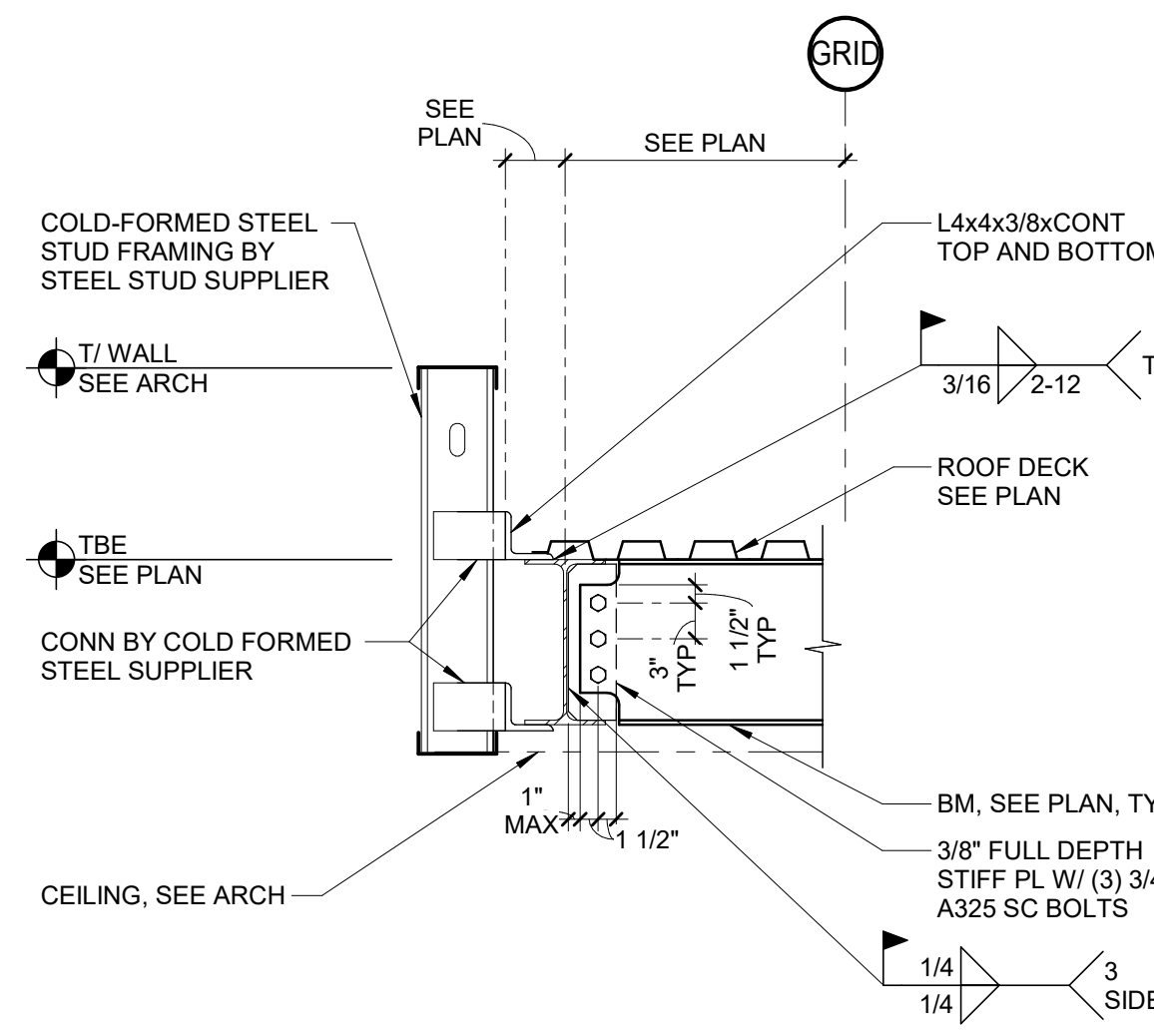
2 DETAIL
S612 3/4" = 1'-0"



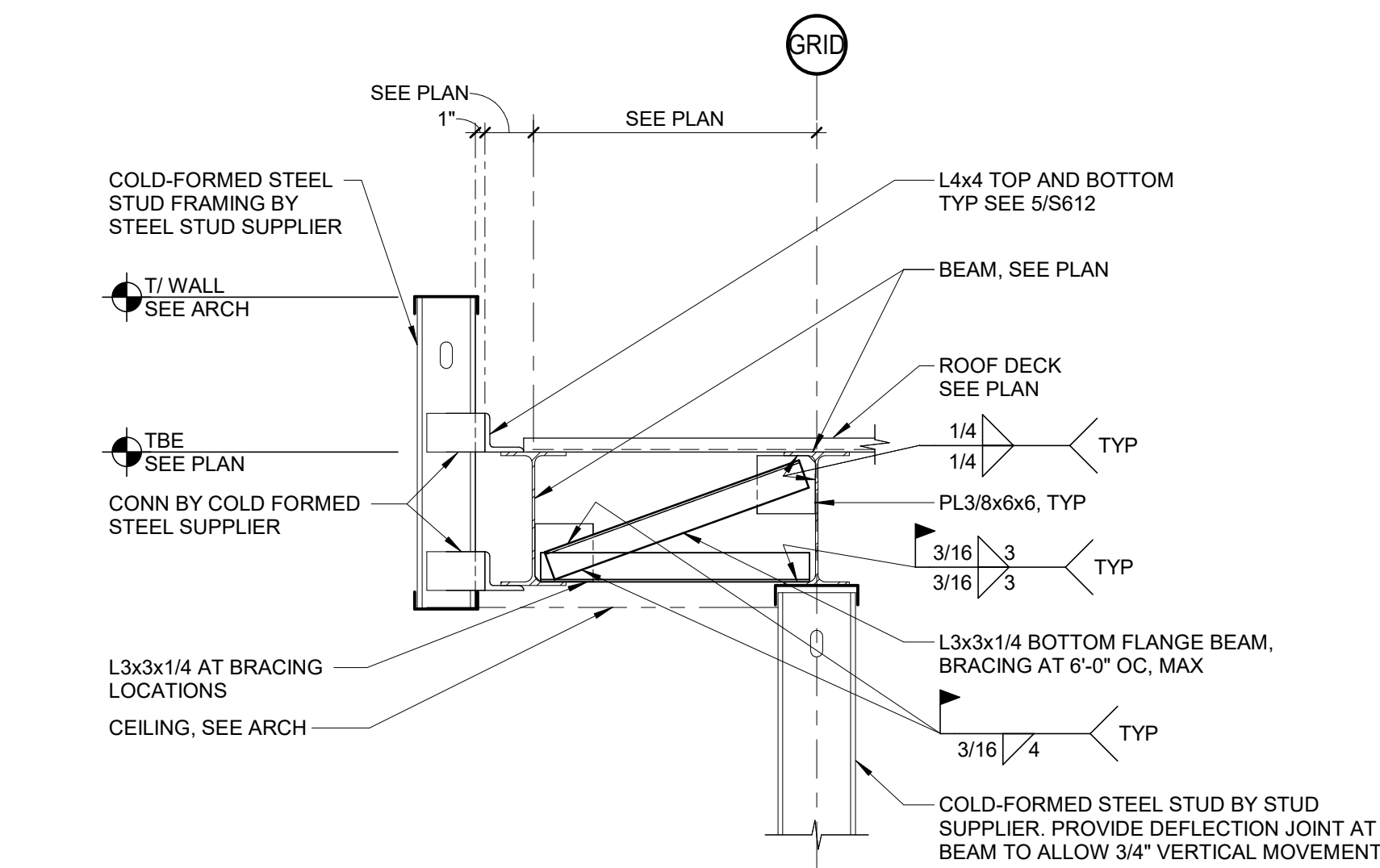
3 DETAIL
S612 3/4" = 1'-0"



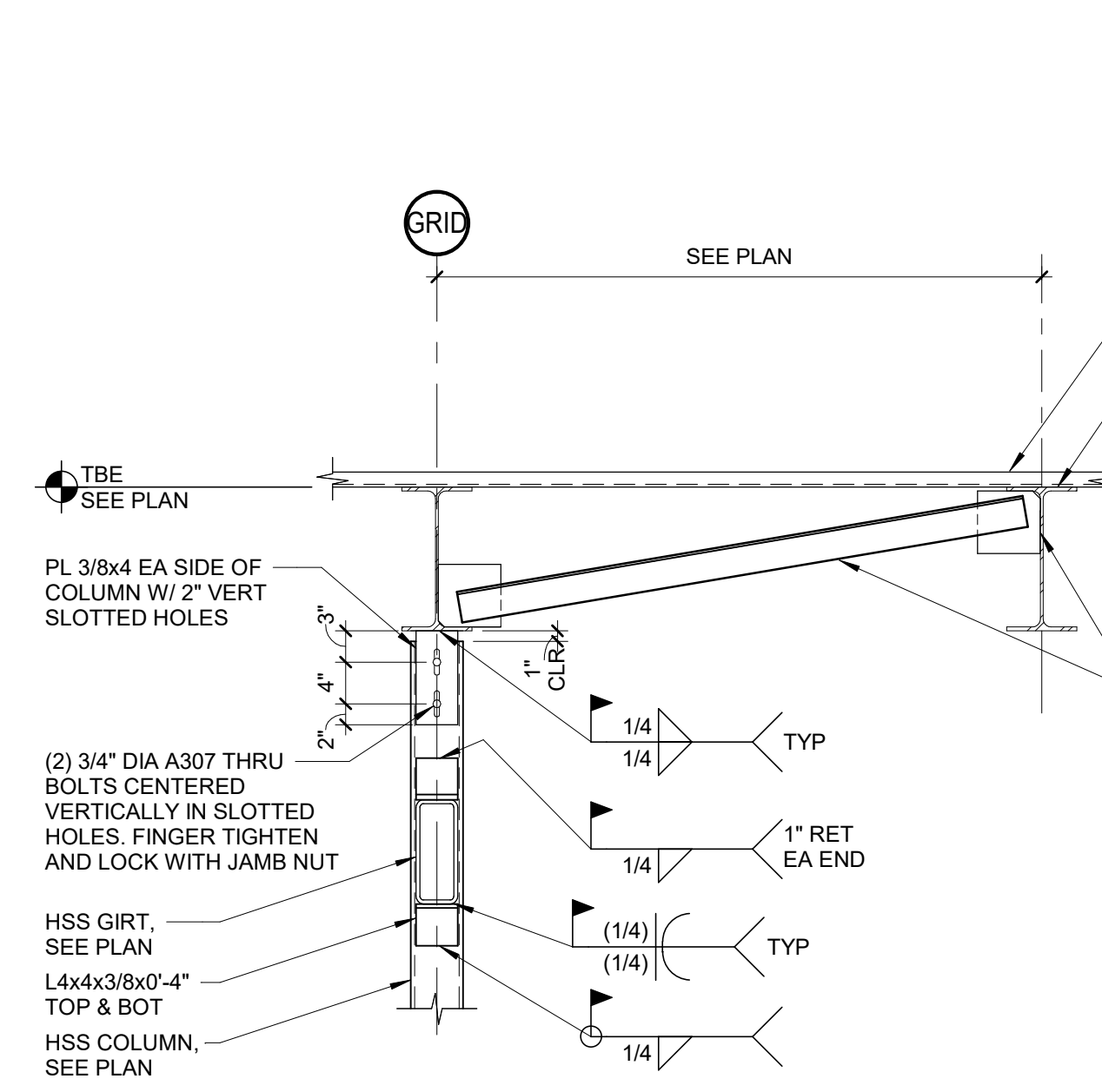
4 SECTION AT ELEVATOR COLUMNS
S612 3/4" = 1'-0"



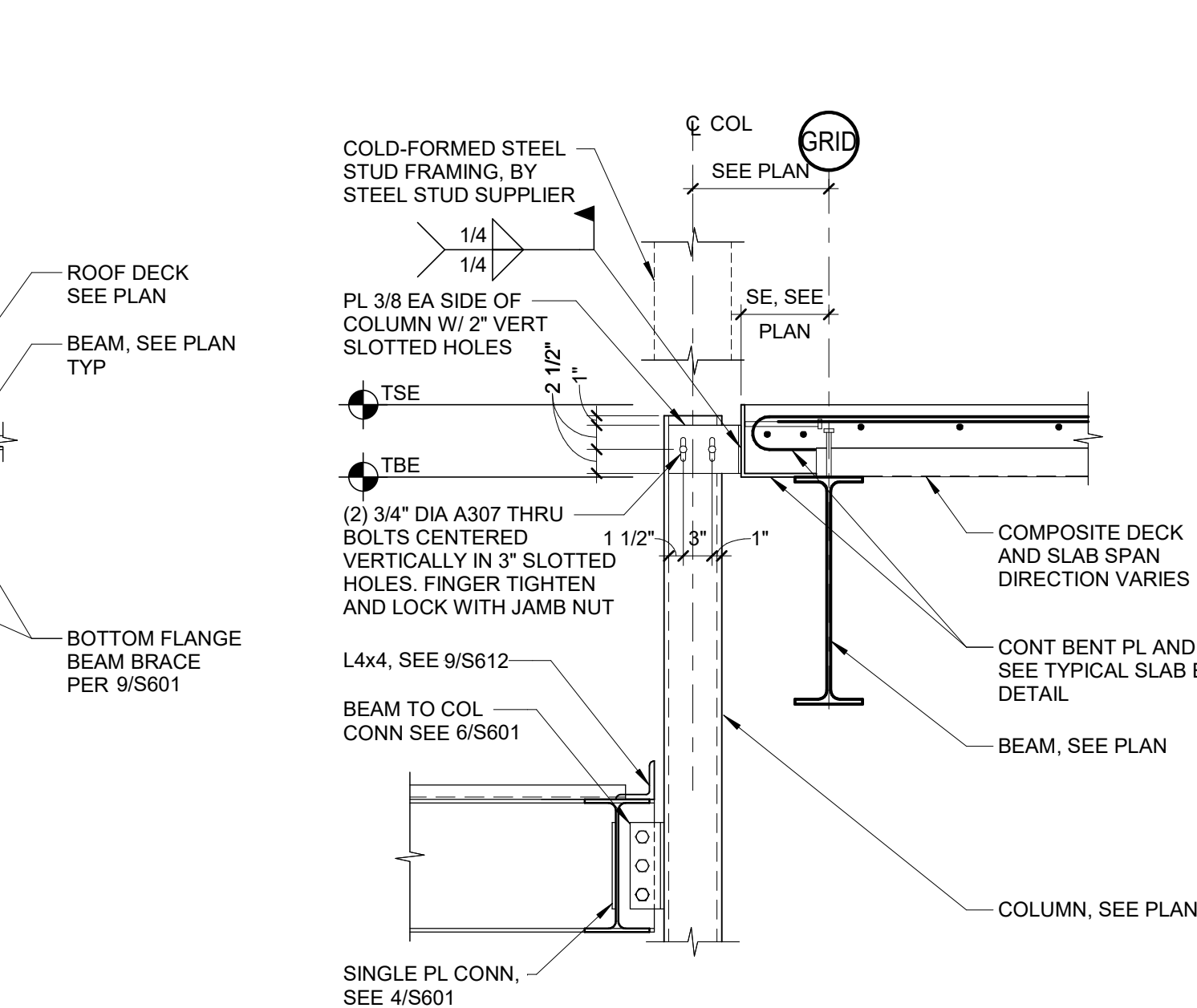
5 ROOF FRAMING DETAIL AT BISTRO
S612 3/4" = 1'-0"



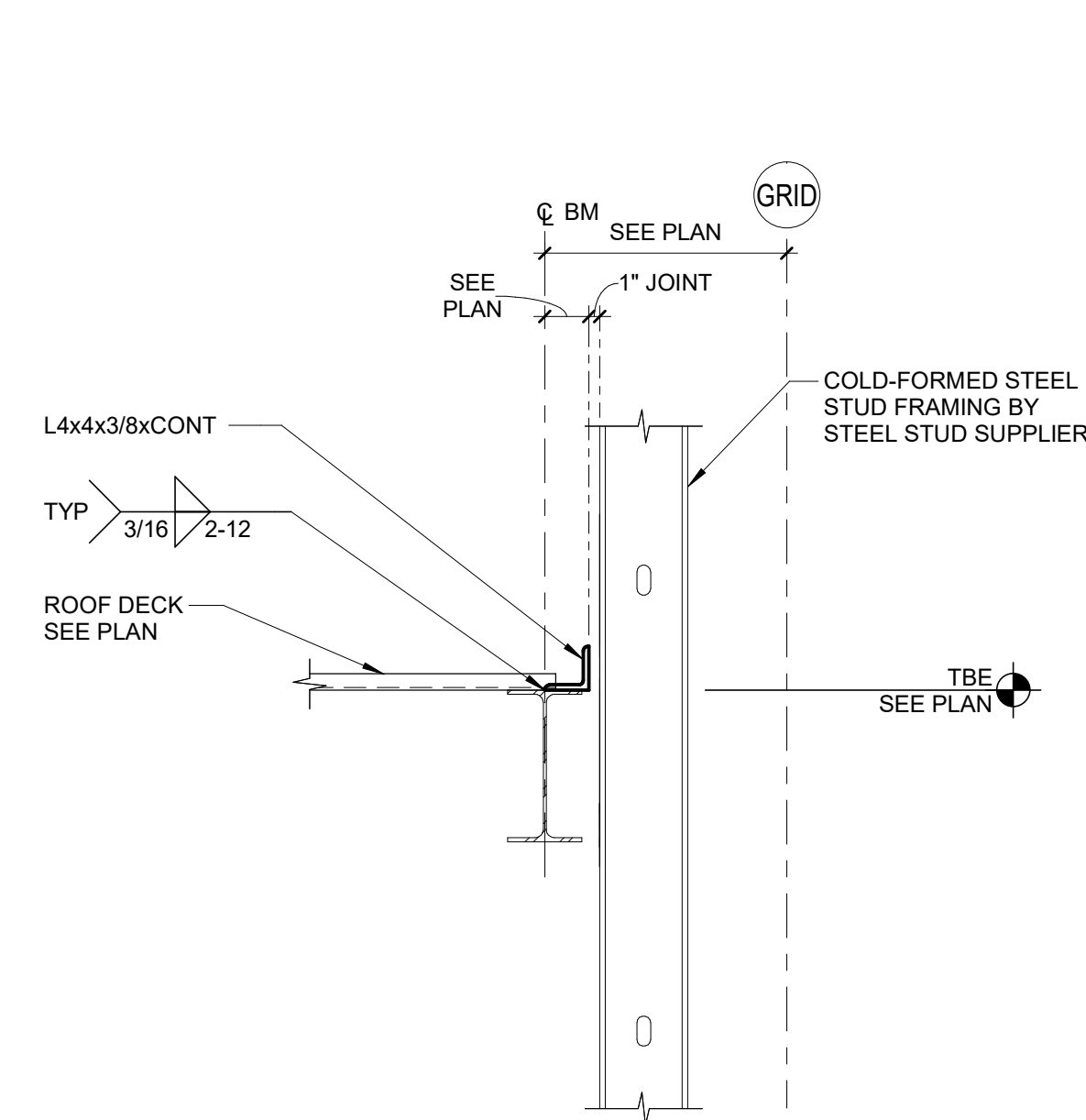
6 DECK TO WALL CONN AT BISTRO
S612 3/4" = 1'-0"



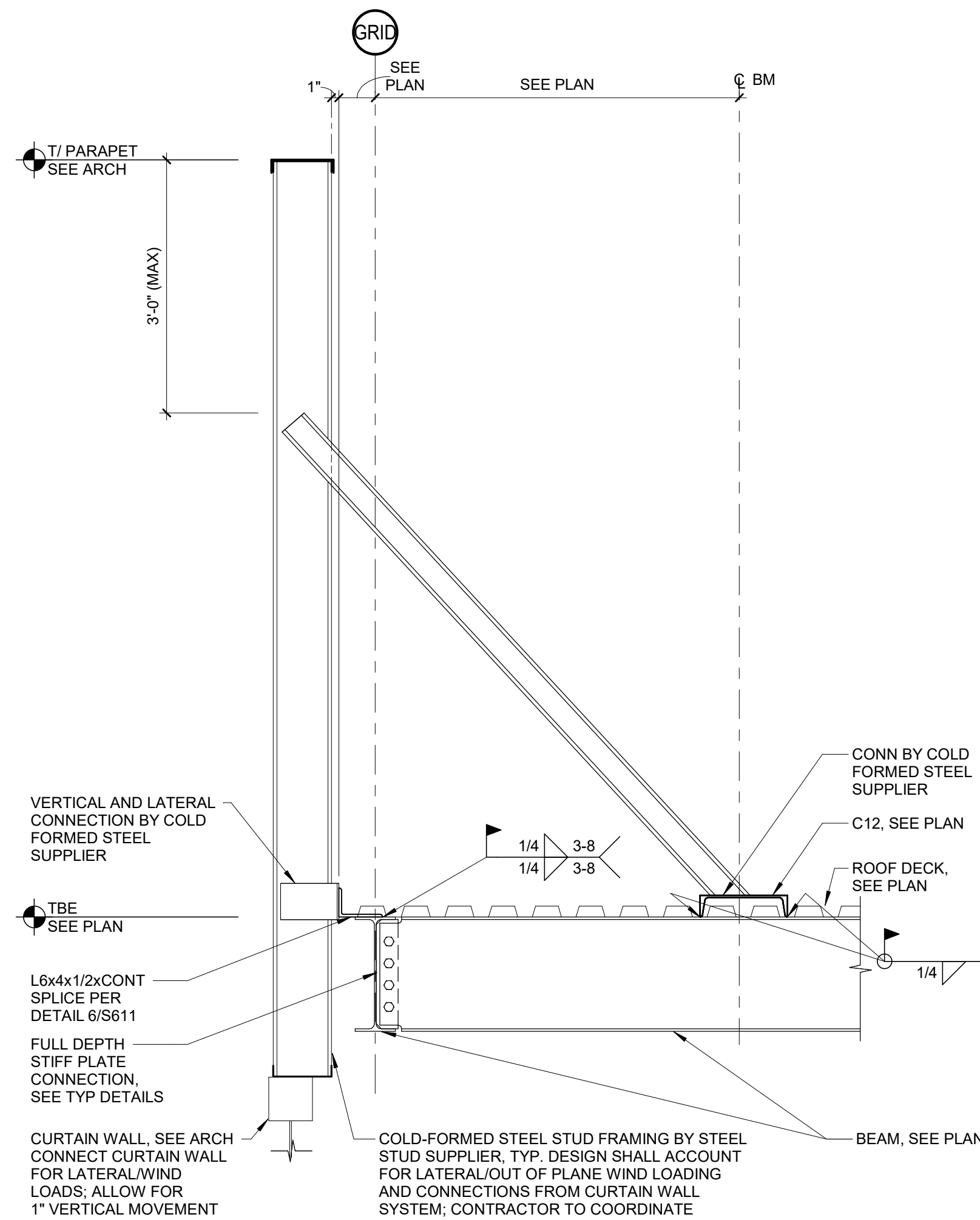
7 SECTION AT OVERHEAD DOOR
S612 3/4" = 1'-0"



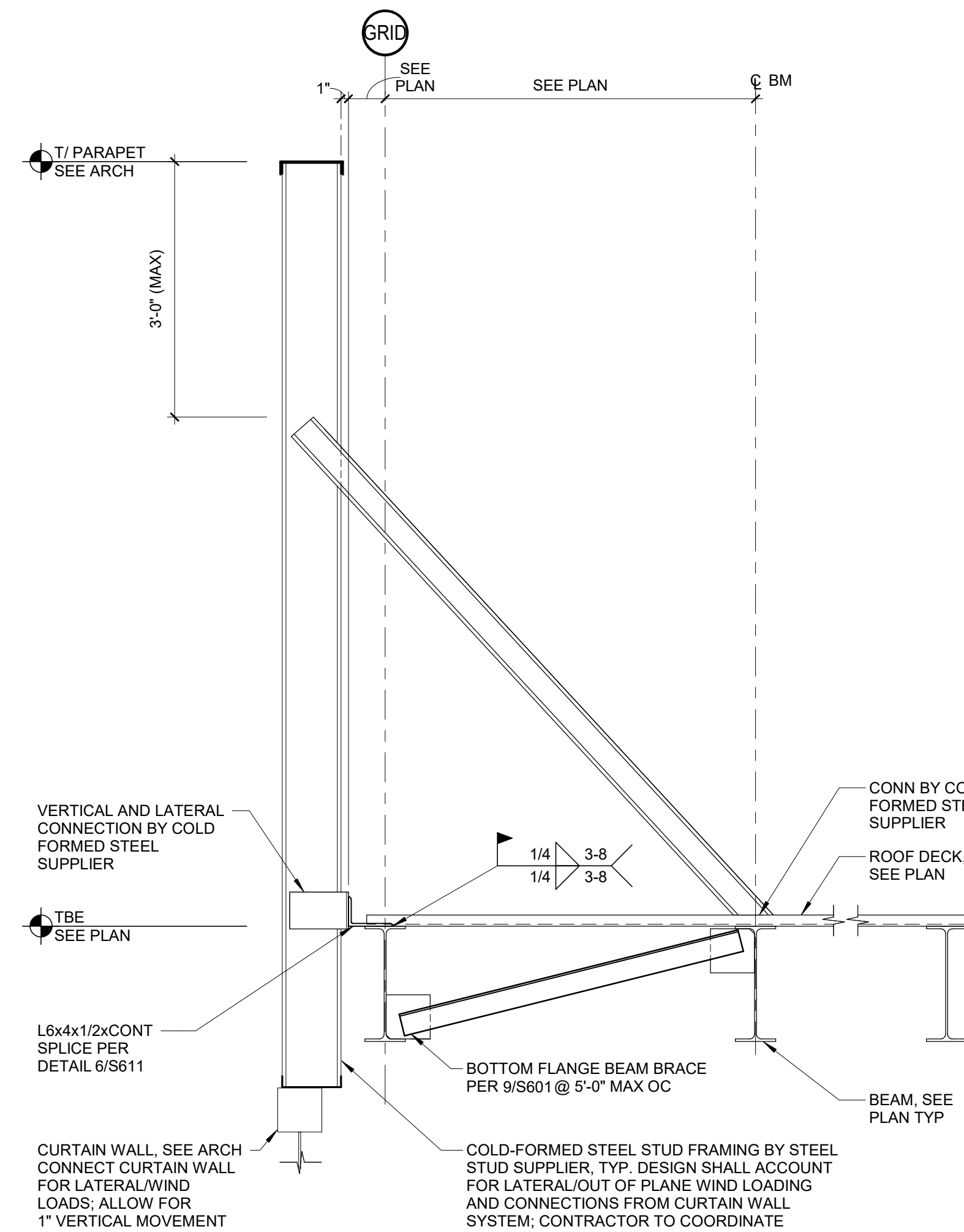
8 DETAIL AT BISTRO
S612 3/4" = 1'-0"



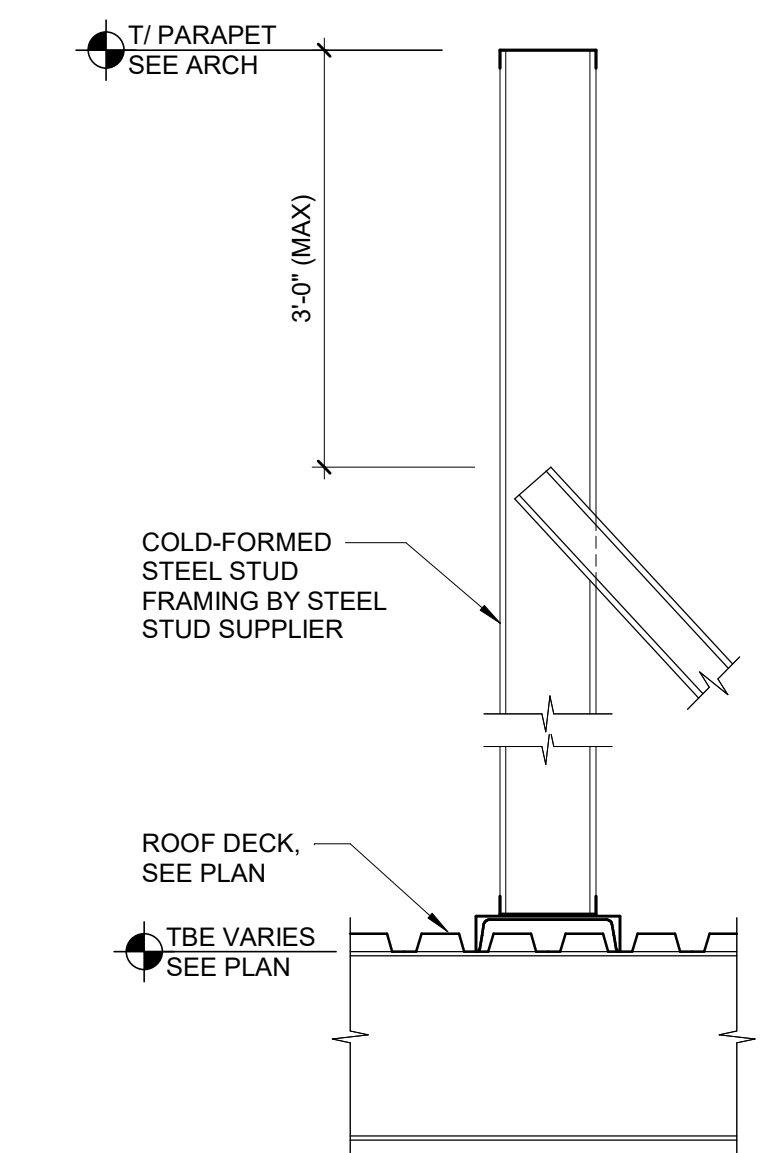
9 DETAIL AT BISTRO
S612 3/4" = 1'-0"



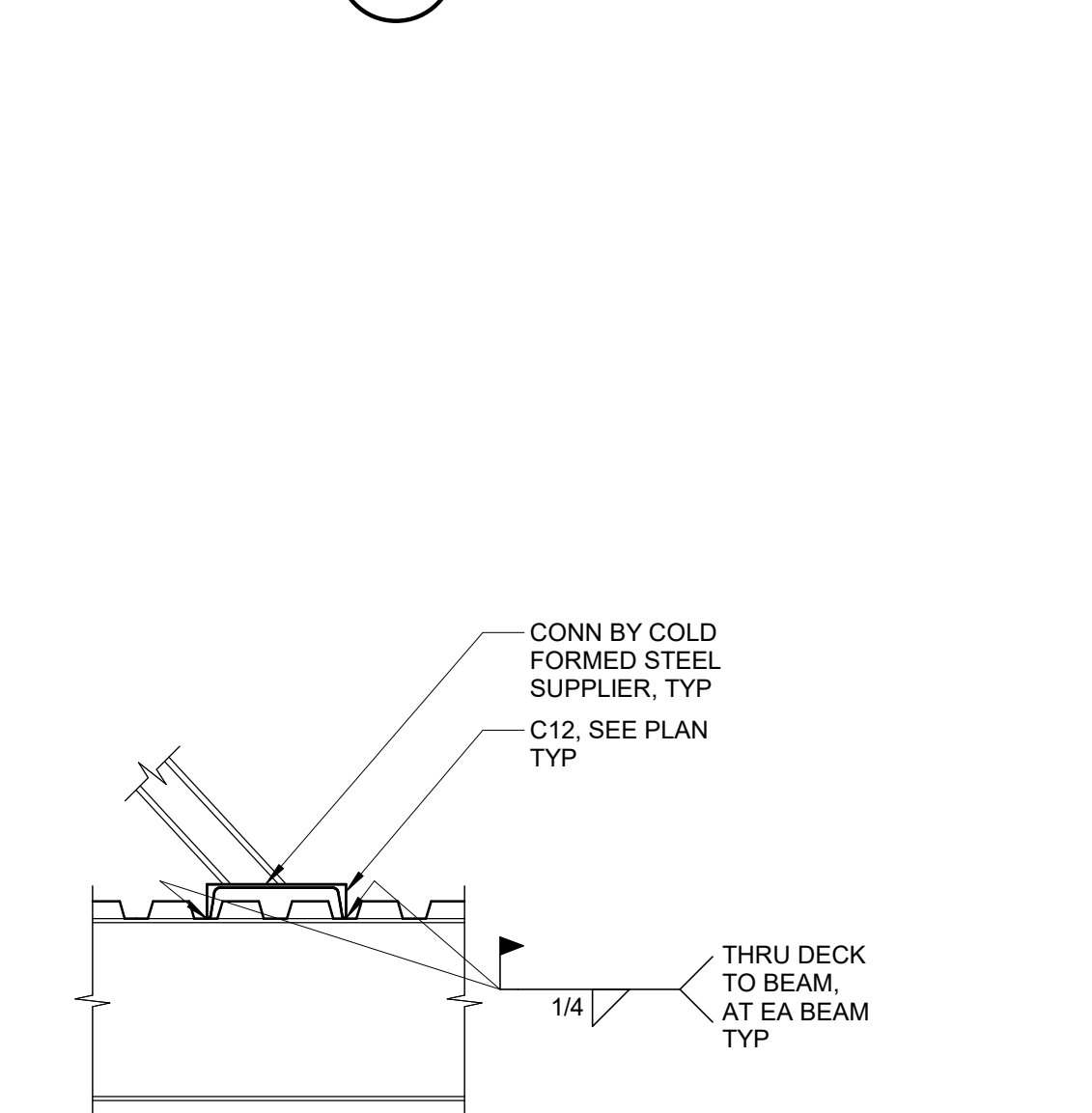
10 SECTION AT ENTRY ROOF
S612 3/4" = 1'-0"



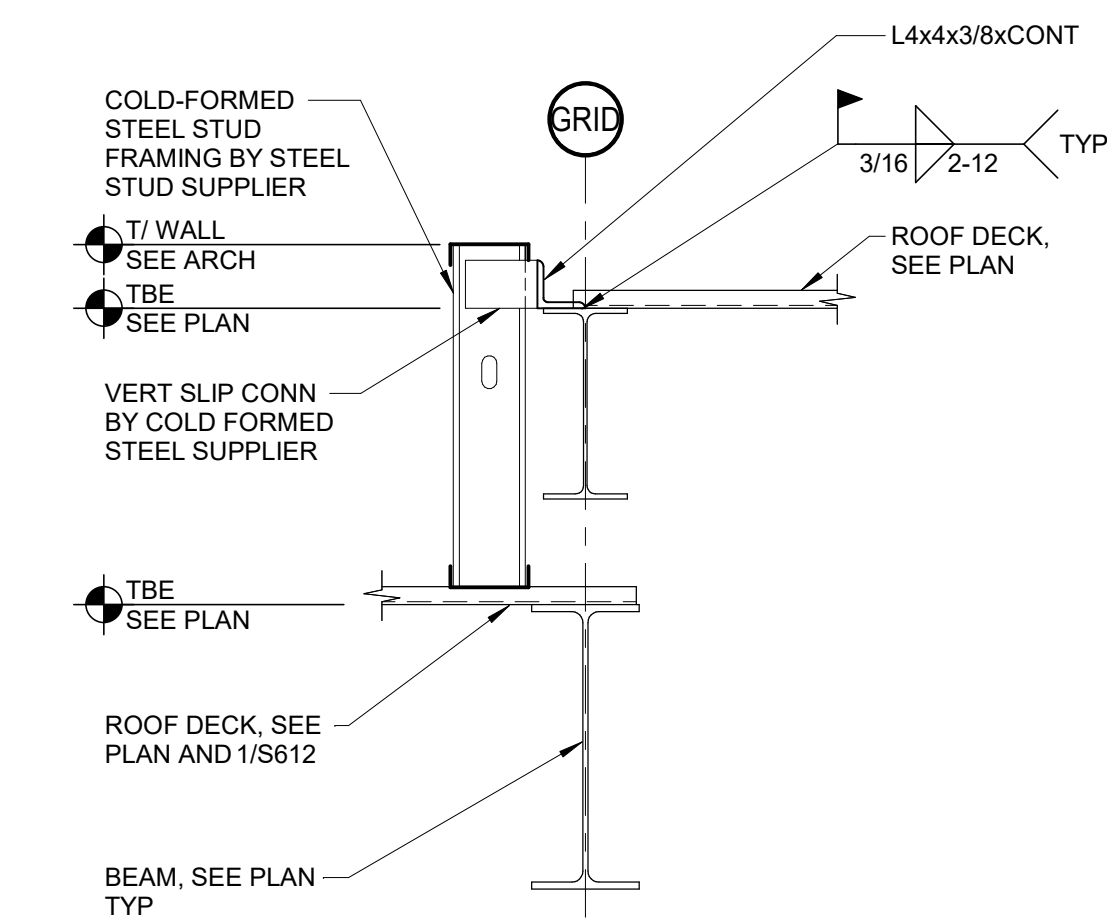
11 SECTION AT ENTRY ROOF
S612 3/4" = 1'-0"



12 DETAIL
S612 3/4" = 1'-0"



13 SECTION AT ENTRY ROOF
S612 3/4" = 1'-0"



14 SECTION
S612 3/4" = 1'-0"

LIFE TIME
HEALTHY WAY OF LIFE
2900 CORPORATE PLACE CHANHASSEN, MN 55317

CONSULTANT
LIFE TIME
PROPERTY DEVELOPMENT



PROJECT INFORMATION

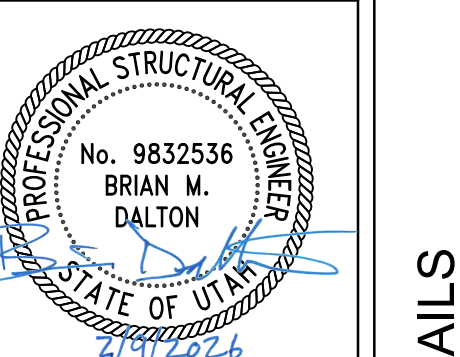
Holladay, UT
1975 East Rodeo Drive
Holladay, Utah

DRAWING LOG

REV DATE DESCRIPTION

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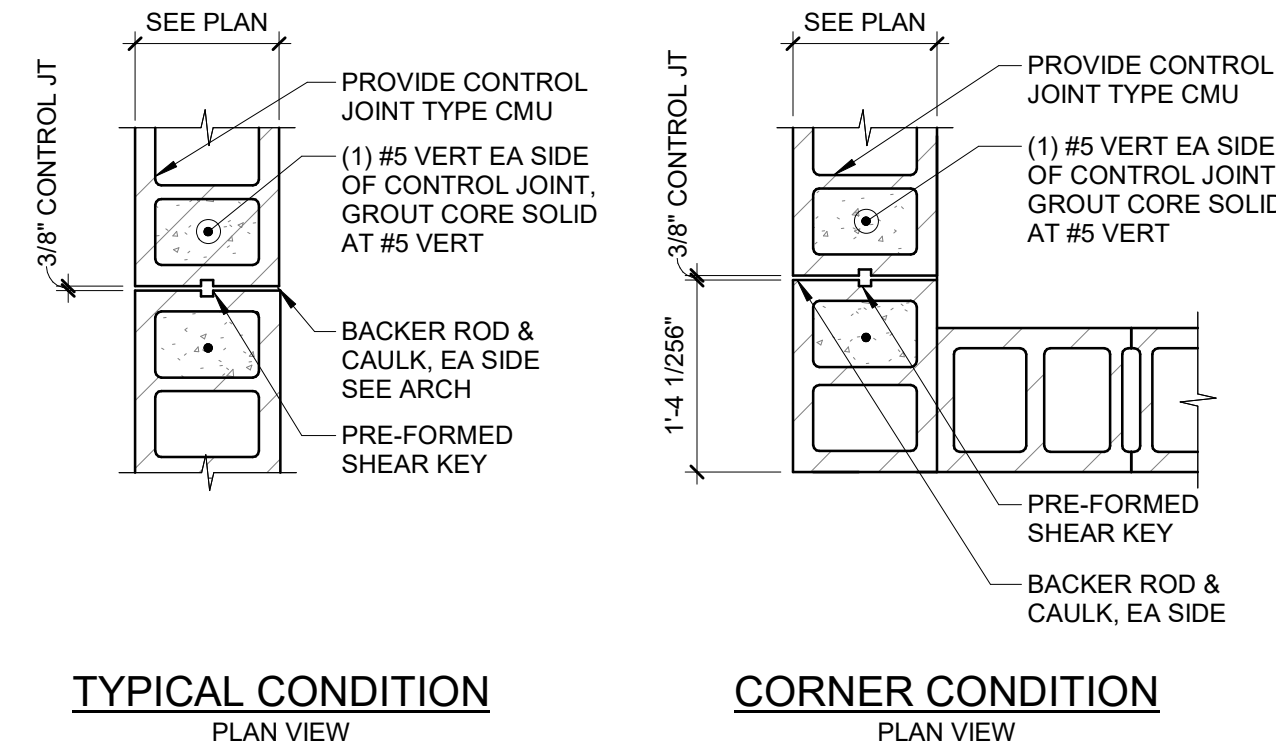
ISSUE DATE: 02/09/26

SHEET NO.

S612

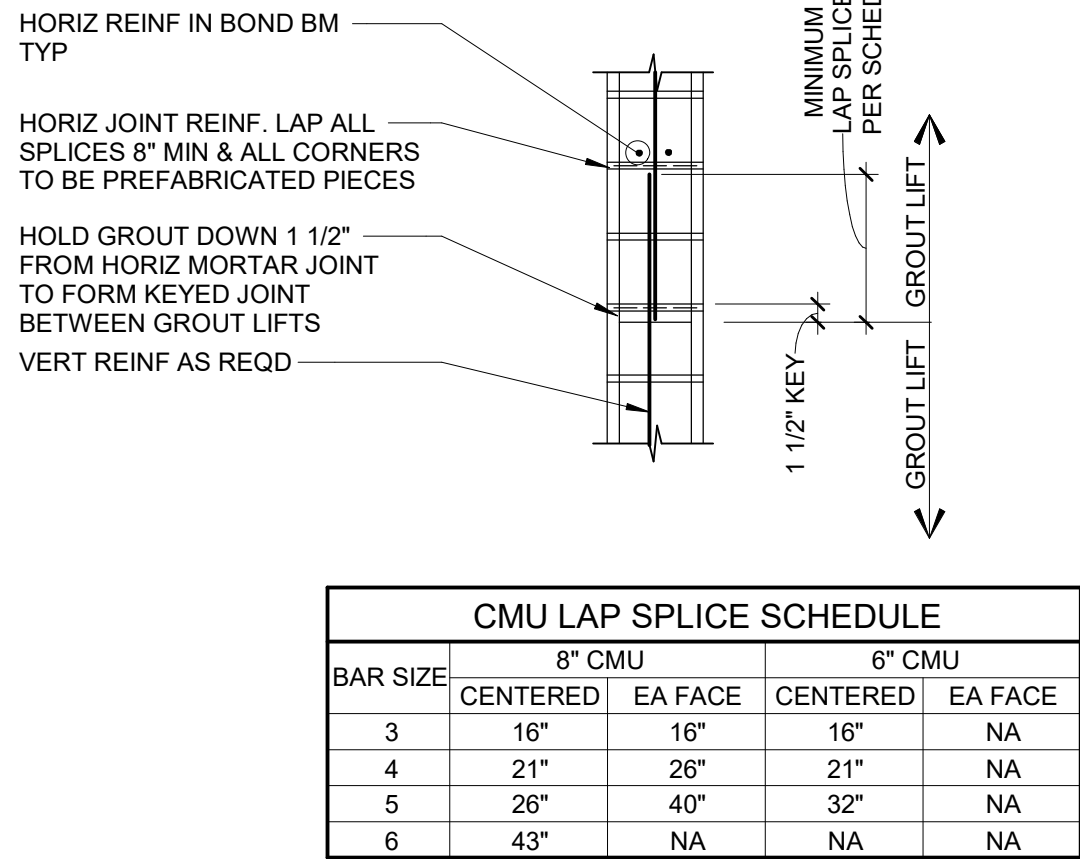
PUBLICATIONS
ISSUE FOR PERMIT/BID

ROOF FRAMING DETAILS



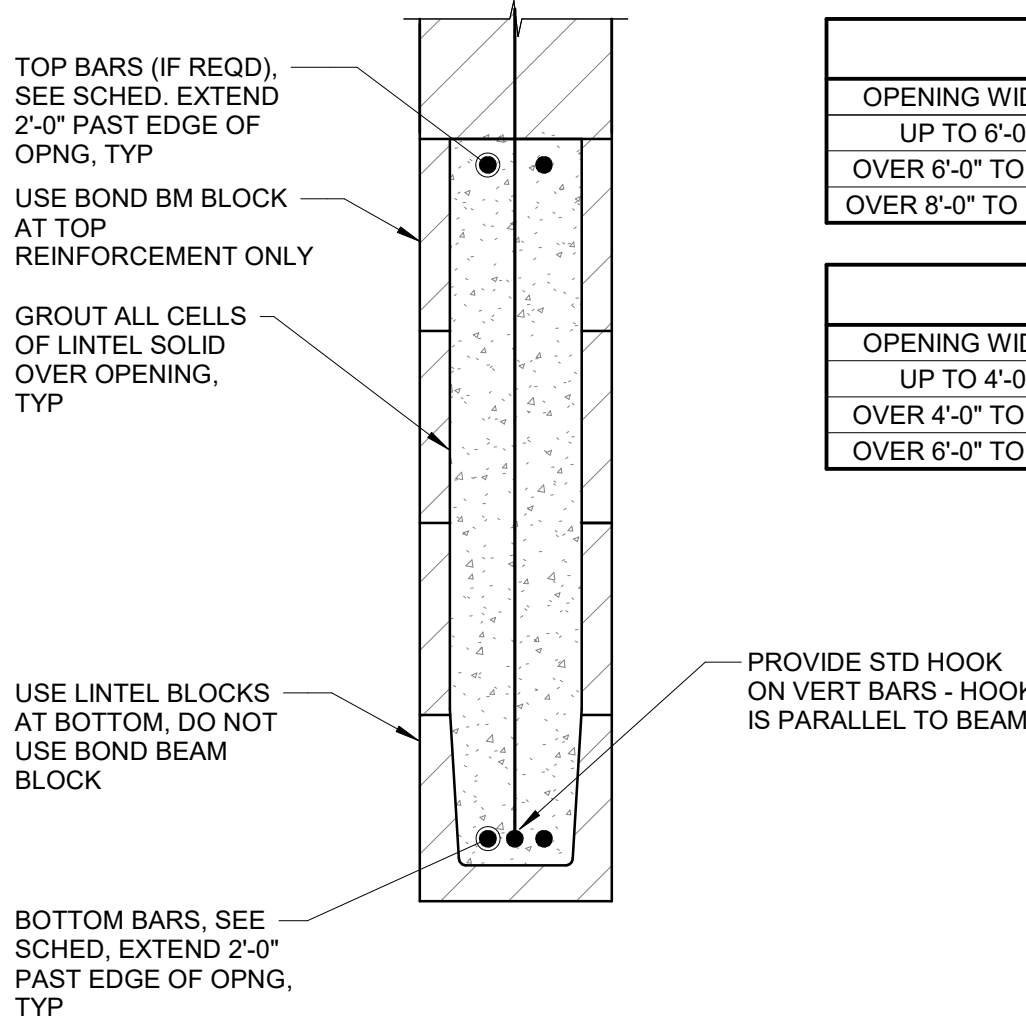
TYPICAL CONDITION
PLAN VIEW

CORNER CONDITION
PLAN VIEW



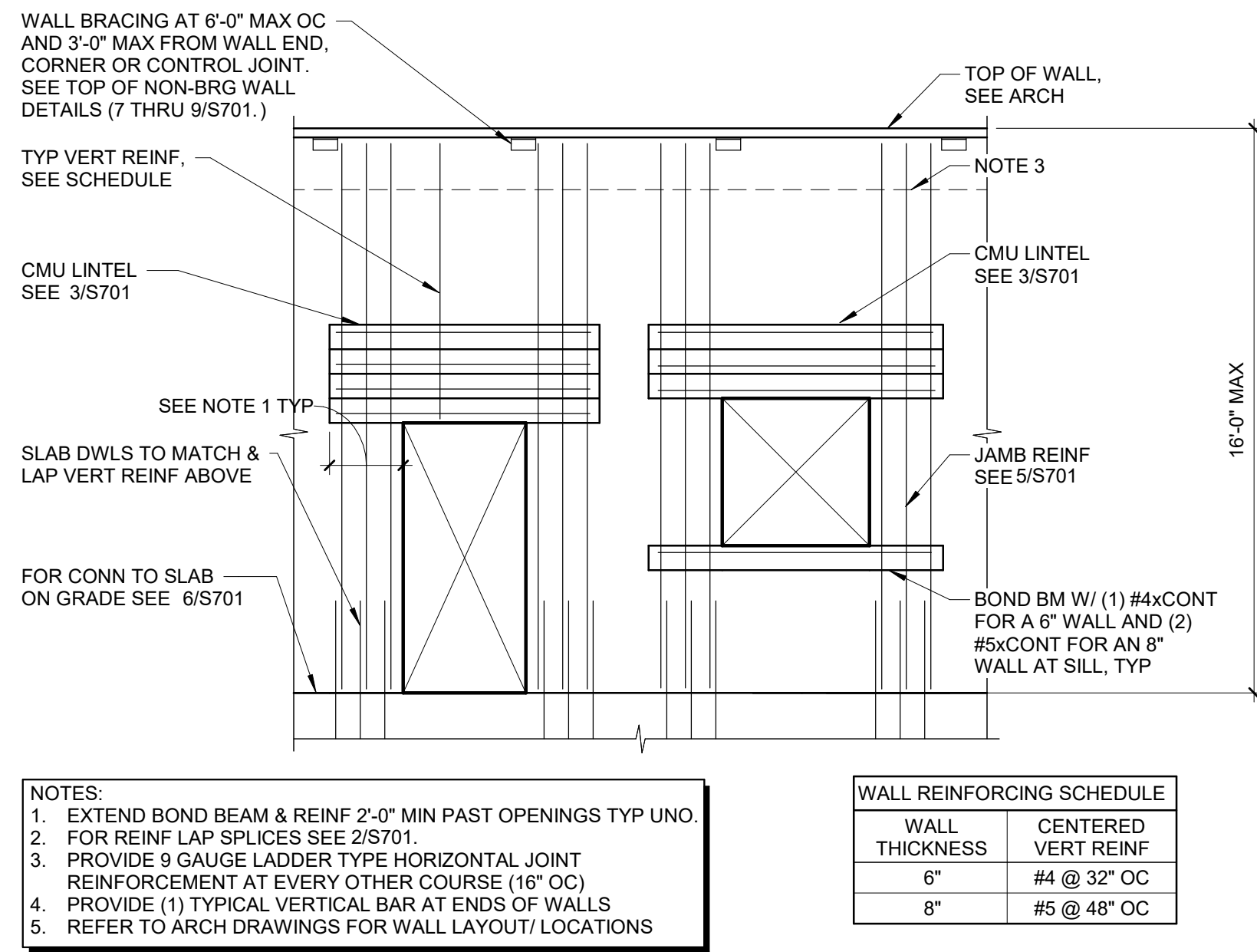
2
S701
12" = 1'-0"

NON-LOAD BEARING CMU REINF
LAP SPlice REQUIRMENTS



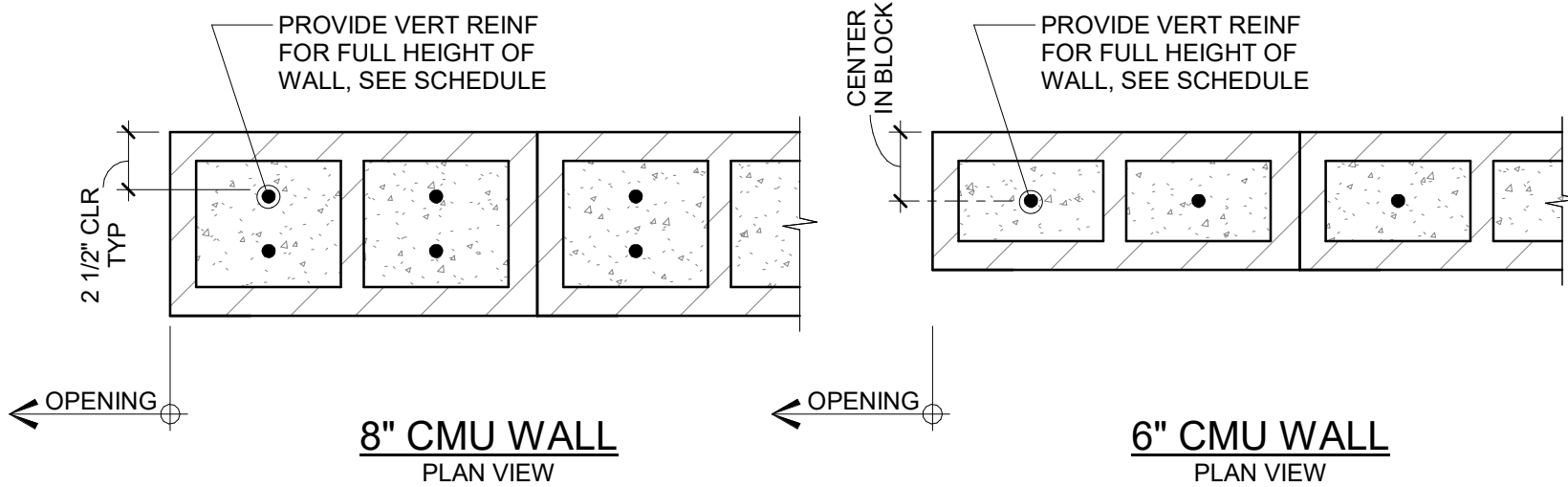
3
S701
1 1/2" = 1'-0"

TYP NON-LOAD BEARING CMU LINTEL BEAM
DETAIL & SCHED



4
S701
1/4" = 1'-0"

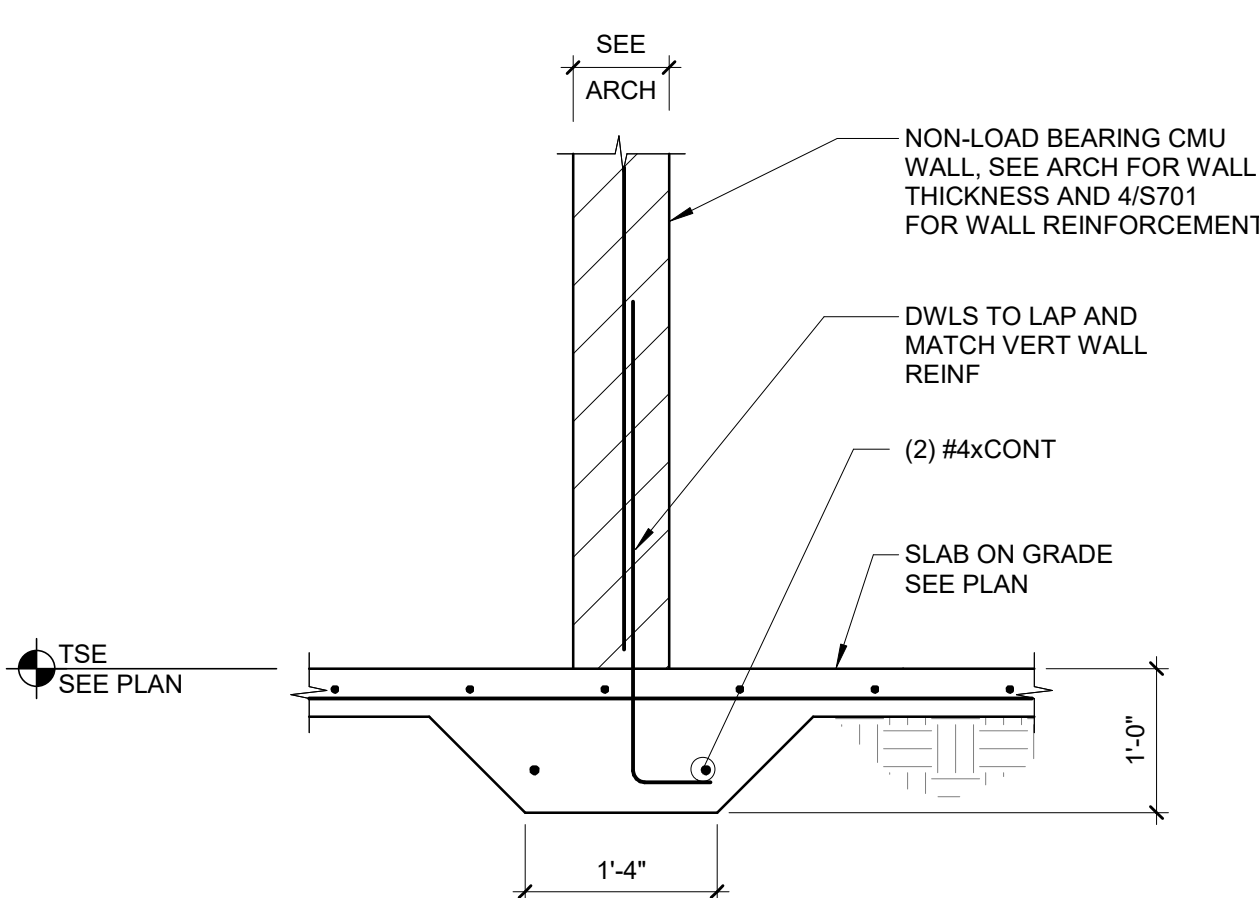
TYP NON- LOAD BEARING CMU WALL
AND OPENING REINF



JAMB REINFORCING SCHEDULE		
OPENING WIDTH	FULL HEIGHT JAMB REINFORCING	
	8" CMU	6" CMU
UP TO 4'-0"	(2) #5 VERT (1 CORE W/ 1 BAR EACH FACE OF CORE)	(2) #4 VERT (2 CORES W/ 1 BAR EACH CORE)
OVER 4'-0" TO 8'-0"	(4) #5 VERT (2 CORES W/ 1 BAR EACH FACE OF CORE)	(3) #4 VERT (3 CORES W/ 1 BAR EACH CORE)
OVER 8'-0" TO 12'-0"	(6) #5 VERT (3 CORES W/ 1 BAR EACH FACE OF CORE)	(4) #4 VERT (4 CORES W/ 1 BAR EACH CORE)

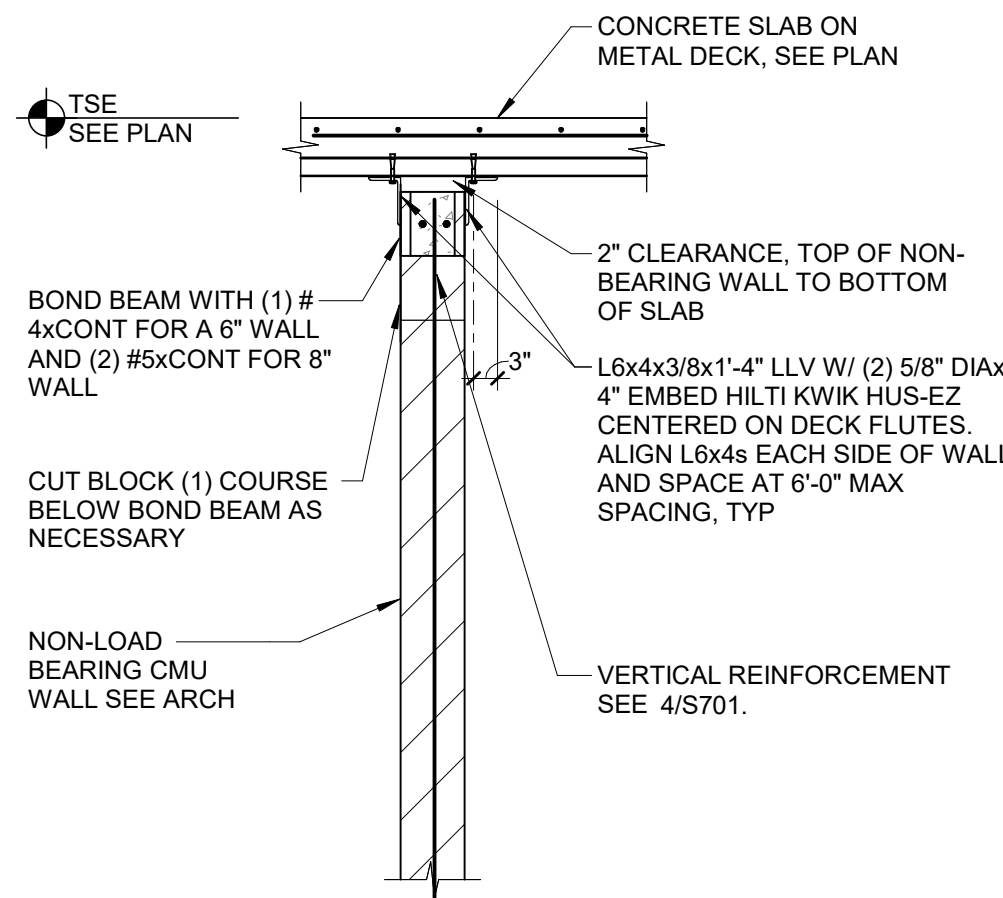
5
S701
1 1/2" = 1'-0"

TYP NON-LOAD BEARING CMU JAMB
REINFORCING DETAIL & SCHEDULE



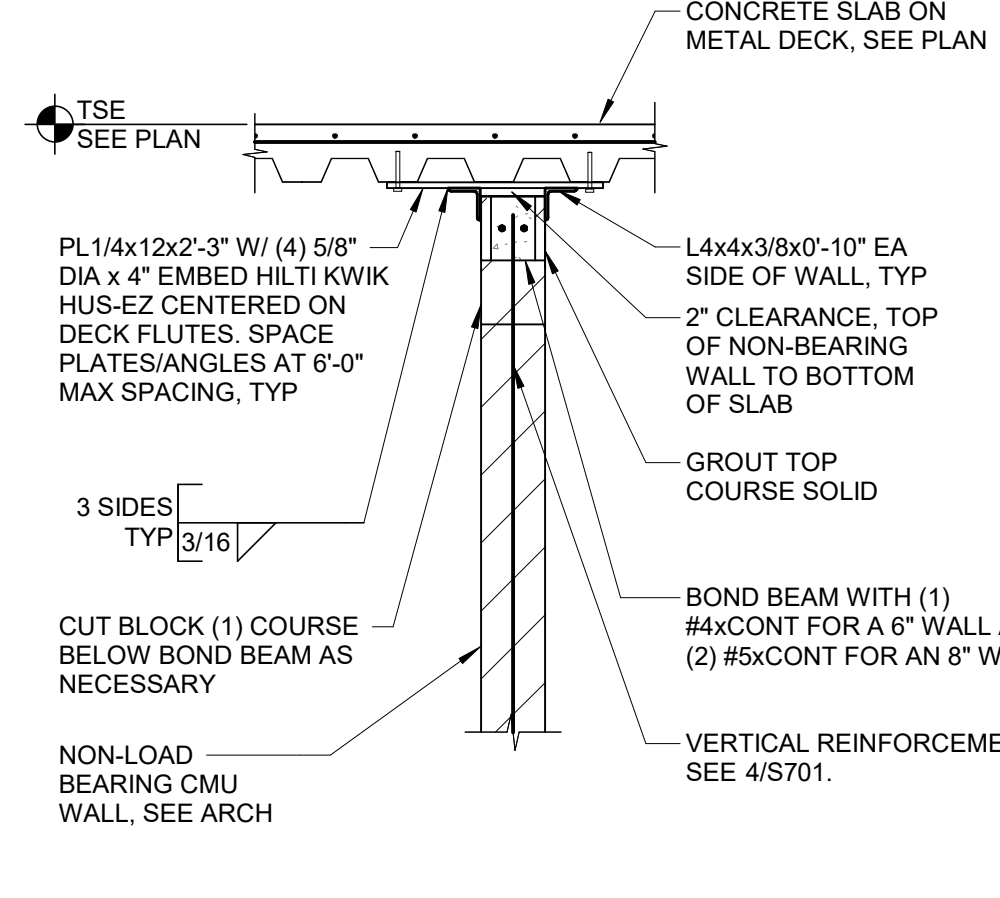
6
S701
3/4" = 1'-0"

THICKENED SLAB AT INTERIOR
NON-LOAD BEARING WALL



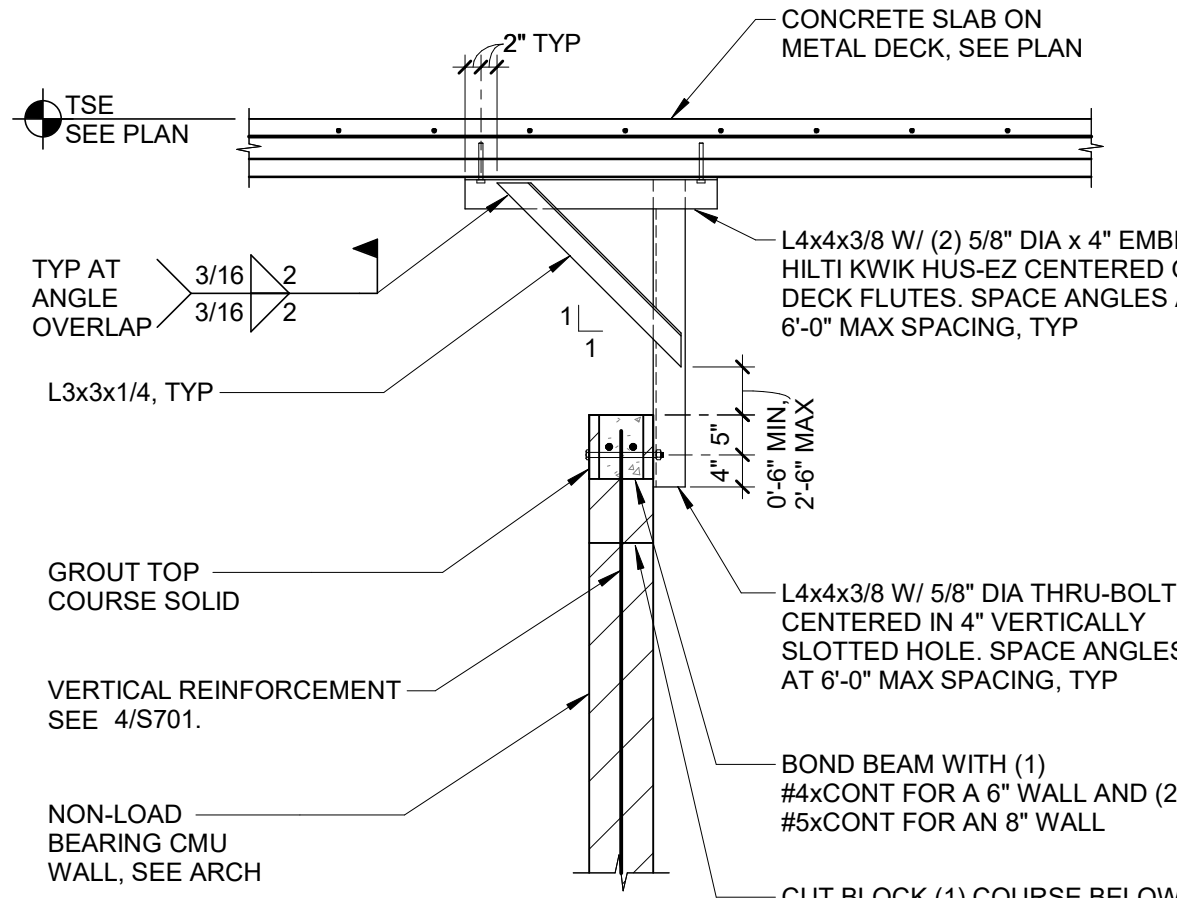
7
S701
1/2" = 1'-0"

TOP OF NON-LOAD BEARING
CMU WALL PERP TO SLAB



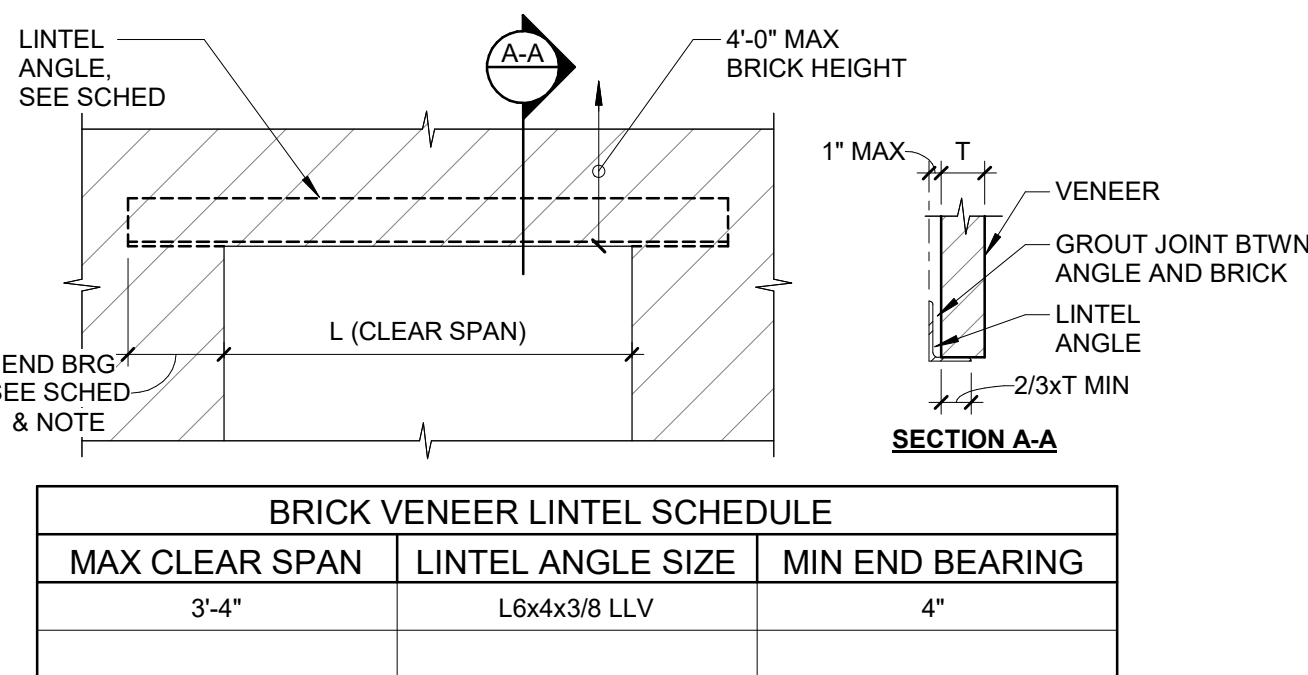
8
S701
1/2" = 1'-0"

TOP OF NON-LOAD BEARING
CMU WALL PARALLEL TO SLAB



9
S701
1/2" = 1'-0"

TOP OF NON-LOAD BEARING
CMU WALL - WALL HELD DOWN



- NOTES
- WHERE BEARING ENDS CROSS VENEER CONTROL JOINTS, DEBOND HORIZONTAL LEG BY WRAPPING W/ 10 MIL POLY TOP & BOTTOM
 - LINTELS TO BE GALVANIZED G90

10
S701
3/4" = 1'-0"

TYP BRICK VENEER
LOOSE LINTEL DETAIL