



MHTN ARCHITECTS MHTN Architects, Inc. 280 South 400 West Suite 250 Salt Lake City, Utah 84101 Telephone (801) 595-6700

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PCSD-2025507-123005-04

GAL ORNAMENTAL FENCE GATE - DOUBLE LEAF 3/4" = 1'-0"







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1 1/2" = 1'-0"

1 1/2" = 1'-0"

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2X4 PRESSURE TREATED NAILER INFILL. - 2" RECYCLED FOAM BOARD

ATTACHED TO CONCRETE WITH 3/8" X 3" LAG BOLTS WITH EXPANSION SHIELDS OR EXPANSION BOLTS AT 2' O.C. TOP OF NAILER = DEPTH OF

- 1" COMPACTED CLEAN WASHED

ANGULAR STONE, 8" DEEP - GEOTEXTILE SEPERATION FABRIC

- COMPACTED SANDY SUBGRADE

PCSD-2025507-321823-03

– (2) #4 BAR CONTUNUOUS

(E4) THICKENED EDGE SLAB / SYNTHETIC TURF

4

BACKSTOP WALL BOLTS AS PER SPECS. TOP OF NAILER = DEPTH OF INFILL. DEPTH OF INFILL VARIES BY MANUFACTURER, SEE SPECIFICATIONS → LIGHT POLE BY OTHERS 2'-0" - LIGHT POLE CONCRETE BASE - GROUND SLEEVE - POST FOOTING SYNTHETIC TURF AT LIGHT POLE BASE TYP. PCSD-2025507-116833-1 PCSD-2025507-116833-18 1 1/2" = 1'-0"

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C 2 X 4 RECYCLED PLASTIC NAILER ATTACHED

WITH EXPANSION SHIELDS OR EXPANSION

TO CONCRETE WITH 3/8" X 3" LAG BOLTS

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E1 BATTERS EYE FENCING PLAN

3/4" = 1'-0" PCSD-2025507-323113-02

OUTSIDE BATTERS EYE FOOTING

PCSD-2025507-323113-05

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PCSD-2025507-323001-01

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PCSD-2025507-055213-18

B5 SOFTBALL - BATTER DETAIL PCSD-2025507-321823-05

SOFTBALL BACKSTOP AND NETTING LAYOUT

3/16" = 1'-0"

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— 9" Ø INLAID
 CENTER MARK

(COLOR TO BE DETERMINED)

9"~

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8'-6"				42'-0 1/8"						8'-6"	10'-0"	10'-0"	10'-0"
									1			1	1
	- 10'-0" HIGH BLACK V	INYL COATED CH	AINLINK FENCE BLACK VINYL C	OATED CHAINL	INK FENCE								
	4'-8" 4'-8"	4'-8"	4'-8"	4'-8"	4'-8"	4'-8"	4'-8"	4'-8"					
			1 .										

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IRRIGATION SCHEDULE

RRIGATIC	ON SCHEDULE		
SYMBOL	MANUFACTURER/MODEL/DESCRIPTION		
	HUNTER MP1000 PROS-06-PRS40-CV TURF ROTATOR, 6" POP-UP WITH CHECK VALVE, PRESSURE REGULATED TO 40 PSI, MP ROTATOR NOZZLE ON PRS40 BODY. M=MAROON ADJ ARC 90 TO 210, L=LIGHT BLUE 210 TO 270 ARC, O=OLIVE 360 ARC.		
<u> (K)</u> (G) (R)	HUNTER MP2000 PROS-06-PRS40-CV TURF ROTATOR, 6" POP-UP WITH FACTORY INSTALLED CHECK VALVE, PRESSURE REGULATED TO 40 PSI, MP ROTATOR NOZZLE ON PRS40 BODY. K=BLACK ADJ ARC 90-210, G=GREEN ADJ ARC 210-270, R=RED 360 ARC.		
	HUNTER MP3000 PROS-06-PRS40-CV TURF ROTATOR, 6" POP-UP WITH FACTORY INSTALLED CHECK VALVE, PRESSURE REGULATED TO 40 PSI, MP ROTATOR NOZZLE ON PRS40 BODY. B=BLUE ADJ ARC 90-210, Y=YELLOW ADJ ARC 210-270, A=GRAY 360 ARC.		
SYMBOL	MANUFACTURER/MODEL/DESCRIPTION	<u>PSI</u>	RADIUS
25	RAIN BIRD 5006-PL-PC-SAM-R-SS-MPR 25 TURF ROTOR, 6" POP-UP, STAINLESS STEEL RISER, WITH FLOW SHUT-OFF DEVICE. MATCHED PRECIPITATION ROTOR (MPR NOZZLE), ARC AND RADIUS AS PER SYMBOL. 25 FT=RED, 30 FT=GREEN, 35FT=BEIGE. WITH SEAL-A-MATIC CHECK VALVE, AND IN-STEM PRESSURE REGULATOR.	45	25'
30	RAIN BIRD 5006-PL-PC-SAM-R-SS-MPR 30 TURF ROTOR, 6" POP-UP, STAINLESS STEEL RISER, WITH FLOW SHUT-OFF DEVICE. MATCHED PRECIPITATION ROTOR (MPR NOZZLE), ARC AND RADIUS AS PER SYMBOL. 25 FT=RED, 30 FT=GREEN, 35FT=BEIGE. WITH SEAL-A-MATIC CHECK VALVE, AND IN-STEM PRESSURE REGULATOR.	45	30'
35	RAIN BIRD 5006-PL-PC-SAM-R-SS-MPR 35 TURF ROTOR, 6" POP-UP, STAINLESS STEEL RISER, WITH FLOW SHUT-OFF DEVICE. MATCHED PRECIPITATION ROTOR (MPR NOZZLE), ARC AND RADIUS AS PER SYMBOL. 25 FT=RED, 30 FT=GREEN, 35FT=BEIGE. WITH SEAL-A-MATIC CHECK VALVE, AND IN-STEM PRESSURE REGULATOR.	45	35'
SYMBOL	MANUFACTURER/MODEL/DESCRIPTION		
	RAIN BIRD XCZ-100-PRB-COM WIDE FLOW DRIP CONTROL KIT FOR COMMERCIAL APPLICATIONS. 1" BALL VALVE WITH 1" PESB VALVE AND 1" PRESSURE REGULATING 40PSI QUICK-CHECK BASKET FILTER. 0.3 GPM-20 GPM		
	AREA TO RECEIVE DRIPLINE		
	NETAFIM TLCV-06-12 TECHLINE PRESSURE COMPENSATING LANDSCAPE DRIPLINE WITH CHECK VALVE. 0.6 GPH EMITTERS AT 12" O.C. DRIPLINE LATERALS SPACED AT 12" APART, WITH EMITTERS OFFSET FOR TRIANGULAR PATTERN. 17MM.		
SYMBOL	MANUFACTURER/MODEL/DESCRIPTION		
•	RAIN BIRD PESB-PRS-D 1", 1-1/2", 2" PLASTIC INDUSTRIAL VALVES. LOW FLOW OPERATING CAPABILITY, GLOBE CONFIGURATION. WITH PRESSURE REGULATING MODULE, AND SCRUBBER TECHNOLOGY FOR RELIABLE PERFORMANCE IN DIRTY WATER IRRIGATION APPLICATIONS.		
	RAIN BIRD 44-LRC 1" BRASS QUICK-COUPLING VALVE, WITH CORROSION-RESISTANT STAINLESS STEEL SPRING, LOCKING THERMOPLASTIC RUBBER COVER, AND 2-PIECE BODY.		
X	SHUT OFF VALVE		
$\langle D \rangle$	DRAIN VALVE		
С	RAIN BIRD ESPLXME2-LXMM W/ (3) ESPLXMSM12 48 STATION, TRADITIONALLY-WIRED, COMMERCIAL CONTROLLER.(1) ESPLXME2 12-STATION, INDOOR/OUTDOOR, PLASTIC WALL-MOUNT ENCLOSURE W/ (3) ESPLXMSM12 - 12-STATION EXPANSION MODULES. INSTALL IN LXMM POWDER COATED, METAL WALL-MOUNTED CABINET.		
۲. י. • ۲	AMIAD 4-A-MSIG-STEEL SCREEN AMIAD 4IN. MINI SIGMA, ANGLED SELF-CLEANING FILTER, FLANGE AND GROOVE COUPLING. STAINLESS STEEL WEAVEWIRE SCREEN ELEMENT. ADI-P BLUETOOTH RANGE CONTROLLER. MAXIMUM WORKING PRESSURE 116PSI. POINT OF CONNECTION 4"		
	IRRIGATION LATERAL LINE: PVC SCHEDULE 40		
	IRRIGATION MAINLINE: PVC SCHEDULE 40 SCHEDULE 40 PVC PIPE WITH SCHEDULE 40 FITTINGS		
	PIPE SLEEVE: PVC CLASS 200 SDR 21		
	/alva Callaut		

Valve Number

Valve Flow

Valve Size

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DRIP IRRIGATION NOTES

- 1. LAYOUT DRIP LINE SO THAT AS A MINIMUM ALL SHRUBS LIE BETWEEN TWO ROWS OF DRIPPER LINE AND SO THAT ROWS ARE 18" TO 24" APART TYPICAL. FIELD VERIFY SOIL, SUBSOIL AND SLOPE CONDITIONS AND MODIFY AS REQUIRED.
- 2. COORDINATE LAYOUT OF DRIP LINE WITH THE LAYOUT OF THE PLANT MATERIAL THROUGHOUT.
- 3. INSTALL AUTOMATIC DRAINS AT ALL LOW POINTS IN THE PVC SUPPLY AND EXHAUST HEADERS TO ENSURE COMPLETE DRAINAGE.
- 4. FLUSH OUT THE SYSTEM COMPLETELY PRIOR TO INSTALLING THE FLUSH VALVES TO PREVENT CLOGGING.
- 5. INSTALL PVC SUPPLY AND EXHAUST HEADERS A MIN. OF 6" BELOW FINISH GRADE TYPICAL THROUGHOUT.
- 6. INSTALL DRIPPER LINE 2" BELOW FINISH GRADE DIRECTLY BELOW MULCH LAYER.
- 7. THIS CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLING AN ADEQUATE AMOUNT OF AIR RELIEF VALVES. EACH ZONE, SEPARATE AREA WITHIN ZONES, OR AREAS WITHIN ZONES SEPARATED BY GRADE VARIATIONS REQUIRE AIR RELIEF VALVES. INSTALL IN 2" PVC SLEEVES WITH MARKER CAP.
- 8. ALL PVC SUPPLY AND EXHAUST HEADERS SHALL BE A MINIMUM OF 1" IN SIZE.

IRRIGATION REMODEL NOTES

- 1. THIS CONTRACTOR SHALL BE RESPONSIBLE FOR FIELD VERIFICATION OF THE EXISTING SPRINKLER IRRIGATION SYSTEM IN TERMS OF FLOW CAPACITY, VALVE WIRING, VALVE AND HEAD LAYOUT AND CONTROLLER CAPACITY.
- 2. THIS CONTRACTOR SHALL BE RESPONSIBLE FOR THE COMPLETE REMODEL OF A SECTION OF THE EXISTING IRRIGATION SYSTEM AS SHOWN AND FOR PROVIDING FOR FULL COVERAGE OF ALL SYSTEM HEADS AND FOR THE FULL AND COMPLETE OPERATION OF BOTH THE NEW AND EXISTING SYSTEM IN THE AREAS BEING MODIFIED.
- 3. THIS CONTRACTOR SHALL COORDINATE ALL DEMOLITION WORK AND TIMES WITH THE OWNER.
- 4. MAINTAIN AND PROTECT AS MUCH OF THE EXISTING IRRIGATION SYSTEM AS POSSIBLE AND FEASIBLE AND STILL PROVIDE FOR FULL COVERAGE OF THE ENTIRE AREA. KEEP ALL LAWN AREA HEADS ON A SEPARATE CIRCUIT FROM SHRUB AREA HEADS.
- 5. THE EXISTING IRRIGATION SYSTEMS NOTED TO REMAIN IN USE SHALL BE PATCHED AND REPAIRED AS NECESSARY. MAINTAIN OPERATION OF THE EXISTING SYSTEM AS REQUIRED TO PROTECT EXISTING PLANT MATERIAL. CONTRACTOR SHALL BE RESPONSIBLE FOR DAMAGE OR DEATH OF EXISTING PLANT MATERIAL.
- 6. ALL SALVAGED HEADS SHALL BE RE-USED IN THE NEW SYSTEM, IF COMPATIBLE WITH NEW HEADS, OR RETURNED TO THE OWNER.
- 7. THIS CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING THE EXISTING VALVE WIRING AND RE-ROUTING AS SHOWN AND AS REQUIRED. MINIMIZE THE USE OF WIRE SPLICES. PROVIDE TEMPORARY WIRING AS REQUIRED TO KEEP THE EXISTING SYSTEM IN OPERATION.
- 8. REPAIR DAMAGE TO EXISTING IRRIGATION SYSTEM AND LAWN RESULTING FROM CONSTRUCTION OPERATIONS.

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IRRIGATION NOTES

- 1. ALL MAIN LINE PIPE SHALL BE NEW CLASS 200 RING-TITE PVC PIPE WITH DUCTILE IRON DEEP SOCKET FITTINGS ON ALL TEES, ELBOWS AND 90'S. ALL SECONDARY LATERAL LINE PIPE SHALL BE SCHEDULE 40 PVC PIPE. FITTINGS ON ALL SECONDARY LATERAL LINES SHALL BE SCHEDULE 40 FITTINGS.
- 2. ALL GALVANIZED PIPE ON DETAILS SHALL BE NEW GALVANIZED STEEL PIPE.
- 3. LIVE SERVICE MAINS SHALL BE INSTALLED A MINIMUM OF 24" BELOW FINISH GRADE. BACKFILL TRENCH AROUND LIVE SERVICE MAIN WITH A MINIMUM OF 8" OF SAND. LATERAL LINES SHALL BE PLACED A MINIMUM OF 18" BELOW FINISH GRADE.
- 4. ALL MAIN LINES SHALL SLOPE TO DRAIN. IF FIELD CONDITIONS NECESSITATE ADDITIONAL DRAINS, THESE DRAINS SHALL BE INSTALLED FOR COMPLETE DRAINAGE OF THE ENTIRE MAINLINE. PROVIDE A 24" DIA. X 24" DEEP GRAVEL SUMP UNDER EACH DRAIN, WHICH DRAIN SHALL BE A MIN. OF 24" BELOW GRADE. ALL MANUAL DRAIN VALVES SHALL BE INSTALLED AS DETAILED ON THE DRAWINGS. INSTALL GEO-TEXTILE FABRIC AROUND EACH GRAVEL DRAIN.
- 5. THIS CONTRACTOR SHALL PROVIDE AND INSTALL AIR RELIEF VALVES ON THE MAINLINE AT ALL DEAD END RUNS AND AT ALL HIGH POINTS THROUGHOUT.
- 6. THE CONTRACTOR SHALL PRESSURE TEST THE MAINLINE AT 150 P.S.I. PRIOR TO INSTALLING ANY LATERAL LINES.
- 7. ALL FITTINGS IN THE MAIN PUMP/FILTER ASSEMBLY SHALL BE FLANGED CAST IRON PIPE. ALL PVC FITTINGS SHALL BE ASTM 2466 FITTINGS.
- 8. IRRIGATION SYSTEM IS DESIGNED SO THAT THE SYSTEM CAN BE WINTERIZED USING COMPRESSED AIR. DO NOT INSTALL AUTOMATIC DRAINS ANYWHERE ON THE SYSTEM.
- 9. THIS CONTRACTOR SHALL PROVIDE RECORD DRAWINGS OF THE IRRIGATION SPRINKLER SYSTEM IN CAD FORMAT SHOWING EXACT MEASURED AND DIMENSIONED LOCATIONS OF ALL VALVES, WIRE SPLICES NOT IN A VALVE BOX AND DRAIN VALVES. TIE DIMENSIONS TO PERMANENT FEATURES SUCH AS STRUCTURES.
- 10. THIS DRAWING IS DIAGRAMMATIC ONLY AND IS INTENDED TO CONVEY THE IDEA OF FULL COVERAGE OF THE IRRIGATION SPRINKLER SYSTEM. PRINTS SHALL NOT BE SCALED. THE IRRIGATION SYSTEM CONTRACTOR SHALL BE RESPONSIBLE FOR THE INSTALLATION LAYOUT OF THE SYSTEM IN ACCORDANCE WITH THE DRAWINGS TO PROPORTIONALLY COVER A GIVEN AREA AS SHOWN. THE LAYOUT MAY BE MODIFIED IF NECESSARY TO OBTAIN COVERAGE TO SUIT THE MANUFACTURERS STANDARD HEADS INDICATED. DO NOT DECREASE THE NUMBER OF HEADS INDICATED UNLESS THIS IS ACCEPTABLE TO THE LANDSCAPE ARCHITECT. THE SYSTEM SHALL BE TESTED FOR COMPLETE COVERAGE AND ALL NECESSARY PROPER ADJUSTMENTS MADE TO GET FULL AND PROPER COVERAGE PRIOR TO ACCEPTANCE BY THE OWNER.
- 11. THE SYSTEM IS DESIGNED FOR 45 PSI OPERATING PRESSURE ON ALL LARGE HEADS AND 30 PSI ON ALL SMALL AREA HEADS UNLESS OTHERWISE NOTED.
- 12. ALL VALVES TO BE WIRED TO CONTROLLERS USING #14 U.F. WIRE AND PEN-TITE WATER RESISTANT WIRE CONNECTORS. ALL VALVE WIRES UNDER PAVING SHALL BE INSTALLED IN A MINIMUM 2" SCHEDULE 40 PVC CONDUIT BURIED 24" DEEP. PROVIDE AND INSTALL A DIFFERENT COLOR VALVE WIRE FOR EACH CONTROLLER. RUN ONE EXTRA WIRE FROM THE ADJACENT CONTROLLER TO EACH GROUP OF VALVES FOR FUTURE USE AND STUB INTO THE VALVE BOX.
- 13. ALL VALVE BOXES SHALL BE JUMBO SIZED PLASTIC BOXES, AMETEC OR EQUAL UNLESS OTHERWISE DETAILED.
- 14. ALL VALVES WILL BE LOCATED IN GROUPS 3' AWAY FROM WALKS AND CURBS-COORDINATE WITH MAINLINE LAYOUT. A DRAIN VALVE WITH SUMP SHALL BE PROVIDED AND INSTALLED AT EACH GROUP OF VALVES. A QUICK COUPLER SHALL BE PROVIDED AT EVERY VALVE MANIFOLD LOCATION. VALVES SHALL BE LOCATED 3'-0" AWAY FROM THE CURBS, WALKS OR MOWSTRIP.
- 15. ALL ISOLATION VALVES AT VALVE MANIFOLDS SHALL BE APOLLO 70-100 SERIES BALL VALVES.
- 16. A MAXIMUM OF FOUR VALVES SHALL BE INSTALLED ON EACH VALVE MANIFOLD OR MAIN LINE TEE. ALL MAINLINE MANIFOLD TEES SHALL HAVE A 4" MINIMUM OUTLET.
- 17. ALL HEADS SHALL BE SET PERPENDICULAR TO THE EXISTING GRADE SO AS TO PROVIDE PROPER COVERAGE.
- 18. ALL HEADS ADJACENT TO BLDG. SHALL BE INSTALLED A MIN. OF 12" AWAY FROM BLDG.
- 19. ALL GEAR DRIVE ROTOR HEADS SHALL BE ENCLOSED IN A ONE CUBIC FOOT GRAVEL SUMP OF 3/4" OR SMALLER AGGREGATE.
- 20. THIS CONTRACTOR SHALL BOTH PROVIDE AND INSTALL THE PREFABRICATED CONTROL PANEL WHICH CONTAINS THE SYSTEM CONTROLLERS, RELAYS, AND OTHER NECESSARY EQUIPMENT AT THE LOCATION SHOWN. THE PIPE CONDUIT THRU THE WALL AND UP TO THE CONTROL PANEL SHALL BE BY OTHERS. PERMANENT POWER TO THE CONTROL PANEL IS BY THE ELECTRICAL CONTRACTOR. THE EXACT LOCATION OF THE CONTROL PANEL SHALL BE DETERMINED BY THE OWNER.
- 21. PROVIDE AND INSTALL ALL THE MANUFACTURER'S RECOMMENDED SURGE AND LIGHTNING PROTECTION EQUIPMENT ON ALL NECESSARY SYSTEM COMPONENTS.
- 22. THIS CONTRACTOR SHALL BE RESPONSIBLE FOR REPAIRING ANY SITE ITEMS DAMAGED DURING THE COURSE OF CONSTRUCTION.
- 23. IRRIGATION PRODUCT SUBMITTALS SHALL INCLUDE PIPE SLEEVING AND CONDUIT.
- 24. INSTALL THE PUMP/FILTER ASSEMBLY AS DETAILED AND AS PER INDUSTRY STANDARDS. THIS CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING AND INSTALLING THE ENTIRE PRE-FABRICATED PUMP/FILTER PACKAGE (INCLUDING PUMP PANEL) AND FOR ITS COMPLETE OPERATION. THIS CONTRACTOR SHALL INSTALL A 6" THICK CONCRETE PAD UNDER THE PUMP SKID (EXACT SIZE TO BE FIELD VERIFIED). MOUNT THE SKID TO THE PAD AS PER PUMP MFGR RECOMMENDATIONS.

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REFE	REFERENCE NOTES				
SYMBOL	CODE	DESCRIPTION	DETAIL		
* * * * * * * * * * * * * * * *	LI-01	PATCH AND REPAIR EXISITNG			
* * * * * * * * * * * * * * * *		IRRIGATION AND LANDSCAPE			
* * * * * * * * * * * * * * * *		DAMAGED BY CONSTRUCTION			
* * * * * * * * * * * * * * * *		OPERATIONS. ENSURE HEAD			
* * * * * * * * * * * * * * *		TO HEAD FULL COVERAGE.			

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REFERENCE NOTES			
SYMBOL	CODE	DESCRIPTION	DETAIL
* * * * * * * * * * * * * * * *	LI-01	PATCH AND REPAIR EXISITNG	
* * * * * * * * * * * * * * *		IRRIGATION AND LANDSCAPE	
* * * * * * * * * * * * * * *		DAMAGED BY CONSTRUCTION	
* * * * * * * * * * * * * * *		OPERATIONS. ENSURE HEAD	
		TO HEAD FULL COVERAGE.	

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Original drawing is 30 x 42. Do not scale contents of this drawing. RETURN TO SHEET INDEX

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- REMOTE CONTROL VALVE WITH DISC FILTER

PCSD-2025507-328413-11

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- FINISH GRADE

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PLANT SCHEDULE

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	11 20	ΠΕΡΟΓΕ				
<u>SYMBOL</u>	CODE	BOTANICAL NAME	COMMON NAME	<u>SIZE</u>		REMARKS
<u>CONIFERO</u> سیمهنور	US EVERG	REENS				
Jan - the	PIC CO2	PICEA PUNGENS	COLORADO SPRUCE	10` HT MIN		
- A	PIN FLE	PINUS FLEXILIS 'VANDERWOLF'S PYRAMID'	VANDERWOLF'S PYRAMID LIMBER PINE	15` HT MIN		
DECIDUOU	<u>S</u>					
\odot	ACE GRA	ACER GRANDIDENTATUM	BIGTOOTH MAPLE	2" CAL		
\bigcirc	CER EAS	CERCIS CANADENSIS	EASTERN REDBUD MULTI-TRUNK	2" CAL		
	PT	POPULUS TREMULOIDES	QUAKING ASPEN	2" CAL		
$\overline{(\cdot)}$	PRU CHO	PRUNUS VIRGINIANA	CHOKECHERRY	2" CAL		
SYMBOL	<u>CODE</u>	BOTANICAL NAME	COMMON NAME	<u>CONT</u>		REMARKS
SHRUBS	45018/4					
•	ARC UVA	ARCTOSTAPHYLOS UVA-URSI	KINNIKINNICK	5 GAL		
	COR HAL	CORNUS ALBA 'BAILHALO' TM	IVORY HALO DOGWOOD	5 GAL		
	JUN JUN	JUNIPERUS COMMUNIS	COMMON JUNIPER	5 GAL		
	MAH REP	MAHONIA REPENS	CREEPING MAHONIA	5 GAL		
	PRU PAW	PRUNUS BESSEYI 'P011S' TM	PAWNEE BUTTES SAND CHERRY	5 GAL		
	RHU GRO	RHUS AROMATICA 'GRO-LOW'	GRO-LOW FRAGRANT SUMAC	5 GAL		
•	RIB ALP	RIBES ALPINUM	ALPINE CURRANT	5 GAL		
	RIB GOL	RIBES AUREUM	GOLDEN CURRANT	5 GAL		
	SAL PUR	SALIX PURPUREA 'NANA'	DWARF PURPLE OSIER WILLOW	5 GAL		
	SYM ALB	SYMPHORICARPOS ALBUS	COMMON WHITE SNOWBERRY	5 GAL		
ORNAMEN	TAL GRASS	8				
	BOU CUR	BOUTELOUA CURTIPENDULA	SIDE OATS GRAMA	1 GAL		
0	SCH LIT	SCHIZACHYRIUM SCOPARIUM	LITTLE BLUESTEM	1 GAL		
	SPO AIR	SPOROBOLUS AIROIDES	ALKALI SACATON	1 GAL		
PERENNIA	LS					
	ERI SUL	ERIOGONUM UMBELLATUM	SULFURFLOWER BUCKWHEAT	1 GAL.		
	PEN PEN	PENSTEMON EATONII	FIRECRACKER PENSTEMON	1 GAL.		
	PEN STR	PENSTEMON STRICTUS	ROCKY MOUNTAIN PENSTEMON	1 GAL.		
SYMBOL		BOTANICAL NAME	COMMON NAME	CONT	<u>SPACING</u>	REMARKS
GROUND C	OVERS					SEE SPECIFICATIONS
		NATIVE MEADOW SEED	NATIVE MEADOW SEED MIX	SEED		FOR SEED MIX
		POA PRATENSIS	KENTUCKY BLUEGRASS	FLAT		

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PLANTING NOTES

- 1. ALL PLANTS SHALL CONFORM TO THE MINIMUM STANDARDS OF HEIGHT, SIZE, CALIPER AND ETC. OF THE AMERICAN ASSOCIATIONS OF NURSERYMEN "AMERICAN STANDARDS FOR NURSERY STOCK".
- 2. THIS CONTRACTOR SHALL SPREAD TOPSOIL TO A DEPTH OF 6" IN ALL LAWN PLANTING AREAS AND 12" IN ALL SHRUB AND PERENNIAL BEDS.
- 3. THIS CONTRACTOR SHALL BE RESPONSIBLE FOR DISPOSING FROM THE SITE ALL SOIL EXCAVATED FROM TREE PITS.
- ALL MOWSTRIPS ARE TO BE INSTALLED PRIOR TO THE INSTALLATION OF THE IRRIGATION SYSTEM SYSTEM AND THE LANDSCAPE PLANTING.
- 5. INSTALL SHREDDED MULCH IN ALL SHRUB PLANTING BEDS AFTER PLANT MATERIAL INSTALLATION.
- 6. THIS CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING AND INSTALLING TREE PIT DRAINAGE AS SHOWN.
- 7. THIS CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING AND INSTALLING THE REQUIRED AMOUNT OF TOPSOIL TO COMPLETE THE PROJECT. NEW TOPSOIL SHALL MATCH QUALITY AND TEXTURE OF THE EXISTING TOPSOIL ON SITE.

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MHTN ARCHITECTS MHTN Architects, Inc. 280 South 400 West Suite 250 Salt Lake City, Utah 84101 Telephone (801) 595-6700 www.mhtn.com

CONSULTANTS, INC. Address Suite 100 City, State 12345 Telephone (123) 456-7890 Telefax (123) 456-7890 www.consultant.com

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MHTN ARCHITECTS MHTN Architects, Inc. 280 South 400 West Suite 250 Salt Lake City, Utah 84101 Telephone (801) 595-6700 www.mhtn.com

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REFE	REFERENCE NOTES			
SYMBOL	CODE	DESCRIPTION	DETAIL	
< x x x x x x x x x x x x x x x x x x x	LI-01	PATCH AND REPAIR EXISITNG		
* * * * * * * * * * * * * * *		IRRIGATION AND LANDSCAPE		
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MHTN ARCHITECTS MHTN Architects, Inc. 280 South 400 West Suite 250 Salt Lake City, Utah 84101 Telephone (801) 595-6700 www.mhtn.com

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MH⁻ ARCHITECTS MHTN Architects, Inc. 280 South 400 West Suite 250 Salt Lake City, Utah 84101 Telephone (801) 595-6700 www.mhtn.com

	1. Design Criteria	
	1.1. Governing Building Code A. Risk Category	2021 International Building Code (IBC) II
	 1.2. Floor Live Loading A. Office B. Exit Facilities & Corridors C. Mechanical Rooms D. Assembly Areas 	50 psf Live Load + 15 psf Partition Load 100 psf Live Load 125 psf Live Load or actual weights, if larger 100 psf Live Load
	1.3. Roof Loading A. Roof Live Load	20 psf
A	 B. Roof Snow Load 1. Ground Snow Load, Pg 2. Snow Exposure Factor, Ce 	62 psf + Drift per IBC 89 psf 1.0
	 Importance Factor, Is Thermal Factor, Ct Slope Factor, Cs Paof Pain Load Intensity: 	1.0 1.0 Roof, 1.2 Canopies 1.0
	 1. 15-min duration/100-year return period, i₁₅ 2. 60-min duration/100-year return period, i₆₀ 	4.12 inches per hour 1.72 inches per hour
	1.4. Superimposed Dead Load A. Floor B. Mechanical Floor	15 psf 25 psf
	C. Roof 1.5. Earthquake	15 psf
	 A. Seismic Design Category B. Spectral Response Accelerations: Ss = 0.58 g S_{DS} =0.49 g 	D
	$S_1 = 0.21 \text{ g}$ $S_{D1} = 0.21 \text{ g}$ C. Site Class D. Basic Seismic-Force-Resisting System $R = 2.50$ $Q_2 = 2.50$	C Intermediate Reinforced Masonry Wall
	E. Importance Factor, I _e F. Redundancy Factor, ρ G. Analysis Procedure	1.0 1.0 Equivalent Lateral Force (Static)
	1.6. Wind A. Basic Design Wind Speed, V	103 mph
	 B. Velocity pressure exponent coefficient, K_d C. Ground elevation factor, K_e D. Exposure category 	0.85 1 B
	 E. Internal Pressure Coefficient, GC_{pi} F. Topographic Factor, K_{zt} G. Components and Cladding Design Pressure: 	0.18 1.0
	Location	Tributary Area (ft²) < 10
В	Walls Within 5.75 ft of building corner All other areas Zone 3: Within 3.75 ft "L" shape at building Roof Zone 2: Within 16.25 ft of the building	18.3 16.5 16 16 18.3 16.5 16 16 ilding corner 52.7 36.2 31.2 24.7 edge 38.7 30.4 27.9 24.7
	Zone 1: All other areas	29.3 22.9 21.0 18.4
	A. The soils report and log of borings was obtained to is not a part of the Contract Documents. This report not a warranty of the subsurface conditions. The Contract Documents is not a warranty of the subsurface conditions. The Contract Documents is not a warranty of the subsurface conditions. The Contract Documents is not a warranty of the subsurface conditions. The Contract Documents is not a warranty of the subsurface conditions. The Contract Documents is not a warranty of the subsurface conditions. The Contract Documents is not a warranty of the subsurface conditions. The Contract Documents is not a warranty of the subsurface conditions. The Contract Documents is not a warranty of the subsurface conditions.	by the Owner for the Engineer's use in the design of the foundation and ort and log of borings is available for the Contractor's information but is Contractor may use the report at their own risk.
	 B. Soils Report by AGEC Applied GeoTech, dated F C. Soil Bearing Pressure 	February 27, 2025. 3500psf on a minimum of 3.5 feet of compacted fill down to suitable, undisturbed, natural soils
	E. Frost Depth F. Lateral Soil Pressure Fluid Equivalent Density: Active	40 inches below grade
	(increase for seismic) (increase for seis 40 pcf (21 pcf) 55 pcf (6 pcf) G. Coefficient of Friction	mic) (decrease for seismic)) 300 pcf (21 pcf) 0.45
	1.8. Classification for Fire Rated ConstructionA. For the purpose of determining fire-resistive asse	mblies, the following framing systems shall be considered unrestrained:
	 Open web steel framing members supporting to 2. Single span and simply supported end spans of a. Steel beams supporting concrete slabs or b. Open web steel joists supporting concrete c. Concrete slabs or metal decking. Interior spans of multiple bays supported by beams of multiple bays of multiple bays supported by beams of multiple bays of multiple bays supported by beams of multiple bays of m	metal decking. of multiple bays that are supported by bearing walls: metal decking. slabs earing walls:
	 a. Steel beams supporting metal decking B. All other steel floor and roof framing members sha C. Diagonal members in vertical braced frames shal 	all be considered restrained. I be considered as secondary members for fireproofing protection.
	2. Earthwork	
	2.1. Soil conditions shall be verified by the Geotechnical Er If soil conditions exist that are different than the soils re foundation design at additional expense to the owner.	ngineer prior to placement of structural fill, floor slabs, and foundations. eport, the Structural Engineer shall be notified to re-evaluate the
С	2.2. Clearing: Prior to placing fill or base course, non-engin be removed at least 3 feet beyond the perimeter of the	eered fill, topsoil, organics, debris, and other deleterious material shall proposed building and flatwork. As a minimum the entire building area
	2.3. Soils identified as unsuitable, including peat and organ feet, as indicated in the Geotechnical Report. All unsuitable	ically contaminated soils, are present at depths of approximately 7 to 9 table shall be removed and replaced with engineered fill. No footings or
	slabs shall be placed on these materials. 2.4. Proof Rolling: The natural undisturbed soil below all for	otings shall be proof rolled prior to placing concrete. Remove all soft
	spots and completely replace them with compacted str2.5. Remove all unsuitable material from below footings and	d replace it with compacted structural fill down to suitable natural soils
	circumstances shall the footings be placed on unsuitable ponded water. The width of structural compacted fill sh thickness. 2.6. Compacted Structural Fill: Structural fill shall be provid	ed at all locations and extents described by the TYPICAL
	COMPACTED STRUCTURAL FILL DETAIL. All fill ma than 4 inches and with no more than 10% passing a No below of the maximum laboratory density as determine	terial shall be a well-graded granular material with a maximum size less umber 200 sieve. All fill shall be compacted to the percent in the table ed by ASTM D1557. The fill shall be compacted flat without ruts or dips
	below all floor slabs. All fill shall be tested (See Specific Fill to Support Buildings and Foundations	cations and the Quality Assurance section of the GSN). Compaction ≥ 95%
	Concrete Flatwork & Floor Slabs Site Grading Retaining Wall Backfill	$ \ge 95\% $ $ \ge 95\% $ $ \ge 95\% $ $ \ge 95\% $
	2.7. Underslab Drainage Base Course (Granular Fill): The l remain stable and support construction traffic. Compac	base material should be a compactible, easy to trim, granular fill that will the base course such that no ruts deeper than $\frac{1}{2}$ inch are left by
	construction traffic. The base course shall be placed in minimum slab thickness specified. 1. Depth of Drainage Base Course	lifts of up to 6 inches and be graded to a tolerance that results in the 4 inches unless noted otherwise
	 The base course shall be one of the following a. Densley Graded: Crusher run or road base the No. 4 sieve, and less than 12% passing grapular fill with loss than 2% alow or fright. 	materials at the contractor's option: e material with 100% passing the 1 ½ inch sieve, 15% to 55% passing g the No. 200 sieve. Material passing the No. 200 sieve shall be clean,
D	b. Open Graded: Crushed rock with 100% pa material is not as stable as densely graded allow compaction and prevent ruts, the sur	I material and rutting due to construction traffic is more common. To face may need to be filled in or choked off with sand or fine gravels and
	compacted to provide a smooth, planar su 2.8. It shall be the responsibility of the Contractor to brace a	rface and shore excavations as required.
	3. Concrete	
	3.1. Materials shall comply with the Standards specified in A Requirements for Structural Concrete."	American Concrete Institute (ACI) 318-19, "Building Code
	Location	f'c at Max Air Max Exposure 28 days W/C Content Aggregate Classes*
	Footings Interior Slabs on Grade	(ps) (value (vo) Size F S C W 3000 0.50 - 1" F0 S0 C0 W1 3000 0.45 - 1" F0 S0 C0 W0 4500 0.45 - 1" F0 S0 C1 W1
	Walls Concrete over Steel Deck	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
nplex.rvt	* Exposure Classes are per ACI 318, Section 19.3 thawing, sulfate, and corrosion protection of reinfo wall, or slab that is placed over corrosive native so	8.1.1, where F, S, C and W are exposure categories for freezing and recement, and contact with water, respectively. Any footing, foundation bills shall have a S1-S3 classification, and concrete mix design shall be
letic Con	adjusted for this condition B. Cementitious Materials: 1. Blended Hydraulic Cements (ASTM C595):	
CSD Ath	 a. Type IL, or approved equivalent, for expos b. Type IL(MS), or approved equivalent, for e c. Type IL(HS), or approved equivalent, for e 	ure class S0. xposure class S1. xposure class S2 and S3.
1-0505 P	 Portiand Cement (ASTM C150) a. Type I or II for exposure class S0. b. Type II or V for exposure class S1. c. Type V for exposure class S2 and S2. 	
524 202 ⁴ 川	 a. Type V for exposure class 52 and 55. 3. Portland Cement (ASTM C150) a. Type I or II for exposure class S0. 4. Fly Ash (ASTM C618. Class C or F)⁻ maximum 	n fly ash content as a percentage of total weight of cementitious
omplex/S	C. Aggregates: 1. For concrete with exposure class W1. evidence	e shall be submitted that aggregates comply with the followina:
Sport Cc	a. Aggregates are not alkali-silica reactive or b. Aggregates are not alkali-carbonate reactiv D. Concrete Density (Maximum Air-Dry Weight):	measures to mitigate alkali-silica reactivity have been established.
5507 PC	 Normal weight concrete shall be approximately Lightweight concrete shall not exceed 110 pour and either lightweight and/or normal weight find 	y 145 to 155 pounds per cubic foot. Aggregate shall be ASTM C33. Inds per cubic foot and shall be made of lightweight coarse aggregates es meeting ASTM C330.
//2025 7 PM	 E. Steel Reinforcement: 1. ASTM A615 Grade 60, fy = 60,000 psi min. un Wire Reinforcement: 	less noted otherwise.

1. Welded Wire Fabric (WWF): ASTM A1064.

G. Fiber Reinforcement: 1. Synthetic Micro-Fiber: Fibrillated polypropylene micro-fibers engineered and designed for use in concrete, complying with ASTM C 1116, 1/2 to 1.1/2 inches long. Add to concrete at a dosage rate of 1 ½ lb/cu yd where indicated.

2.	Macrosynthetic Fibers: Monofilament, non-fibrillating fibers made of a polypropylene/polyethylene blend. Macro fibers
	shall comply with ASTM C 1116, Type III, and meet the criteria of ASTM D 7508.
	a. Where noted in the Steel Deck Schedule, macrosynthetic fibers shall be added to concrete over steel deck at a
	dosage rate determined by the fiber manufacturer but not less than 4 lb/cu yd.
	b. Do not burn off exposed fibers.
Ac	Imixtures:
1.	Air-entraining admixtures, comply with ASTM C 260 (when used).
	a. Tolerance on air content as delivered shall be +/- 1.5%.

b. When air content of a trowel finished floor slab exceeds 3%, there is an increased risk for delaminations and blistering to occur. When this situation is present, the Contractor shall pay special attention to the finishing procedures to help minimize such risks. Refer to ACI 302.1R-15 "Guide for Concrete Floor and Slab Construction" for proper finishing guidelines.

2. The use of super plasticizers and water reducers is allowed, but not required. . Calcium chloride or admixtures containing calcium chloride shall not be added to the concrete mix. Chloride Ion: Maximum water-soluble chloride ion concentrations in hardened concrete at age between 28 and 42 days contributed from the ingredients including water, aggregates, cementitious materials, and admixtures shall not exceed a maximum, by weight of cementious materials, 1.00% for concrete with exposure class C0, 0.30% for concrete with

exposure class C1. J. Slump Limit: 4 inches, maximum for all concrete prior to the addition of plasticizers and water reducing admixtures. The concrete supplier shall indicate the final slump of each concrete mix in the submitted mix design. K. Shrinkage Limit: Interior slabs on grade shall have a drying shrinkage limit of 0.040% tested in accordance with ASTM C157. Drying shrinkage test results shall be submitted with mix designs.

3.2. Formwork shall comply with ACI Standards Publication 347 and the project specifications. The Contractor shall be responsible for the design, detailing, care, placement and removal of the formwork and shores. A. Pre-camber forms and screeds with a camber of ½ inch per every 10 feet of span to compensate for dead load

deflection, unless noted otherwise. 3.3. Concrete cover requirements for deformed bar reinforcing steel shall comply with ACI 318, "Building Code Requirements for Structural Concrete". A. Cast-in-place Concrete: Specified Cover . Cast against and permanently exposed to earth 2. Formed concrete exposed to earth or weather

#6 thru #18 bars #5 and smaller bars

1.1/2" 3. Concrete not exposed to weather or in contact with ground 3/4' Slabs, Walls; #11 bars and smaller

L. Only one grade or type of concrete shall be poured on the site at any given time.

- 3.4. Minimum Spacing of Reinforcement
- A. For parallel reinforcing bars in a horizontal layer, clear spacing between bars shall be at least the greatest of 1 inch, nominal diameter of the bars considered, and $\frac{4}{3}$ times the nominal maximum size of coarse aggregate in the concrete. B. For longitudinal bars in columns, pedestals, struts and boundary elements in walls, clear spacing shall be at least the greatest of 1 $\frac{1}{2}$ inches, 1.5 times the diameter of the bars considered and $\frac{4}{3}$ times the nominal maximum size of coarse aggregate in the concrete.
- C. If scheduled reinforcing cannot meet these provisions, notify the Engineer of Record.
- 3.5. Construction Joints and Control Joints A. Provide a surface intentionally roughened to ¼ inch amplitude in all wall footings. A continuous keyway shall not be used for concrete shear wall to footing connections, unless specifically indicated. Refer to project plans, schedules and details for the shear wall to footing connection requirements.
 - B. All horizontal and vertical construction joints shall have a surface intentionally roughened to ¼ inch amplitude. A continuous 2x4 keyway may be used on elements other than shear walls. C. Provide reinforcement dowels to match the member reinforcement across the joint, unless noted otherwise. For dowels
 - across construction joints and wall to footing connections of concrete shear walls, refer to specific project plans, schedules, and details. D. Slabs on grade shall have construction or control joints spaced not to exceed 30 times the slab thickness in any
 - E. Control joints shall be installed in slabs on grade so the length to width ratio of the slab is no more than 1.25:1. See
 - typical details for joint configuration F. Control joints in visually exposed walls, unless noted otherwise (Joints shall line up with masonry and architectural joints, see drawings): 1. Vertical control joints at 10'-0" on center
 - 2. Reinforcing shall be continuous through control and construction joints, unless noted otherwise. 3. Control joints in concrete foundation walls shall line up with masonry control joints.
 - G. Control joints shall be installed in concrete slabs over steel deck by saw-cutting along girders and purlins at interior grid lines. See typical details for joint size and reinforcement. Reinforcement required shall be in addition to any slab reinforcement.
- H. All sawcut control joints shall be completed within 12 hours of concrete placement. 3.6. Detailing: All reinforcing, including welded wire fabric, shall be detailed, bolstered & supported to comply with ACI 315, "Details
- and Detailing of Concrete Reinforcement" and the Concrete Reinforcing Steel Institute (CRSI) recommendations. Reinforcing bars shall not be welded unless specifically shown on drawings. A. All reinforcing shall be developed in compliance with the CONCRETE REINFORCING BAR DEVELOPMENT AND LAP
- SPLICE SCHEDULE. As indicated in the drawings or upon approval of the Engineer of Record, standard tension hooks or headed bars described by the TENSION HOOK DEVELOPMENT SCHEDULE All embedded elements and dowels shall be securely tied to formwork or to adjacent reinforcing prior to the placement of
- concrete. C. Use chairs or other support devices recommended by CRSI to support and tie reinforcement bars and welded wire fabric prior to placing concrete. Welded wire fabric shall be continuously supported at 36 inches on center maximum.
- D. See typical details for reinforcing at wall intersections and ends, reinforcing around wall openings and suspended slab openings, vertical wall dowels, concrete column ties and splices in vertical column reinforcing. E. See typical details for column cross-ties. The 90-degree hooks of two successive crossties engaging the same longitudinal bars shall be alternated end for end.
- F. Where required, reinforcement is to be terminated in a standard hook or headed bar anchor. Refer to the TENSION HOOK DEVELOPMENT SCHEDULE, the TENSION HEADED BAR DEVELOPMENT SCHEDULE and the REINFORCEMENT END HOOK SCHEDULE as appropriate. Unless otherwise noted, a standard hook or headed bar are equivalent and may be substituted at the Contractor's option.
- G. Contractor shall coordinate placement of all openings, curbs, dowels, sleeves, conduits, bolts, inserts and other embedded items prior to concrete placement
- H. All reinforcement shall be bent cold and shall be bent only once at the same location. All reinforcement shall be shop bent, unless otherwise permitted by the Engineer of Record.

3.7.	Minimum	Reinforcing: Wall reinforcin	ng shall be as follows, unless noted	otherwise:
		Wall Thickness	Horizontal Reinforcing	Vertical F

Wall Thickness	Horizontal Reinforcing	Vertical Reinforcing
6"	#4 @ 13" o.c.	#4 @ 18" o.c.
8"	#5 @ 15" o.c.	#4 @ 16" o.c.
10"	#5 @ 12" o.c.	#4 @ 13" o.c.
12"	#4 @ 13" o.c. Each Face	#4 @ 18" o.c. Each Face
Others	0.25% of Wall Area	0.15% of Wall Area
pacing shall exceed neither 3 times	the wall thickness nor 18 inches. In	addition to the above reinforcing.

- #5 x continuous horizontal bars shall be placed at the bottom of the wall (near the footing) and at each floor level, at the roof level and at the top of wall

3.8. No aluminum conduit or product containing aluminum or any other material injurious to concrete shall be embedded in concrete.

3.9. Unless otherwise noted, all slabs on grade shall be 4 inches thick.

4. Masonry

4.1. Materials shall comply with the Standards specified in TMS 402-16 and TMS 602-16, "Building Code Requirements and Specification for Masonry Structures."

- A. Materials, unless noted otherwise: 1. Concrete Masonry Units: ASTM C 90, Medium Weight.
- 2. Material Strength: The Prism Test Method or the Unit Strength Method according to TMS 602-16 Section 1.4B may be used to determine the compressive strength of masonry assemblies. The contractor shall select the desired method and meet the required material strengths as follows:
- a. Prism Test Method, TMS 602-16 Section 1.4 B.3:
- Concrete Masonry Unit Assembly, f'_m = 2000 psi. b. Unit Strength Method, TMS 602-16 Section 1.4 B.2:
- 1) Concrete Masonry Units, minimum unit strength of 2000 psi average or better. ($f_m = 2000 \text{ psi}$) 3. Mortar: Use Type "S" according to ASTM C270, proportion specification. Admixtures shall not be added to the mortar
- 4. Grout: For masonry assemblies with $f'_m = 2,000$ psi or less conform to ASTM C476, proportion specification. Grout that does not meet the requirements of ASTM C476 proportion specification or that is used in masonry assemblies with $f'_m > 2,000$ psi shall meet the following requirements: Meet the material requirements of ASTM C476, obtain a minimum compressive strength of f_m or 2,000 psi, whichever is larger, at 28 days tested according to ASTM C1019, and a slump of 8 inches to 11 inches as determined by ASTM C143.
- a. Self-Consolidating Grout: Conform to the material requirements of ASTM C476, obtain a minimum compressive strength of f'm or 2,000 psi, whichever is larger, at 28 days tested according to ASTM C1019, obtain a slump flow of 24 inches to 30 inches as determined by ASTM C1611, and shall have a Visual Stability Index less than or equal to 1 as determined in accordance with ASTM C1611 Appendix X.1. Field addition of admixtures is not
- 5. Reinforcing: Grade 60 reinforcing steel shall comply with ASTM A615. Wire joint reinforcing shall comply with ASTM
- 6. Deformed Bar Anchors (DBA): All DBAs shall comply with ASTM A496. 7. Anchor Bolts (AB): ASTM A307 with ASTM A563 heavy hex nuts and hardened washers, Grade A, unless noted
- 8. Headed Stud Anchors (HSA): Manufacture all HSAs in conformance with ASTM A108 with dimensions complying with AISC specifications.

4.2. Construction Requirements

- Mortar Joints: Joints shall be "concave", "V-joint" or "weathered raked" for structural members unless noted otherwise on architectural drawings. Masonry walls, beams and columns shall be constructed with running bond, unless noted otherwise.
- C. Grouting Requirements: Comply with IBC Section 2104 and TMS 602 Section 3.5. Grout shall be mechanically consolidated and mechanically reconsolidated according to TMS 602 Section 3.5 E. 1. Grout Pour Heights that exceed 4 feet shall meet the following requirements:
- a. Provide cleanouts in the bottom course of masonry for each grout pour in accordance with TMS 602 Section 3.2 b. For grout other than Self Consolidating Grout a demonstration panel representative of the proposed wall
- construction and construction procedures shall be provided and approved by the Architect. The demonstration panel may be a part of the completed construction as approved by the Architect. c. For Self-Consolidating Grout placed in masonry that has cured for at least 4 hours, place in lifts not exceeding the Maximum Grout Pour Height in listed in TMS 602 Section 3.5C.
- 2. When grouting, form grout keys between grout pours. Form grout keys between grout lifts when the first lift is permitted to set prior to placement of the subsequent lift.
- a. Form a grout key by terminating the grout a minimum of 1 ½ inches below a mortar joint. b. Do not form grout keys within beams.
- c. At beams or lintels laid with closed bottom units, terminate the grout pour at the bottom of the beam or lintel without forming a grout key. 3. Masonry shall be solid grouted for the following conditions:
- a. Masonry not laid in running bond.
- b. Masonry columns. c. Masonry beams/lintels
- d. Masonry below grade or retaining soils. e. Exterior free-standing walls.
- D. Reinforcing Bars shall not be welded unless specifically shown on drawings. In such cases, use only AWS standards. Do not substitute reinforcing bars for DBAs or HSAs.
- E. Control Joints: Spacing shall not exceed 40 feet or 2.5 times the wall height, whichever is less. Joints shall not be located over masonry openings, and shall be a minimum of the schedule masonry column width away from masonry openings.
- See architectural drawings for locations. F. Grout all beam and joist pockets solid after installation of beams and joists.
- G. Masonry Veneer Attachment and Reinforcing:
- 1. Joint reinforcement: Veneer not laid in running bond shall have continuous, galvanized 9-gauge wire joint reinforcement spaced vertically at 18 inches on center maximum. Mechanically attach veneer anchors to the continuous wire joint reinforcing with Hohmann & Barnard Seismiclip Interlock System (S.I.S.) clips. . Veneer shall be attached with anchors as follows: Veneer Anchors Insulation Backup Fasteners to Backup Present (Hohmann & Barnard) Reinforced masonry walls with 270-2X Ladder
- Cast in place Yes or No joints aligned in veneer Eve Wire
 - 3. Anchors shall be spaced at a maximum of 16 inches on center horizontally and 18 inches on center vertically.

- 4. Anchors, channels, and ties shall be corrosion resistant. 5. Other methods of attachment may be used after written acceptance by the Architect and Engineer of Record. size. All steel angle lintels shall be galvanized unless noted otherwise.
- 4.3. Detailing Requirements A. Standards: Reinforcing detailing shall comply with American Concrete Institute (ACI) Standard 315, "Details and Detailing of Concrete Reinforcement." B. Reinforcement Protection (cover): Joint reinforcement shall have not less than 5/8 inch mortar coverage from the exposed face.
- 2. Other reinforcement shall have a minimum coverage of one bar diameter over all the bars, but not less than ³/₄ inch. When masonry is exposed to soil, minimum coverage shall be $1\frac{1}{2}$ inch. C. Vertical steel reinforcement shall be placed and secured against displacement prior to grouting by wire positioners or
- inches. Vertical reinforcing shall be located at the center of the wall, unless noted otherwise reinforcement shall lap a minimum of 6 inches. E. Corner Bars: Horizontal reinforcement shall be continuous at all corners and at intersecting walls. Provide corner bars
- with the required lap splice length.
- beyond the opening as possible and terminate them with a 90 degree standard ACI hook. H. Horizontal wall reinforcing shall be continuous through joining concrete walls, masonry walls, columns, and pilasters. Provide a key between the wall and the column or pilaster. The horizontal wall reinforcing shall be placed inside the
- column vertical reinforcing. inch of grout surrounding the shank at its penetration. Grout shall be flush with the face or top of the masonry.
- J. All masonry column ties shall terminate with 135-degree hooks plus an extension of (6) bar diameters (4 inches minimum) K. The exposed face of all embed plates shall be set flush with the face of masonry wall or column.
- 4.4. Minimum Reinforcing
- grouted cells: Wall Thickness Horizontal Reinford #4 @ 48" o.c. 5 @ 48" o.c. :6 @ 48" o.c

5. Structural Steel

- 5.1. Material A. W-Shapes: ASTM A992, ($F_v = 50$ ksi), except as noted otherwise B. All Other Shapes and Plates: ASTM A36 (F_y = 36 ksi), except as noted otherwise
 - Rectangular and Square Hollow Structural Sections (HSS): ASTM A500, Grade C (F_y = 50 ksi) D. Steel Deck: 1. Galvanized Steel Sheet: ASTM A653 or A1063, Grade 50 with G60 galvanized coating.
 - 2. Ungalvanized Steel Sheet: ASTM A1008 or A1039, Grade 50 E. High-Strength Bolts:
 - 1. Group A: ASTM F3125 Grades A325 & F1852 Deformed Bar Anchors (DBA): ASTM A496 or ASTMA1064, 70 ksi minimum yield strength.
- G. Headed Stud Anchors (HSA): ASTM A108, with dimensions complying with AISC specifications H. Anchor Rods: ASTM F1554, Grade 36, unless noted otherwise, with ASTM A563 heavy hex nuts and ASTM F436 hardened washers
- 5.2. Fabrication and construction shall comply with the following Codes and Standards: A. American Institute of Steel Construction (AISC) 1. AISC 360-16, "Specification for Structural Steel Buildings" 2. AISC 303-16, "Code of Standard Practice for Steel Buildings and Bridges"
- B. Research Council on Structural Connections (RCSC), "Specification for Structural Joints Using High-Strength Bolts,"
- dated August, 2014. C. American Welding Society (AWS)
- 1. AWS D1.1–2015: "Structural Welding Code Steel" (specific items do not apply when they conflict with the AISC requirements) D. Steel Joist Institute (SJI)
- 1. SJI 100-20, "45th Edition Standard Specifications, Load Tables and Weight Tables for K-Series, LH-Series, DHL-Series, and Joist Girders"

5.3. Structural shapes and plates shall be fabricated from newly rolled (milled) one-piece sections without splices, unless specifically noted otherwise on the structural drawings. Connections for structural steel shall comply with the structural drawings, unless written approval is given by the Engineer of Record.

- 5.4. Welding A. It is recommended the steel erection contractor and steel fabricator contact the Quality Assurance Agency prior to parties before the welding is started so that correct welds will be made from the beginning. B. Certification of Welders: All shop and field welding shall be executed by AWS certified welders who have been
- AWS Standards. Certification and appropriate records must be provided to the Architect prior to beginning work. Electrodes: E-70 XX or as noted otherwise. E60 XX may be used for welding steel floor and roof decks.
- D. Minimum Welds: All intersecting steel shapes that are not bolted shall be connected by a fillet weld all around, unless connected parts.
- E. Reinforcing Bars: Do not weld rebar except as specifically detailed in the drawings. In such cases, use only AWS standards. Do not substitute reinforcing bars for deformed bar anchors (DBAs), machine bolts, or headed stud anchors
- F. Bolts: Do not apply any welds, including "tack" welds to bolts, including anchor bolts, except as specifically detailed in the
- specifications. Welding shall comply with AWS D1.1 Section 7.6 through 7.9 and Annex G. 5.5. Bolted Connections
- A. Provide snug tightened joints with Group A (threads not excluded) bolts for steel-to-steel connections, unless noted wrench, application of an electric torque wrench until the wrench begins to slow, or the full effort of a worker on an
- ordinary spud wrench. B. Provide hardened washers beneath the turned element of all bolts or nuts. Provide hardened beveled washers, to
- a size sufficient to completely cover the slot after installation. Where a steel-to-steel beam connection is not detailed in the drawings, provide a standard AISC framed connection with specified
- D. Bolts, nuts and washers shall not be reused.
- 5.6. Steel Lintels
- be galvanized unless noted otherwise.
- 5.7. Beam Web Stiffener Plates A. Provide full-height web stiffener plates to each side of all beams above all bearing points. Unless noted otherwise, stiffener plates shall be the thickness indicated in the typical stiffener plate detail.
- 5.8. Open Web Steel Joists and Girders

written consent and direction from the manufacturer

openings with the Mechanical and General Contractor.

noted otherwise on the structural drawings.

Steel Screw Anchor

structural drawings.

IBC Section 2207.5.

C. Anchorage to Concrete:

noted otherwise

6.1. Post-Installed Anchors in Concrete and Masonry

6. Miscellaneous

- of the joist or girder sizes indicated on the framing plan, as if the joists or girders were installed level. B. Open web joists and girders shall be designed with maximum deflection limits of L/240 for total load and L/360 for live
- load, where L is the joist span.

6. Steel Lintels: Provide steel angle lintels at all openings through the masonry veneer. Provide one inch of bearing for each foot of width of opening, with a minimum bearing of six inches. See the STEEL ANGLE LINTEL SCHEDULE for

other suitable devices: at intervals not exceeding the least dimension of the grout lift height, or bar splice locations, or 64 D. Lap Splice Lengths: Lap all masonry reinforcing bars per the "Masonry Reinforcing Bar Lap Splice Schedule." Joint

F. Dowels: All vertical reinforcing shall be doweled to the foundation wall, footing (structure below) and to the structure above with the same size dowel, spacing (and in the same core) as the vertical wall reinforcing unless noted otherwise. G. Wall Openings 24 inches wide and wider: Provide reinforced masonry lintels per Masonry Lintel Schedule over the top of, and 2 - #5 bars, in grouted spaces, on all sides and adjacent to every unscheduled opening, unless noted otherwise. Bars for all openings shall extend a minimum of 24 inches beyond the corners of the opening. Vertical bars shall extend from floor level below to the floor, or roof, level above. Where a 24 inch extension is not possible, extend bars as far

I. Anchor bolts and headed stud anchors shall be set in a grouted cell. Anchor bolts and headed stud anchors shall have 1

All masonry walls shall be reinforced as follows, unless shown otherwise on the drawings. Reinforcing shall be placed in

ing	Vertical Reinforcing
	#5 @ 32" o.c.
	#5 @ 32" o.c.
	#6 @ 32" o.c.
1	#6 @ 32" o.c.

a. The structural drawings shall be used in conjunction with the architectural drawings. Detailing and shop drawing production for structural elements will require information (including dimensions) contained in architectural, structural, and/or other consultants' drawings. Refer to the Special Instructions section of the general notes,

2. SJI 200-15, "2nd Edition Standard Specification, Weight Tables and Bridging Tables for CJ-Series Composite Steel

beginning any welds. A program of joint preparation and welding procedures should be worked out between the two

specifically certified for the process of welding being performed. The welder's certification will be considered as being current unless the welder is not engaged in the process of welding being performed for a period exceeding six months or there is a specific reason to question a welder's ability as required by AWS. Certification and records must comply with

noted otherwise. Fillet weld sizes that are not shown shall be $\frac{1}{16}$ inch less than the thinnest of the connected parts for thicknesses ¼ inch and larger. Fillet welds on plates less than ¼ inch shall be of the same size as the thinnest of the

G. Headed Stud Anchor (HSA) welding and Deformed Bar Anchor (DBA) welding shall conform to the manufacturer's

otherwise. Snug tightened joints shall be used in connections for simple span framing and beam (or girder) to bearing plate connections. Snug tight is the condition that exists when all of the plies in a connection have been pulled into firm contact by the bolts in the joint and all of the bolts in the joint have been tightened sufficiently to prevent the removal of the nuts without the use of a wrench. The snug tightened condition is typically achieved with a few impacts of an impact

compensate for the lack of parallelism, where the outer face of the bolted parts has a slope greater than one in twenty with respect to the plane normal to the bolt axis. Hardened washers or plates installed over oversized holes or slotted holes shall be at least 5/16 inch thick and shall conform to ASTM F436. Plates or bars installed at slotted holes shall have

the capacity to support one half of the total uniform load capacity of the given shape for the span and for the steel

A. Provide steel angle lintels at all openings through the masonry veneer. Provide 1 inch of bearing for each foot of width of opening, with a minimum bearing of 6 inches. See the STEEL ANGLE LINTEL SCHEDULE for size. All steel lintels shall

A. The steel joist supplier shall be responsible for the design of all open web steel joists and girders. Joists or girders with slopes greater than 1/2 inch per foot shall be designed to meet or exceed the load capacities, listed in the SJI load tables,

C. Where uplift loads due to wind are indicated, rows of bottom chord bridging shall be provided at the first bottom chord panel points per SJI Specifications. This is in addition to the bridging shown on the framing plans. Provide special bearing ends to accommodate slopes from sloped joists, sloped girders or sloped bearing conditions. Camber: All joists shall be cambered as specified in the SJI specifications, unless noted otherwise.

Field Modifications: Do not modify any joist or girder, including holes through the top and bottom chords, without the G. Shop Drawings and Design Calculations: Shop drawings for all joists and girders used in the project shall be submitted for review. Prior to the fabrication of joists and girders, the open web steel joist and girder manufacturer shall prepare complete joist and girder calculations under the direct supervision of a Professional Engineer licensed in the State of the Project. Calculations shall be submitted for review for joists and girders designated as SPECIAL or SP and for all joists or girders with axial loads or additional concentrated loads (as noted on the drawings). Submitted calculations shall bear the seal of a Professional Engineer licensed in the State of the Project. H. Stabilizer Plates: Provide steel joist and joist girder stabilizer plates as indicated. Stabilizer plates shall be 6 inch by 6 inch with a ¹³/₁₆ inch diameter hole with 1 ½ inch minimum edge distance, and shall extend 3" minimum below the bottom

chord of the joist or girder. Plate thickness shall be equal to the chord gap minus ¼ inch, or ¾ inch, whichever is less. Verify size, weight, location and configuration of all roof top equipment with Architect and Mechanical Engineer. Provide steel frames for support of roof top equipment as indicated on structural details in the structural drawings. Coordinate All concentrated loads greater than 100 pounds supported by open web steel joists and girders shall be located within 6 inches of joist or girder panel points or the joist or girder shall be reinforced with an additional web member. Refer to the

"TYPICAL DETAIL AT ADDITIONAL CONCENTRATED POINT LOAD" in the structural drawings. K. Concentrated point loads, single or multiple, totaling 100 pounds or less can be located at any point along the top or bottom chord of an open web steel joist or girder between adjacent panel points without meeting the requirements above. A limit of four concentrated 100 pound maximum point loads per joist or girder will be permitted on spans of 12 feet or greater, one concentrated 100 pounds max. load on spans less than 12 feet, unless specifically noted otherwise on the Joist bridging shall never be used to support hanging loads.

Bracing of miscellaneous items (mechanical, electrical, plumbing, etc.) to the bottom chord of joists or girders will not be allowed in any instance. All lateral braces must connect to the top flange/top chord of the framing member above unless N. A certificate of compliance must be submitted to the building official upon completion of fabrication in accordance with

A. Anchorage to hardened concrete and grout-filled masonry shall include all mechanical and adhesive anchors and epoxy doweled reinforcing bars of size, quantity, spacing, and embedment as shown on the drawings. Additional anchors shall not be used without approval from the Engineer of Record prior to installation. B. Special inspection is required during the installation of all post-installed anchors. Refer to applicable code evaluation reports and the Quality Assurance and Statement of Special Inspections sections of the General Structural Notes.

1. All post-installed anchors into hardened concrete shall be selected from the following pre-approved products, unless

Evaluation Report

Hilti Kwik HUS-EZ CC FSR-302 DeWalt Screw-Bolt+ CC ESR-3889 ICC ESR-2713 Simpson Titen HD Steel Expansion/Wedge Anchor Evaluation Report Hilti Kwik Bolt TZ2 ICC ESR-4266 ICC ESR-2502 DeWalt Power-Stud+ SD2 Simpson Strong-Bolt 2 ICC ESR-3037 Adhesive Anchor System Evaluation Report ICC ESR-4868 Hilti HIT-HY 200 CC ESR-5065 Hilti Kwik-X Hilti HIT-RE 500 V3 ICC ESR-3814 DeWalt AC200+ ICC ESR-4027 ICC ESR-3298 DeWalt Pure 110+ Simpson SET-3G ICC ESR-4057

2. Adhesive anchors shall be installed into concrete having a minimum age of 21 days. For installations sooner than 21 days, consult the adhesive manufacturer. D. Anchorage to Masonry: 1. All post-installed anchors into grout-filled masonry shall be selected from the following pre-approved products, unless

noted otherwise:	
Steel Screw Anchor	Evaluation Report
Hilti Kwik HUS-EZ	ICC ESR-3056
DeWalt Screw-Bolt+	ICC ESR-4042
Simpson Titen HD	ICC ESR-1056
Steel Expansion/Wedge Anchor	Evaluation Report
Hilti Kwik Bolt TZ2	ICC ESR-4561
DeWalt Power-Stud+ SD1	ICC ESR-2966
Simpson Wedge-All	ICC ESR-1396
Adhesive Anchor System	Evaluation Report
	ICC ESR-4143
	ICC ESR-4144
DeWalt AC100+ Gold	ICC ESR-3200

- E. Alternate anchors or adhesives are permitted with approval of the Engineer of Record. The Contractor shall submit the proposed anchor product data and code evaluation report demonstrating the anchor is equivalent to or exceeds the capacity of the specified anchor. F. Installation of adhesive anchors horizontally or upwardly inclined to support sustained tension loads shall be performed by personnel certified by an applicable certification program. Certification shall include written and performance tests in
- accordance with the ACI/CRSI Adhesive Anchor Installer Certification program, or equivalent. Proof of current certification shall be submitted to the Engineer of Record for approval prior to commencement of installation. G. Anchors shall be installed according to the Manufacturer's Printed Installation Instructions and applicable code evaluation reports including: 1. Hole diameter, depth, and cleaning procedure
- Adhesive mixing, preparation, and placement Installation torque
- H. Locate all existing reinforcement and embedded items prior to drilling into concrete or masonry elements. Do not damage rebar or embeds while drilling or installing anchors. Grout all defective or abandoned holes with non-shrink grout or an injectable epoxy adhesive matching the surrounding
- concrete compressive strength. Consult the Architect for additional requirements at architecturally exposed concrete. Carbon steel anchors are limited to use in dry, interior locations. K. Holes for post-installed anchors may not be core drilled unless specifically allowed by the manufacturer's installation instructions and the code evaluation report. Holes shall not be re-used unless approved by the manufacturer.

7. Special Instructions

- 7.1. The project specifications are not superseded by the General Structural Notes but are intended to be complementary to them. Consult the specifications for additional requirements in each section. Notes and specific details on the drawings shall take precedence over General Structural Notes and typical details.
- 7.2. The architectural drawings are the prime contract drawings. Consultant drawings by other disciplines are supplementary to the architectural drawings. All omissions or conflicts, including dimensions, between the various elements of the consultants' drawings and/or specifications shall be brought to the attention of the Architect before proceeding with any work involved. In case of conflict, follow the most stringent requirement as directed by the Architect without additional cost to the Owner. Any work done by the Contractor after discovery of such discrepancy shall be done at the Contractor's risk.
- 7.3. The structural drawings shall be used in conjunction with the architectural drawings. Primary structural elements and overall structural layout are indicated within the structural plans and details. Some secondary elements, architectural layouts, alcoves, elevations, slopes, depressions, curbs, mechanical equipment and electrical equipment, are not indicated within the structural drawings. Detailing and shop drawing production for structural elements will require information (including dimensions) contained in the architectural, structural and/or other consultants' drawings.
- 7.4. Shoring and Bracing Requirements A. Floor and Roof Structures -- The General Contractor is responsible for the method and sequence of all structural erection. The Contractor shall provide temporary shoring and bracing as the method of erection requires to provide adequate vertical and lateral support. Shoring and bracing shall remain in place as the chosen method requires until all
 - permanent members are in place and all final connections are completed, including all roof and floor attachments. The building shall not be considered stable until all connections are complete. B. Foundation walls must be braced until the complete floor or roof systems is completed. Do not backfill until the floor or
- roof systems are in place C. Walls above grade shall be braced until the structural system is complete. Walls shall not be considered to be self-D. restored with similar materials and workmanship to levels acceptable to the Owner.
- 7.5. Submittals: A copy of all shop drawings that have been submitted for review must be kept at the construction site for reference. These drawings must bear the appropriate review stamps. The shop drawing review shall not relieve the Contractor of the responsibility of completing the project according to the contract documents. The General Contractor shall review and mark all shop drawings prior to submitting them to the Architect for review. Shop drawings made from reproductions of (these) contract drawings will be rejected.
- 7.6. Project Coordination: It shall be the responsibility of the General Contractor to coordinate with all trades any and all items that are to be integrated into the structural system. Openings or penetrations through, or attachments to the structural system that are not indicated on these drawings shall be the responsibility of the General Contractor and shall be coordinated with the Architect/Engineers. The order of construction is the responsibility of the General Contractor. It is the Contractor's obligation to provide all items necessary for the chosen procedure.
- 7.7. Contractor shall field verify all dimensions, and conditions. If the contract drawings do not represent actual conditions, Contractor shall notify Architect/Engineer prior to fabrication or construction within that area.
- 7.8. Notice of Copyright: The structural drawings, plans, schedules, notes and details are hereby copyrighted by Reaveley Engineers. Submission or distribution of documents to meet official regulatory requirements or for similar purposes in connection with the project is not to be construed as publication in derogation of Reaveley Engineers' reserved rights. The documents defining the structure are instruments of service prepared by Reaveley Engineers for one use only. Furthermore, these documents shall not be reproduced, or copied, in whole or in part by the Contractor or subcontractors for preparation of shop drawings or other submittals.

8. Quality Assurance

8.1. Quality Assurance Agency Requirements A. The Owner shall engage a qualified Quality Assurance Agency (QAA) to provide all special inspection and quality assurance testing for the project. The QAA shall provide all information necessary for the building official to determine that the agency meets the applicable requirements.

- 1. The QAA shall be objective, competent and independent from the Contractor responsible for the work being inspected. The agency shall disclose to the building official and the registered design professional in responsible charge possible conflicts of interest so that objectivity can be confirmed. 2. The QAA shall have adequate equipment to perform required tests. The equipment shall be periodically calibrated.
- 3. The QAA shall employ experienced personnel educated in conducting, supervising and evaluating tests and special inspections. Experience or training shall be considered relevant where the documented experience or training is related in complexity to the same type of special inspection or testing activities for projects of similar complexity and material qualities.
- 4. The QAA shall send copies of all inspection and testing reports to the building official, Owner, Architect, Engineer of Record and Contractor. Reports shall indicate that the work inspected was or was not completed in conformance to the approved construction documents. Discrepancies shall be brought to the immediate attention of the Contractor for correction. If they are not corrected, the discrepancies shall be brought to the attention of the Architect and Engineer
- of Record 5. The QAA shall submit a final report documenting required special inspections and tests, and correction of any discrepancies noted in the inspections or tests. The final report shall be distributed to the building official, Owner, Architect and Engineer of Record in a timely manner prior to the completion of the project.

8.2. Contractor Responsibilities

- A. The Contractor shall submit a written statement of responsibility to the building official and the Owner or the owner's authorized agent prior to the commencement of work on the systems or components listed in the statement of special inspections. The Contractor's statement of responsibility shall contain acknowledgement or awareness of the special requirements contained in the statement of special inspections. B. Notification of QAA: The Contractor shall notify the QAA in a timely manner so that inspection and testing may be
- performed as outlined in the statement of special inspections.
- 8.3. Structural Observations by the Engineer of Record A. The Engineer of Record will perform structural observations at critical phases of the project as listed below. Observations will be made on a periodic basis throughout the construction of the structural system. Copies of the observation report will be distributed to the Architect, Contractor, Owner, and QAA.
- B. The contractor shall notify the Engineer of Record at least 24 hours in advance before any of the following actions. 1. Placing concrete in any footing, mat footing, deep foundation, grade beam, or pier. Closing any wall forms. Grouting any masonry.
- 4. Completing the structural steel framing.
- 5. Completing the welding or fastening of major sections of steel decking. 6. Completing the fastening of any sheathing on wall, roof, or floor system.
- C. The Engineer of Record will perform a structural observation at a critical phase of the project. Copies of the Engineer's report will be distributed to the Architect, Contractor, Owner, and QAA. D. The contractor shall notify the Engineer of Record at least 24 hours before performing items listed in 8.3B. . Observation visits to the site by the Engineer of Record or the Engineer's field representatives shall not be construed as inspection or approval of construction.

9. Statement of Special Inspections

9.1. The following materials, systems and components require special inspection or testing per Chapter 17 of the International Building Code (IBC).

- 9.2. For items requiring continuous inspection, a special inspector must be present onsite during the performance of that task. In most cases, periodic inspections/tests shall be performed prior to commencing the task, intermittently during the task, and at the completion of the task. Frequency marked with (E) designates periodic inspections that must be performed prior to or upon completion of every task and for each member, welded joint, and bolted connection.
- 9.3. Special inspections during fabrication are not required where the work is done on the premises of a fabricator approved by the authority having jurisdiction to perform such work without special inspection. At the Owner's discretion, periodic inspection, testing, or auditing of the fabricator's quality control processes may be performed by the Owner's quality assurance agency. At completion of fabrication, the fabricator shall submit a certificate of compliance for submittal to the building official, as specified in Section 1704.5, stating that the work was performed in accordance with the approved construction documents.

Structural Steel per IBC Section 1705.2.1, 1705.13.1 & 1705.14.1 Detailed Instructions Item Frequency

Prior to Welding (Table N5.4-1, AISC 360-16):		
Welder qualification records	Periodic	Verify welder qualification records and continuity records
Verify welding procedures (WPS) and consumable certificates	Periodic (E)	
		_

ARCHITECTS **MHTN Architects, Inc** 280 South 400 West, Suite 250 Salt Lake City, Utah 84101 Telephone (801) 595-6700 Telefax (801) 595-6717 www.mhtn.com

515 East 100 South, Suite 1200 Salt Lake City, Utah 84102 801 486 3883 www.reaveley.com

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MHTN PROJECT NO. 2017559

Original drawing is 30 x 42. Do not scale contents of this drawing. CONTRACTOR TO VERIFY DRAWINGS IN FIELD USE REFLECT LAST REVISION DATE NO. A DATE DESCRIPTION

BID PACKAGE #1 MAY 12, 2025

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Material identification Welder identification	Periodic Periodic	Verify type and grade of material. Confirm a system is in place by which a welder who has welded a joint or member can be identified
Fit-up groove welds	Periodic	Verify joint or member can be identified. Verify joint preparation, dimensions, cleanliness, tacking, and backing.
Fit-up of CJP welds to HSS T-, Y- and K- oints without backing Access holes	Periodic Periodic	Verify joint preparation, dimensions, cleanliness and tacking Verify configuration and finish
Fit-up of fillet welds	Periodic	Verify dimensions, cleanliness and tacking.
During Welding (Table N5.4-2, AISC 360-16): Use of qualified welders	Periodic	Verify that welders are appropriately qualified.
Control and handling of welding consumables	Periodic	Verify packaging and exposure control.
Cracked tack welds Environmental conditions	Periodic Periodic	Verity that welding does not occur over cracked tack welds. Verify wind speed is within limits as well as precipitation
WPS followed	Periodic	Verify items such as settings on welding equipment, travel speed, welding materials, shielding gas type/flow rate,
Molding toobniquos	Poriodio	preheat applied, interpass temperature maintained, and proper position.
Steel headed stud anchors	Periodic	verify interpass and inflat cleaning, each pass is within profile limitations, and quality of each pass. Verify placement and installation of steel headed stud
After Welding (Table N5.4-3. AISC 360-16)		anchors. Verify all stud welds show a full 360° flash.
Welds cleaned Size, length, and location of welds	Periodic Periodic (E)	Verify that welds have been properly cleaned. Verify the size, length and location of welds.
Welds meet visual acceptance criteria	Periodic (E)	Verify that welds meet crack prohibition, base metal fusion, profile, size, undercut, and porosity provisions.
Arc strikes	Periodic (E)	Verify that arc strikes do not exist outside the permanent weld areas. When welding of doubler plates, continuity plates or
		stiffeners has been performed in the k-area, visually inspect the web k-area for cracks.
Backing & weld tabs removed Repair activities	Periodic (E)	If required on the approved construction documents, verify that back and weld tabs are removed. Verify that repair activities are performed in accordance
Documentation	Periodic (E)	with AISC 360 and AWS D1.1. Document the acceptance or rejection of the welded joint of
Prohibited welds	Periodic (E)	Verify no prohibited welds have been added without approval of the EOR.
Nondestructive Testing (Section N5.5, AISC 36 CJP welds (Risk Cat II)	i0-16): Periodic	UT testing shall be performed on 10% of CIP groove welde
		in butt, T- and corner joints subject to transversely applied tension loading in materials 5/16-inch thick or greater. (This
Welded joints subject to fatique	Periodic (E)	Must be performed on 100% of CJP welds in SDC 'D-F' per AISC 341.) Welded joints subject to fatigue (see Table A-3.1 of AISC
Pre-production tests of steel headed stud	Start of shift	360) shall have radiographic or UT testing. Before production welding with a particular set-up and with
andiois		a given size and type of stud, and at the beginning of each day's or shift's production, testing shall be performed on the first two studs that are welded per AWS 9.7.1. Studs
		shall exhibit full 360° flash with no evidence of undercut into the stud base. Each stud shall be bent to an angle of approximately 30° from its original axis per AW/S 9.7.1.4
Bend tests of steel headed stud anchors	Periodic	Test all studs that do not show a 360° flash and all studs that have been repaired by welding. Each stud shall be
		bent to an angle of approximately 15° from its original axis. Test per AWS 9.7.1.4. The direction of bending for studs with less than 360° flash shall be opposite to the missing
		point of the flash. Test a reasonable number of additional studs.
Prior to Bolting (Table N5.6-1, AISC 360-16): Certifications of fasteners	Continuous	Verify that manufacturer's certificates are available for
Fasteners marked	Periodic	fastener materials. Verify that fasteners have been marked in accordance with
Proper fasteners for joint	Periodic	Verify grade, type, and bolt length if threads are excluded from the shear plane.
Proper bolting procedure Connecting elements	Periodic Periodic	Verify proper procedure is used for the joint detail.Verify appropriate faying surface condition and hole
Pre-installation verification testing	Periodic	 preparation, if specified, meet requirements. Observe and document verification testing by installation personnel for fastener assemblies and methods used
Proper storage	Periodic	Verify proper storage of bolts, nuts, washers, and other fastener components.
During Bolting (Table N5.6-2, AISC 360-16): Fastener assemblies	Periodic	Verify that fastener assemblies are of suitable condition,
Snug-tight prior to pretensioning	Periodic	paced in all holes, and washers and nuts are positioned as required.
Fastener component	Periodic	Verify that joints are brought to shug-tight condition prior to pretensioning operation. Verify that fastener component not turned by wrench is
Pretensioned fasteners	Periodic	prevented from rotating. Verify that fasteners are Pretensioned in accordance with RCSC Specification, progressing systematically from the
		most rigid point toward the free edges.
After Bolting (Table N5.6-3, AISC 360-16): Documentation	Periodic (E)	Document the acceptance or rejection of bolted connections.
Other Steel Inspections (Section N5.8, AISC 38	60-16: Table J8.1, J10.1,	AISC 341-16):
	Fellouic	verify compliance with the details shown in the approved construction documents, such as braces, stiffeners,
Anchor rods and other embedments	Periodic	member locations, and proper application of joint details at each connection.
supporting structural steel		rods and other embedments supporting structural steel for compliance with construction documents. Verify the
		diameter, grade, type, and length of the anchor rod or embedded item, and the extent or depth of embedment prior to placement of concrete.
Galvanized structural steel	Periodic	Verify that exposed cut surfaces of galvanized structural steel does not include cracks prior to galvanizing the
Composite Construction — Steel & Concrete (Table J9.1, J9.2 7 J9.3 AI	SC 341-16):
Prior to concrete placement	Continuous	Verify type and grade of reinforcing steel; carbon equivalen if other than A706 bars; proper reinforcing steel size, spacing and orientation; that has had not been used by
		is tied and supported; proper clearances are provided; and composite member has required size.
During concrete placement	Periodic	Verify appropriate mix design; limitations on water added to truck/pump; and proper placement techniques are used to limit segregation
After concrete placement	Periodic	Document that minimum concrete compressive strength was achieved at specified age.
el Roof and Floor Decks per IBC Section 17	05.2.2 and SDI QA/QC -	2017
Steel Roof and Floor Decks Prior to Placement	ггеquency : (IBC 1705.2.2 and Table	Detailed Instructions <u>1.1, SDI QA/QC 20</u> 17):
Materials	Periodic (E)	Verify compliance of deck and all deck accessories with approved construction documents, including profiles, material
Documentation	Periodic (E)	Document acceptance or rejection of deck and deck accessories
Steel Roof and Floor Decks After Placement (In	BC 1705.2.2 and Table 1. Periodic (F)	.2, SDI QA/QC 2017): Verify compliance of deck and all deck accessories installation
Compliance with construction documents		with construction documents. Verify deck materials are represented by the mill certifications that comply with the
Document acceptance or rejection of deck and deck accessories	Periodic (E)	construction documents.
Steel Roof and Floor Decks Prior to Welding (II	BC 1705.2.2 and Table 1.	.3, SDI QA/QC 2017):
Certifications of welding consumables	Periodic Periodic	Verify that WPS is available.
Material identification	Periodic	Consumables are available. Verify type and grade of materials to be welded
Steel Roof and Floor Decks During Welding //E	C 1705 2 2 and Table 1	4. SDI QA/QC 2017) [.]
Use of qualified welders	Periodic Periodic	Verify that welders are appropriately qualified.
Environmental conditions	Periodic	Verify wind speed is within limits as well as precipitation and temperature.
WPS followed	Periodic	Verify items such as settings on welding equipment, travel speed, welding materials, shielding gas type/flow rate, prehea
		applied, interpass temperature maintained, and proper position.
<u>Steel Roof and Floor Decks After Welding (IBC</u> Size, length, and location of welds	2 1705.2.2 and Table 1.5, Periodic	SDI QA/QC 2017): Verify size and location of welds, including support, sidelap, and perimeter welds
Welds meet visual acceptance criteria	Periodic (E)	Verify weld meets visual acceptance criteria based upon weld/base-metal fusion, weld profiles, weld size, undercut.
Repair activities	Periodic (E)	and porosity. Verify that repair activities are acceptable.
Document acceptance or rejection of welds Steel Roof and Floor Decks Prior to Mechanica	Periodic (E) al Fastening (IBC 1705.2.	2 and Table 1.6, SDI QA/QC 2017):
Pre-installation verification	Periodic	Verify manufacturer installation instructions are available for mechanical fasteners as well as the proper tools and storage
Steel Roof and Floor Decks During Mechanical	 Fastening (IBC 1705.2.2	Tor the fasteners. 2 and Table 1.7, SDI QA/QC 2017):
	LEINOIC	in accordance with the manufacturer's instructions.

Steel Roof and Floor Decks After Mechanical Fastening (IBC 1705.2.2 and Table 1.8, SDI QA/QC 2017): Spacing, type and installation of fasteners Verify the spacing, type and installation of support, sidelap Periodic (E) and perimeter fasteners.

Repair activities Periodic (E) Document acceptance or rejection of Periodic (E) mechanical fasteners Open-web Steel Joists and Joist Girders per IBC 1705.2.3 Frequency End connections — welded or bolted Periodic Bridging — horizontal or diagonal Periodic Concrete Construction per IBC Sections 1705.3 & 1705.12 Frequency Reinforcing steel Periodic Welding of reinforcing steel Periodic Cast-in bolts & embeds Periodic Post-installed adhesive anchors installed in Continuous horizontally or upwardly inclined orientations to resist sustained tension loads Periodic Post-installed mechanical anchors and adhesive anchors not defined above Periodic Use of required mix design Concrete sampling for strength tests, slump, Continuous air content, and temperature Concrete placement Continuous Periodic Curing temperature and techniques In-situ strength verification Periodic Periodic Formwork Masonry Construction per IBC Section 1705.4 Frequency Prior to Construction (Table 3, TMS-602-16): Review material certificates, mix designs, Periodic test results and construction procedures Periodic Verify f'_m prior to construction As Masonry Construction Begins (Table 4, TMS-602-16): Proportions of site-prepared mortar Periodic Reinforcement, connectors, and Periodic

Grade, type, and size of reinforcement and | Periodic anchor bolts Placement of reinforcement and Periodic connectors and anchor bolts Placement of masonry units and Periodic construction of mortar joints During Masonry Construction (Table 4, TMS-602-16): Materials and procedures Periodic Periodic Placement of masonry units and mortar joint construction Size and location of structural elements Periodic Type, size, and location of anchors, Periodic including other details of anchorage of masonry to structural members, frames, or other construction. Welding of reinforcement Continuous Preparation, construction, and protection Periodic of masonry during cold weather (<40°F) or hot weather (>90°F). Periodic Observation of grout specimens, mortar specimens, and/or prisms Self-consolidating grout Continuous Minimum Testing: Verification of Slump Flow and Visual Periodic Stability Index (VSI) for self-consolidating grout Verification of f'm Periodic

Periodic

Periodic

Periodic

Periodic

Verification of proportions of materials in Periodic premixed or pre-blended mortar and grout Post-installed anchors or dowels

Soils per IBC Section 1705.6

anchorages

Sample panel

observed

Grout space

Construction of mortar joints

Preparation of required grout specimens,

mortar specimens and/or prisms shall be

Prior to Grouting (Table 4, TMS-602-16):

Item	Frequency
Verify subgrade is adequate to achieve design	Periodic
Verify excavations extend to proper depth and	Periodic
material	
Verify that subgrade has been appropriately prepared prior to placing compacted fill	Periodic
Perform classification and testing of compacted fill materials	Periodic
Verify proper materials, densities and lift thicknesses during placement and compaction.	Continuous
Aggregate Piers	Continuous

Detailed Instructions Visual inspection to confirm that end connections conform to the approved construction documents. Visual inspection to confirm that bridging is provided per the approved construction documents.

Detailed Instructions Verify prior to placing concrete that reinforcing is of specified type, grade and size; that it is free of ice, mud, oil,

or other deleterious coatings that decrease bond. Prestressing reinforcement shall be free of mill scale, pitting, and excessive rust. A light coating of rust shall be permitted; that it is located and spaced properly; that hooks, bends, ties, stirrups and supplemental reinforcement are placed correctly; that lap lengths, stagger and offsets are provided; that all mechanical connections are installed per the manufacturer's instructions and/or evaluation report, and that minimum clear spacing requirements between bars and lap splices are in accordance with the Detailing provisions of the General Structural Notes.

Visually inspect all welds and also verify weldability of reinforcing steel based upon carbon equivalent and in accordance with AWS D1.4. Inspection of anchors or embeds cast in concrete.

All post-installed anchors/dowels shall be specially inspected as required by the approved ICC-ES report. Horizontally or upwardly inclined anchors that resist sustained tension loads require continuous inspection and approved installers.

Verify that all mixes used comply with the approved construction documents; ACI 318: Ch.19, 26.4.3-26.4.4; and IBC 1904.1.1904.2 Samples for strength tests shall be taken in accordance with ASTM C172, cured per ASTM C31 and tested in accordance with ASTM C39 by a testing agency complying with ASTM C1077. Acceptance criteria for strength tests shall be per ACI 318 Section 26.12.3. For each mix placed, samples shall be taken not less than once a day, nor less than once for each 150 yd³ of concrete, nor less than once for each 5000 ft² of surface area for slabs or walls. At the time fresh concrete is sampled to fabricate specimens for strength tests, perform slump and air content tests and determine the temperature of the concrete.

Verify that the ambient temperature for concrete is kept at > 50°F for at least 7 days after placement. High-earlystrength concrete shall be kept at > 50°F for at least 3 days. Accelerated curing methods may be used (see ACI 318: 26.5.3).All concrete materials, reinforcement, forms, fillers, and ground shall be free from frost. In hot weather conditions ensure that appropriate measures are taken to avoid plastic shrinkage cracking and that the specified water/cement ratio is not exceeded. Verify that adequate strength has been achieved prior to the removal of shores and forms or the stressing of posttensioned tendons.

Verify that the forms are placed plumb and conform to the shapes, lines, and dimensions of the members as required by the approved construction documents.

Detailed Instructions

Verify that materials conform to the approved construction documents. Mix design, test results, material certificates, and construction procedures should be submitted to inspector for review. Mortar mix designs shall conform to ASTM C 270 while grout shall conform to ASTM C 476. Material certificates shall be provided for reinforcement; anchors, ties, fasteners, and metal accessories; masonry units; mortar and grout materials. Construction procedures for cold-weather or hot-weather construction shall be

reviewed. Determine the compressive strength for each wythe by the "unit strength method" or by the "prism test method" as specified in Section 1.4B of ACI 530.1-13 prior to construction.

Verify that mortar is of the type and color specified on the construction documents, that it conforms to ASTM C 270, and that it is mixed in accordance with Article 2.6 A of TMS-602-16. Verify that reinforcement, connectors, and anchorages are of the proper grade, type and size in accordance with Article 3.4 of TMS-602-16. Prestressing tendons shall be

placed per Article 3.6 A. Verify that sample panels are properly constructed and that subsequent work conforms per Article 1.6 D of TMS-602-16. (If Risk Category IV this should be performed on a continuous basis.) Verify that mortar joints comply with Article 3.3 B of TMS-602/ACI 530.1-13.

If the prism test method is used a minimum of three prisms shall be constructed in accordance with ASTM C1314. If the unit strength method is selected the compressive strength of the grout shall be determined per ASTM C1019 (not required if grout complies with ASTM C476).

Verify that grout space is free of mortar droppings, debris, loose aggregate, and other deleterious materials and that cleanouts are provided per Article 3.2 D and 3.2 F of TMS-602-16.

Verify that reinforcement, joint reinforcement, wall ties, anchor bolts and veneer anchors comply with the approved construction documents and Section 1.6 of TMS 402/ACI <u>530-13.</u> Verify that reinforcement, joint reinforcement, wall ties,

anchor bolts and veneer anchors are installed in accordance with the approved construction documents and Articles 3.2 E and 3.4 of TMS-602-16. Verify that face shells and head joints are fully mortared and that vertical cells are aligned and unobstructed openings for grout are provided. All units are to be clean and placed while mortar is soft and plastic. Verify that mortar joints are placed in accordance with Article 3.3 B of TMS 602/ACI 530.1-13.

Ensure that materials and procedures conform to the approved construction documents and Article 1.5 of TMS-602-16. Verify that masonry units are properly placed and that mortar joint construction conforms to Article 3.3 B of TMS-602-16. Verify the locations of structural elements with respect to the approved plans and confirm that tolerances meet the requirements of Article 3.3 F of TMS-602-16. Verify that correct anchorages and connections are provided per the approved plans and Sections 1.2.1, 6.2.1 and 6.3.1 of TMS-402-16.

Verify that cold-weather construction is performed in accordance with Article 1.8 C of TMS-602-16 and hot weather construction per Article 1.8 D of TMS-602-16. Confirm that specimens/prisms are performed as required by Article 1.4 B of TMS-602-16.

Compressive strength tests should be performed in accordance with ASTM C 1019 for slump flow and ASTM C 1611 for VSI. Determine the compressive strength for each wythe by the "unit strength method" or by the "prism test method" as specified in Article 1.4 B of TMS 602-16 prior to construction. (For Risk Category IV buildings this should be verified at every 5,000ft² of construction.) Verify that proportions for mortar meet ASTM C 270 and proportions for grout meet ASTM C 476. All post-installed anchors/dowels shall be specially inspected as required by the approved ICC-ES report.

Detailed Instructions Prior to placement of concrete. Prior to placement of compacted fill or concrete. Prior to placement of compacted fill. All materials shall be checked at each lift for proper classifications and gradations not less than once for each 10,000 ft² of surface area.

Confirm installation meets design requirements

Architectural Components (Seismic Design Category D, E, or F) per IBC Sections 1705.13.5

	Item	Frequency	Detailed Instructions
	Erection and fastening of exterior cladding or interior and exterior veneers	Periodic	Verify appropriate materials, fasteners, and attachment at commencement of work and at completion. Performed by code inspection firm. (Not required if height of wall is less than 30 feet above grade or walking surface, or less than 5 psf)
	Erection and fastening of interior and exterior nonbearing walls	Periodic	Verify appropriate materials, fasteners and attachment at commencement of work and at completion. <i>Performed by code inspection firm.</i> (Not required if height of wall is less than 30 feet above grade or walking surface, or less

than 15 psf)

ARCHITECTS MHTN Architects, Inc. 280 South 400 West, Suite 250 Salt Lake City, Utah 84101 Telephone (801) 595-6700 Telefax (801) 595-6717 www.mhtn.com

515 East 100 South, Suite 1200 Salt Lake City, Utah 84102 801 486 3883 www.reaveley.com

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	PLAN LEGE	ND	
	FOOTING STEP		CONCRETE WALL
	FOOTING - CONTINUOUS		CONCRETE WALL - RECESSED (FDTN PLAN)
	FOOTING - THICKENED SLAB		CONCRETE WALL - RECESSED AT DOOR
]	FOOTING - SQUARE FOOTING - RECTANGULAR FOOTING - MAT FOOTING		CONCRETE PIER IN CONCRETE WALL. TOP OF PIER RECESSED BELOW SLAB.
/////.	CHANGE IN ELEVATION		CONCRETE COLUMN
<u> </u>	SLAB BLOCK-OUT AT COLUMN	4-	CONCRETE JAMB COLUMN POURED MONOLITHIC WITH CONCRETE WALL
/			MASONRY WALL
	SLAB CONTROL/CONSTRUCTION JOINT		MASONRY WALL - RECESSED (FDTN PLAN) MASONRY LINTEL (FRAMING PLAN)
			MASONRY COLUMN IN MASONRY WALL
		E = = = 3	NON-BEARING MASONRY WALL
	SPECIAL SLAB OR DECK AREA		NON-BEARING MASONRY LINTEL
	SPECIAL SLAB OR DECK AREA		STEEL STUD WALL - STRUCTURAL
			STEEL HEADER IN STEEL STUD WALL
\bigotimes	SPECIAL SLAB OR DECK AREA	<u> </u>	BRICK WALL
		ZA PZ	BRICK WALL - RECESSED (FDTN PLAN) BRICK LINTEL (FRAMING PLAN)
	RECESSED/DEPRESSED SLAB		BRICK COLUMN IN BRICK WALL
			STEEL BEAM OR GIRDER
	OF LINING		STEEL JOIST OR PURLIN
]===[CROSS BRIDGING
4	CONCILITE HOUSEKEEP ING FAD		HORIZONTAL BRIDGING
	CONCRETE BEAM		STEEL COLUMN - TUBE (HSS)
	CONCRETE SUSPENDED SLAB	I	STEEL COLUMN - WIDE FLANGE
	-CONCRETE OVER STEEL DECK		
	-STEEL DECK		

SHT N SE001 SE002 SE003 SB100A SB100B SB100C SB100D

SB501 SB502 SB601 SB602 SB611 SF101A SF101B

SF101C SF101D

SF102B SF501 SF511 SF512 SF601 SF602

MBER	DESIGNATION	

-TEXT INDICATES ADDITIONAL REQUIREMENTS FOR BOLTS AT SPECIFIED CONNECTION: SC = SLIP CRITICAL BOLTS

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PT = FULLY PRETENSIONED BOLTS SS = SHORT SLOTTED HOLES

LS = LONG SLOTTED HOLES

-INDICATES STEEL JOIST, BEAM OR GIRDER SIZE -INDICATES MEMBER IS PART OF LFRS, (USE PT BOLTS AT EACH END UNO)

-NUMBER OF HEADED STUDS ON BEAM — POSITIVE OR UPWARD CAMBER AT MIDSPAN

-WHERE NOTED, NUMBER INDICATES DIMENSION FROM TOP OF STEEL ELEVATION REFERENCED ON PLAN (TOST). SEE ARCH FOR DECK BEARING ELEVATIONS AT SLOPED ROOF FRAMING.

-CONNECTION TYPE. SEE FRAMING CONNECTION LEGEND. IF ABSENT, PROVIDE TYPICAL SINGLE PLATE CONNECTION.

S	TRUCTURAL DRAWING LIST
10.	SHT NAME
	GENERAL STRUCTURAL NOTES
	GENERAL STRUCTURAL NOTES
	LEGENDS & ABBREVIATIONS
٨	FOOTING & FOUNDATION PLAN - TREASURE SUPPORT
3	FOOTING & FOUNDATION PLAN - PRESS BOX
)	FOOTING & FOUNDATION PLAN - TEAM ROOM/DUGOUT
)	FOOTING & FOUNDATION PLAN - DUGOUT
	TYPICAL FOOTING & FOUNDATION DETAILS
	FOOTING & FOUNDATION DETAILS
	CONCRETE SCHEDULES
	REINFORCING SCHEDULES
	MASONRY SCHEDULES
١	ROOF FRAMING PLAN - TREASURE SUPPORT
}	SECOND FLOOR/ROOF FRAMING PLAN - PRESS BOX
)	ROOF FRAMING PLAN - TEAM ROOM/DUGOUT
)	ROOF FRAMING PLAN - DUGOUT
}	UPPER ROOF FRAMING PLAN - PRESS BOX
	FLOOR FRAMING DETAILS
	TYPICAL ROOF FRAMING DETAILS
	ROOF FRAMING DETAILS
	TYPICAL STEEL FRAMING SCHEDULES
	STEEL DECK SCHEDULES

AT ANCHOR BOLT (S) IAB ABOVE ABV ALTERNATE AI T APPROXIMATE APPROX ARCH ARCHITECT(URAL) BLDG BUILDING BLW BELOW BM BEAM BOT BOTTOM BRG BEARING BTWN BETWEEN CONSTRUCTION JOINT OR CONTROL CJ JOINT C.IP COMPLETE JOINT PENETRATION CMU CONCRETE MASONRY UNIT COL COLUMN CONCRETE CONC CONST CONSTRUCTION CONT CONTINUOUS CONTR CONTRACTOR CTR CENTER D.B. DECK BEARING DIAMETER OF REINFORCING BAR db DBA DEFORMED BAR ANCHORS DBL DOUBLE DET DETAIL DIAMETER DIA (OR Ø) DIAGONAL DIAG DIM DIMENSION DECK DK DOWN DN DRAWING DWG DWL DOWEL EACH FACE EXPANSION JOINT (SEISMIC SEPARATION JOINT) EACH WAY E.W. EACH FΑ ELEVATION ELEC ELECTRICAL ELEV ELEVATOR ENG ENGINEER EQ EQUAL EQUIPMENT EQUIP EXIST (E) EXISTING EXPANSION / EXPOSED EXP EXT EXTERIOR F.D. FLOOR DRAIN FINISH FLOOR F.V. FIELD VERIFY FDTN FOUNDATION FIN FINISH FLOOR FOOT FOOTING FTG GAUGE GA GALV GALVANIZED GLU-LAMINATED BEAM GLB GR GRADE GSN GENERAL STRUCTURAL NOTES HORIZONTAL BRIDGING HORIZ HORIZONTAL HSA HEADED STUD ANCHORS HSS HOLLOW STRUCTURAL STEEL HEIGHT INSIDE FACE INTERNATIONAL BUILDING CODE IBC INTERNATIONAL CODE COUNCIL ICC INCH INSUL INSULATION INT INTERIOR JST JOIST JOINT KIPS - 1,000 POUNDS KIPS PER LINEAL FOOT KLF KSF KIPS PER SQUARE FOOT KSI KIPS PER SQUARE INCH LBS POUNDS SEE CONCRETE REINFORCING BAR Ld, Lt, Lsb, Lsbt, Ldc, Lsc DEVELOPMENT AND LAP LENGTH SCHEDULE LINEAL FOOT LFRS LATERAL FORCE RESISTING SYSTEM (SFRS & WFRS) LONG LEG HORIZONTAL LLH LONG LEG VERTICAL LLV LSH LONG SIDE HORIZONTAL LSV LONG SIDE VERTICAL MAS MASONRY MAX MAXIMUM MCJ MASONRY CONTROL JOINT MECH MECHANICAL MFGR MANUFACTURER MIN MINIMUM MISC MISCELLANEOUS NIC NOT IN CONTRACT NORM NORMAL NTS NOT TO SCALE ON CENTER 0.C. OUTSIDE FACE 0.F. OPNG OPENING OPP OPPOSITE OPEN WEB STEEL JOIST OWSJ POST-TENSIONED P.T. POWDER ACTUATED FASTENER PAF POUNDS/CUBIC FOOT PCF PARTIAL JOINT PENETRATION PJP PLATE POUNDS/LINEAL FOOT PLF PANEL PNL POUNDS/SQ FOOT PSF POUNDS/SQ INCH PSI R.D. ROOF DRAIN

ABBREVIATIONS

	ABBREVIATIONS
EINF	REINFORCING
EQD	REQUIRED
DS	SELF-DRILLING SCREW
FRS	SEISMIC FORCE RESISTING SYSTEM
HT	SHEET
I	SPECIAL INSPECTION (SP. INSP.)
IM	SIMILAR
OG	SLAB ON GRADE
Q	SQUARE
TAG	STAGGERED
TD	STANDARD
TIFF	STIFFENER
TL	STEEL
TRUCT	STRUCTURAL
& B	TOP AND BOTTOM
.0.	TOP OF
EMP	TEMPERATURE
HDS	THREADS
OC	TOP OF CONCRETE
OCP	TOP OF CONCRETE PIER
OF	TOP OF FOOTING
OS	TOP OF SLAB
OST	TOP OF STEEL
OW	TOP OF WALL
YP	TYPICAL
NO	UNLESS NOTED OTHERWISE
ERT	VERTICAL
√.P.	WORK POINT
//	WITH
/F	WIDE FLANGE
/FRS	WIND FORCE RESISTING SYSTEM
/T	WEIGHT
WF	WELDED WIRE FABRIC
D	YARD

	PLAN MARKS
BF-#	BRACED FRAME
CB-#	CONCRETE BEAM
CC-#	CONCRETE COLUMN
CCSS-#	CANTILEVERED CONCRETE SUSPENDED SLAB
CDP-#	CONCRETE DRILLED PIER
CFW-#	CONCRETE FOUNDATION WALL
CGB-#	CONCRETE GRADE BEAM
CJ-#	CONCRETE JOIST
CJC-#	CONCRETE JAMB COLUMN
CL-#	CONCRETE LINTEL
CP-#	CONCRETE PIER
CRW-#	CONCRETE RETAINING WALL
CSG-#	CONCRETE SLAB ON GRADE
CSH-#	CONCRETE SHEAR HEAD
CSS-#	CONCRETE SUSPENDED SLAB
CSW-#	CONCRETE SHEAR WALL
CW-#	CONCRETE WALL
FC#	CONTINUOUS FOOTING
FM#	MAT FOOTING
FR#	RECTANGULAR FOOTING
FS#	SQUARE FOOTING
FTS#	THICKENED SLAB FOOTING
HD-#	HOLD DOWN ANCHOR
MC-#	MASONRY COLUMN
MF-#	MOMENT FRAME
ML-#	MASONRY LINTEL
MP-#	MASONRY PIER
MW-#	MASONRY WALL
PTB-#	POST-TENSIONED CONCRETE BEAM
SBP-#	STEEL BASE PLATE
SC-#	STEEL COLUMN
SCP-#	STEEL CAP PLATE
SD-#	STEEL DECK
SDA-#	STEEL DECK ATTACHMENT
SG-#	STEEL GIRDER
SJ-#	STEEL JOIST
SND-#	
VVB-#	
VVBVV-#	
۷۷ ८- #	
VV U- #	
۷۷J-#	
VVOVV-#	WOOD SHEAK WALL

MHTN ARCHITECTS MHTN Architects, Inc. 280 South 400 West, Suite 250 Salt Lake City, Utah 84101 Telephone (801) 595-6700 Telefax (801) 595-6717 www.mhtn.com

515 East 100 South, Suite 1200 Salt Lake City, Utah 84102 801 486 3883 www.reaveley.com

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SLAB ON GRADE PLAN NOTES

1. ALL SLABS ON GRADE SHALL BE 4 INCHES THICK, UNLESS NOTED OTHERWISE. SEE TYPICAL CONCRETE SLAB ON GRADE PROFILE DETAIL B5/SB501 FOR SUBGRADE REQUIREMENTS.

2. SEE ARCHITECTURAL, CIVIL AND LANDSCAPE DRAWINGS FOR EXTERIOR CONCRETE WORK AT DOORS, SIDEWALKS, ETC.

3. SEE ARCHITECTURAL DRAWINGS AND FINISH SCHEDULE FOR SLAB DEPRESSIONS, SLOPES TO DRAINS AND SLAB AREAS TO RECEIVE FLOOR TILE.

4. SEE TYPICAL CONCRETE SLAB ON GRADE DETAILS FOR CONSTRUCTION JOINTS, CONTROL JOINTS AND ADDITIONAL SLAB REINFORCING C2/SB501.

5. SUBMIT SLAB ON GRADE CONTROL JOINT PLAN FOR REVIEW.

6. PROVIDE HOUSEKEEPING PADS AND CURBS PER DETAIL C4/SB501. VERIFY DIMENSIONS AND LOCATIONS OF CURBS AND PADS WITH MECHANICAL AND EQUIPMENT SUPPLIER.

FOOTING & FOUNDATION PLAN NOTES

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1. SEE ARCHITECTURAL, CIVIL AND LANDSCAPE DRAWINGS FOR EXTERIOR CONCRETE RETAINING AND / OR SITE WALLS NOT SHOWN ON THE STRUCTURAL DRAWINGS.

2. SEE TYPICAL STEP DETAIL AT CONTINUOUS FOOTING FOR REINFORCING REQUIREMENTS D1/SB501. SEE A3/SB501 FOR TYPICAL EXTERIOR TO INTERIOR FOOTING STEP DETAIL.

3. PROVIDE REINFORCEMENT AT WALL ENDS, INTERSECTIONS AND OPENINGS PER TYPICAL

DETAILS C2/SB601 AND D2/SB601.

4. DOWEL ALL CONCRETE WALLS TO FOOTING PER TYPICAL DETAIL D2/SB501.

5.ALL CONCRETE FOOTING ARE PLACED ON STRUCTURAL FILL EXTENDING TO SUITABLE SOILS OR ON SOIL IMPROVED WITH RAMMED AGGREGATE PIERS. REFER TO GEOTECH FOR DETAIL

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4. SEE TYPICAL CONCRETE SLAB ON GRADE DETAILS FOR CONSTRUCTION JOINTS, CONTROL JOINTS AND ADDITIONAL SLAB REINFORCING C2/SB501.

5. SUBMIT SLAB ON GRADE CONTROL JOINT PLAN FOR REVIEW.

6. PROVIDE HOUSEKEEPING PADS AND CURBS PER DETAIL C4/SB501. VERIFY DIMENSIONS AND LOCATIONS OF CURBS AND PADS WITH MECHANICAL AND EQUIPMENT SUPPLIER.

FOOTING & FOUNDATION PLAN NOTES

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3. PROVIDE REINFORCEMENT AT WALL ENDS, INTERSECTIONS AND OPENINGS PER TYPICAL

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5.ALL CONCRETE FOOTING ARE PLACED ON STRUCTURAL FILL EXTENDING TO SUITABLE SOILS OR ON SOIL IMPROVED WITH RAMMED AGGREGATE PIERS. REFER TO GEOTECH FOR DETAIL

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SLAB ON GRADE PLAN NOTES

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2. SEE ARCHITECTURAL, CIVIL AND LANDSCAPE DRAWINGS FOR EXTERIOR CONCRETE WORK AT DOORS, SIDEWALKS, ETC.

3. SEE ARCHITECTURAL DRAWINGS AND FINISH SCHEDULE FOR SLAB DEPRESSIONS, SLOPES TO DRAINS AND SLAB AREAS TO RECEIVE FLOOR TILE.

4. SEE TYPICAL CONCRETE SLAB ON GRADE DETAILS FOR CONSTRUCTION JOINTS, CONTROL JOINTS AND ADDITIONAL SLAB REINFORCING C2/SB501.

5. SUBMIT SLAB ON GRADE CONTROL JOINT PLAN FOR REVIEW.

6. PROVIDE HOUSEKEEPING PADS AND CURBS PER DETAIL C4/SB501. VERIFY DIMENSIONS AND LOCATIONS OF CURBS AND PADS WITH MECHANICAL AND EQUIPMENT SUPPLIER.

FOOTING & FOUNDATION PLAN NOTES

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1. SEE ARCHITECTURAL, CIVIL AND LANDSCAPE DRAWINGS FOR EXTERIOR CONCRETE RETAINING AND / OR SITE WALLS NOT SHOWN ON THE STRUCTURAL DRAWINGS.

2. SEE TYPICAL STEP DETAIL AT CONTINUOUS FOOTING FOR REINFORCING REQUIREMENTS D1/SB501. SEE A3/SB501 FOR TYPICAL EXTERIOR TO INTERIOR FOOTING STEP DETAIL.

3. PROVIDE REINFORCEMENT AT WALL ENDS, INTERSECTIONS AND OPENINGS PER TYPICAL

DETAILS C2/SB601 AND D2/SB601. 4. DOWEL ALL CONCRETE WALLS TO FOOTING PER

TYPICAL DETAIL D2/SB501.

5.ALL CONCRETE FOOTING ARE PLACED ON STRUCTURAL FILL EXTENDING TO SUITABLE SOILS OR ON SOIL IMPROVED WITH RAMMED AGGREGATE PIERS. REFER TO GEOTECH FOR DETAIL

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AND / OR SITE WALLS NOT SHOWN ON THE STRUCTURAL DRAWINGS. 2. SEE TYPICAL STEP DETAIL AT CONTINUOUS FOOTING FOR REINFORCING REQUIREMENTS D1/SB501. SEE A3/SB501 FOR TYPICAL EXTERIOR TO INTERIOR FOOTING STEP DETAIL. 3. PROVIDE REINFORCEMENT AT WALL ENDS, INTERSECTIONS AND OPENINGS PER TYPICAL DETAILS C2/SB601 AND D2/SB601. 4. DOWEL ALL CONCRETE WALLS TO FOOTING PER TYPICAL DETAIL D2/SB501.

FOOTING & FOUNDATION PLAN NOTES

1. SEE ARCHITECTURAL, CIVIL AND LANDSCAPE

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5.ALL CONCRETE FOOTING ARE PLACED ON STRUCTURAL FILL EXTENDING TO SUITABLE SOILS OR ON SOIL IMPROVED WITH RAMMED AGGREGATE PIERS. REFER TO GEOTECH FOR DETAIL

SLAB ON GRADE PLAN NOTES

1. ALL SLABS ON GRADE SHALL BE 4 INCHES THICK, UNLESS NOTED OTHERWISE. SEE TYPICAL CONCRETE SLAB ON GRADE PROFILE DETAIL B5/SB501 FOR SUBGRADE REQUIREMENTS.

2. SEE ARCHITECTURAL, CIVIL AND LANDSCAPE DRAWINGS FOR EXTERIOR CONCRETE WORK AT DOORS, SIDEWALKS, ETC.

3. SEE ARCHITECTURAL DRAWINGS AND FINISH SCHEDULE FOR SLAB DEPRESSIONS, SLOPES TO DRAINS AND SLAB AREAS TO RECEIVE FLOOR TILE.

4. SEE TYPICAL CONCRETE SLAB ON GRADE DETAILS FOR CONSTRUCTION JOINTS, CONTROL JOINTS AND ADDITIONAL SLAB REINFORCING C2/SB501.

5. SUBMIT SLAB ON GRADE CONTROL JOINT PLAN FOR REVIEW.

6. PROVIDE HOUSEKEEPING PADS AND CURBS PER DETAIL C4/SB501. VERIFY DIMENSIONS AND LOCATIONS OF CURBS AND PADS WITH MECHANICAL AND EQUIPMENT SUPPLIER.

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515 East 100 South, Suite 1200 Salt Lake City, Utah 84102 801 486 3883 www.reaveley.com

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SHEET NUMBER

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SB601 NO SCALE

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WALL REINF TREATED AS A SINGLE Detail Name

OPENINGS SHALL BE

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NOTE 3

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S" MAX

CONCRETE FOOTING SCHEDULE													
				CRO	CROSSWISE REINFORCING LENGTHWISE REINFORCING								
MARK	ARK WIDTH LENGTH THICK NO. SIZE LENGTH SPACE NO. SIZE LENGTH SPACE										REMARKS		
FTS2.0	2' - 0"	CONT.	1' - 0"		NONE REQ'D 3 #4 CONT. 9"								
FC2.0	2' - 0"	CONT.	1' - 0"		NONE REQ'D 3 #4 CONT. 9"								
FC3.0	3' - 0"	CONT.	1' - 0"		#5	2' - 6"	10"	4	#5	CONT.	10"		
FS3.0 3'-0" 3'-0" 1'-0" 4 #4 2'-6" 10.0" 4 #4 2'-6" 10.0"													

PLACE ALL FOOTING REINFORCING IN BOTTOM OF FOOTING WITH 3" CLEAR CONCRETE COVER UNLESS NOTED OTHERWISE. TOP REINFORCING, WHERE SPECIFIED, SHALL BE PLACED IN THE TOP OF THE FOOTING WITH 2" CLEAR CONCRETE COVER. SPOT FOOTINGS SHALL BE CENTERED UNDER COLUMNS AND CONTINUOUS FOOTINGS SHALL BE CENTERED UNDER WALLS, UNLESS NOTED OTHERWISE.

ALL FOOTINGS SHALL BE FORMED. FOOTINGS SHALL NOT BE EARTH FORMED OR OVERSIZED WITHOUT WRITTEN PERMISSION FROM THE STRUCTURAL ENGINEER.

NOTES: 1. SLEEVED OPENINGS SHALL NOT OCCUR IN JAMB COLUMNS OR COUPLING BEAMS.

2. SLEEVED OPENINGS SHALL NOT CUT HORIZ OR VERT WALL REINF. SEE TYPICAL OPENING THROUGH CONCRETE WALL DETAIL WHERE THIS OCCURS.

3. MULTIPLE SLEEVED **OPENINGS WITHIN 1.5D OF** EACH OTHER SHALL BE TREATED AS ONE OPENING.

4. WHERE SLEEVE EXCEEDS DIMENSIONAL REQUIREMENTS OF THIS DETAIL, USE THE TYPICAL OPENING THROUGH CONCRETE WALL DETAIL.

5. CONTRACTOR SHALL COORDINATE WITH ARCH THAT SLEEVE MATERIAL IS CONSISTANT WITH FIRE RESISTANCE REQUIREMENTS

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<u>90° HOOK</u> <u>180° HOOK</u>

1

END HOOK SCHEDULE

		END F		DULE				
BAR SIZE	STANDA	RD FINISHE WIDTH	D HOOK	SEISMIC FINISHED HOOK WIDTH				
	D	90° HOOK	180° HOOK	D	90° TIE HOOK	135° TIE HOOK		
#3	2.1/4"	6"	3"	1.1/2"	4"	4.1/4"		
#4	3"	8"	4"	2"	4.1/2"	4.1/2"		
#5	3.3/4"	10"	5"	2.1/2"	6"	5.1/2"		
#6	4.1/2"	12"	6"	4.1/2"		8"		
#7	5.1/4"	14"	7"	5.1/4"		9"		
#8	6"	16"	8"	6"		10.1/2"		
#9	9.1/2"	19"	11.3/4"					
#10	10.3/4"	22"	13.1/4"					
#11	12"	24"	14.3/4"					
#14	18.1/4"	31"	21.3/4"					
#18	24"	41"	28.1/2"					

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REINFORCEMENT END HOOK SCHEDULE / D1 `

SB602 NO SCALE

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TENSION HOOK DEVELOPMENT LENGTH (Ldh)											
	NORMAL WEIGHT CONCRETE, fc = PSI										
BAR SIZE	3,000	4,000	4,500	5,000	6,000						
#3	6"	6"	6"	6"	6"						
#4	6"	6"	6"	6"	6"						
#5	8"	8"	8"	8"	7"						
#6	11"	10"	10"	10"	10"						
#7	14"	13"	12"	12"	12"						
#8	16"	15"	15"	15"	15"						
#9	20"	18"	18"	18"	17"						
#10	23"	22"	21"	21"	21"						
#11	27"	26"	25"	25"	24"						
#14	#14 71" 66" 65" 64" 63"										
#18 109" 102" 100" 98" 96"											

1.SCHEDULE DOES NOT APPLY FOR THE FOLLOWING HOOKED BAR CASES:

a. BAR SPACING(s) LESS THAN 6db. b. BARS TERMINATING IN A COLUMN CORE WITH SIDE COVER PERPENDICULAR TO PLANE OF HOOK LESS THAN 2.5 IN.

c. BARS WITH SIDE COVER PERPENDICULAR TO PLANE OF HOOK LESS THAN 6db. d. BARS AT DISCONTINUOUS ENDS OF MEMBERS WITH SIDE,

TOP, AND BOTTOM COVER TO HOOK LESS THAN 2.5 IN.

2. VALUES IN SCHEDULE ARE APPLICABLE TO GRADE 60 **REINFORCING BARS** a. FOR GRADE 80 BARS, MULTIPLY VALUES BY 1.33.

b. FOR GRADE 100 BARS, MULTIPLY VALUES BY 1.67.

3. ADJUST VALUES AS NOTED FOR THE FOLLOWING CASES a. FOR BARS COATED IN EPOXY AND/OR ZINC MULTIPLY VALUES

BY 1.2. b. FOR LIGHTWEIGHT CONCRETE MULTIPLY VALUES BY 1.33.

4. CASES NOT APPLICABLE TO THIS SCHEDULE SHALL BE IN ACCORDANCE WITH SECTION 25.4.3 OF ACI 318-19

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TENSION HOOK DEVELOPMENT SCHEDULE SB602 NO SCALE

TENSION HEADED BAR DEVELOPMENT LENGTH (Ldt)											
	NORMAL WEIGHT CONCRETE, fc = PSI										
BAR SIZE	3,000	4,000	4,500	5,000	6,000						
#3	6"	6"	6"	6"	6"						
#4	6"	6"	6"	6"	6"						
#5	6"	6"	6"	6"	6"						
#6	8"	8"	7"	7"	7"						
#7	10"	9"	9"	9"	9"						
#8	12"	11"	11"	11"	11"						
#9	14"	14"	13"	13"	13"						
#10	17"	16"	16"	16"	15"						
#11	20"	19"	18"	18"	18"						
NOTES:											

1. SCHEDULE DOES NOT APPLY FOR THE FOLLOWING HEADED BAR CASES:

a. BAR SPACING(s) LESS THAN 6db. b. BARS TERMINATING IN A COLUMN CORE WITH SIDE COVER PERPENDICULAR TO PLANE OF HEADED BAR LESS THAN 2.5 IN. 2. BARS WITH SIDE COVER PERPENDICULAR TO PLANE OF HEADED BAR LESS THAN 6db.

2. VALUES IN SCHEDULE ARE APPLICABLE TO GRADE 60 REINFORCING BARS. a. FOR GRADE 80 BARS, MULTIPLY VALUES BY 1.33.

b. FOR GRADE 100 BARS, MULTIPLY BY VALUES 1.67 3. FOR EPOXY AND/OR ZINC COATED BARS MULTIPLY VALUES BY 1.2

4. CASES NOT APPLICABLE TO THIS SCHEDULE SHALL BE IN ACCORDANCE WITH SECTION 25.4.4. OF ACI 318-19

5. TENSION HEADED REINFORCING BARS SHALL CONFORM TO SECTION 25.4.4.1 OF ACI 319-19.

C3 TENSION HEADED BAR DEVELOPMENT SCHEDULE \SB602/ NO SCALE

B4 SB602/ NO SCALE

	A	DHESIVE ANCHORS IN	CONCRETE SCHEDULE					
_^	REINFORG	CING BAR	THREAD	ED ROD				
V	DOWEL SIZE	EMBEDMENT LENGTH (SEE NOTE #2)	SIZE	EMBEDMENT LENGTH (SEE NOTE #2)				
	#3	4"	3/8"Ø	4.1/2"				
	#4	6"	1/2"Ø	6"				
	#5	9"	5/8"Ø	7.1/2"				
	#6	10"	3/4"Ø	9"				
	#7	12.1/2"	7/8"Ø	10.1/2"				
	#8	13"	1"Ø	12"				
	#9	14"	1.1/4"Ø	15"				
	#10	18"						
-NEW THREADED ROD	#11	19"						
>	NOTES:							
—NEW REBAR DOWEL —ANCHOR REBAR OR THREADED	1. THIS SCHEDULE SHALL AND AT OTHER LOCATIO	BE USED ONLY WHERE SPE ONS WITH APPROVAL OF THE	CIFICALLY REFERENCED ON E STRUCTURAL ENGINEER.	THE DRAWINGS				
ROD IN ADHESIVE FILLED HOLE. USE APPROVED ADHESIVE AND FOLLOW ALL MANUFACTURERS	2. EMBEDMENT LENGTHS LENGTHS IN THIS SCHE	SPECIFIED ON PLANS OR DE DULE.	TAILS TAKE PRECEDENCE C	OVER EMBEDMENT				
RECOMMENDATIONS PER THE CODE EVALUATION REPORT (SEE GENERAL STRUCTURAL NOTES)	 WHERE THE THICKNESS OF THE EXISTING CONCRETE MEMBER IS NOT SUFFICIENT TO ACHIEVE SCHEDULED EMBEDMENT AND SPECIFIED CLEAR COVER FOR THE ANCHOR, CONTACT THE STRUCTURAL ENGINEER. 							
	4. SEE GENERAL STRUCTURAL NOTES FOR LIST OF APPROVED ADHESIVES AND OTHER REQUIREMENTS FOR ADHESIVE ANCHORING.							

3

CONORETE REINEOROINO DAR DEVELORMENT AND LAR ODUOE LENOTU COLERUILE

			(JONCI	REIE	REINF	ORCII	NG BA	R DE	/ELOP	MENI	AND	LAP S	PLICE	LENG	SIHS	HEDU	JLE				
BAR		f'c = 30	00 PSI			f'c = 40	00 PSI			f'c = 45	00 PSI			f'c = 50	00 PSI			f'c = 60	00 PSI		f'c =	ALL
SIZE	Ld	Lt	Lsb	Lsbt	Ld	Lt	Lsb	Lsbt	Ld	Lt	Lsb	Lsbt	Ld	Lt	Lsb	Lsbt	Ld	Lt	Lsb	Lsbt	Ldc	Lsc
#3	17"	22"	22"	28"	15"	19"	19"	25"	14"	18"	18"	23"	13"	17"	17"	22"	12"	16"	16"	20"	8"	12"
#4	22"	29"	29"	38"	19"	25"	25"	33"	18"	24"	24"	31"	17"	23"	23"	29"	16"	21"	21"	27"	10"	15"
#5	28"	36"	36"	47"	24"	31"	31"	41"	23"	30"	30"	38"	22"	28"	28"	36"	20"	26"	26"	33"	12"	19"
#6	33"	43"	43"	56"	29"	37"	37"	49"	27"	35"	35"	46"	26"	34"	34"	44"	24"	31"	31"	40"	15"	23"
#7	48"	63"	63"	81"	42"	54"	54"	71"	40"	51"	51"	67"	38"	49"	49"	63"	34"	45"	45"	58"	17"	27"
#8	55"	72"	72"	93"	48"	62"	62"	81"	45"	59"	59"	76"	43"	56"	56"	72"	39"	51"	51"	66"	19"	30"
#9	62"	81"	81"	105"	54"	70"	70"	91"	51"	66"	66"	86"	48"	63"	63"	81"	44"	57"	57"	74"	22"	34"
#10	70"	91"	91"	118"	61"	79"	79"	102"	57"	74"	74"	96"	54"	71"	71"	92"	50"	64"	64"	84"	24"	39"
#11	78"	101"	101"	131"	67"	87"	87"	114"	64"	82"	82"	107"	60"	78"	78"	102"	55"	71"	71"	93"	27"	43"
#14	93"	121"	NA	NA	81"	105"	NA	NA	76"	99"	NA	NA	72"	94"	NA	NA	66"	86"	NA	NA	33"	NA
#18	124"	161"	NA	NA	108"	140"	NA	NA	101"	132"	NA	NA	96"	125"	NA	NA	88"	114"	NA	NA	43"	NA
OTES:			1			·																

1. DEFINITIONS:

MULTIPLY VALUES BY 1.2.

Ld: TENSION DEVELOPMENT LENGTH FOR REINFORCEMENT SATISFYING THE FOLLOWING CONDITIONS:

SLABS AND WALLS: CLEAR SPACING > 2db AND CONCRETE CLEAR COVER > db BEAMS AND COLUMNS: CLEAR COVER SPACING > db AND CONCRETE CLEAR COVER > db

Lt: DEVELOPMENT LENGTH FOR TOP BARS IN TENSION

Lsb: TENSION LAP SPLICE LENGTH FOR OTHER THAN TOP BARS (CLASS B) Lsbt: TENSION LAP SPLICE LENGTH OF TOP BARS.

Ldc: DEVELOPMENT LENGTH FOR BARS IN COMPRESSION

Lsc: TIED COLUMN LAP SPLICE IN COMPRESSION db: NOMINAL BAR DIAMETER (INCHES)

TOP BARS: HORIZONTAL BEAM REINFORCEMENT WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW

2. MULTIPLY VALUES IN SCHEDULE BY 1.5 IF CLEAR SPACING OR CONCRETE COVER DO NOT MEET REQUIREMENTS FOR Ld IN NOTE 1.

3. MULTIPLY VALUES IN SCHEDULE BY 1.3 FOR USE IN LIGHTWEIGHT AGGREGATE CONCRETE.

4. FOR EPOXY COATED BAR: MULTIPLY VALUES IN SCHEDULE BY 1.5 FOR BARS WITH CLEAR COVER < 3db OR CLEAR SPACING < 6db. OTHERWISE

5. a. FOR BUNDLED BARS OF THREE OR LESS MULTIPLY LENGTHS BY 1.2.

b. FOR BUNDLED BARS OF FOUR OR MORE MULTIPLY LENGTHS BY 1.33. c. INDIVIDUAL BAR SPLICES WITHIN A BUNDLE SHALL NOT OVERLAP. ENTIRE BUNDLES SHALL NOT BE LAP SPLICED.

6. SCHEDULE LENGTHS ARE FOR fy=60ksi REINFORCING, MULTIPLY LENGTHS BY 1.53 FOR fy=80ksi, 2.17 FOR fy=100ksi REINFORCING.

7. LAP SPLICES ARE NOT PERMITTED FOR #14 & #18 BARS. USE BAR COUPLERS PER G.S.N.

8. MINIMUM CLEAR SPACING BETWEEN THE CONTACT LAP SPLICES SHOWN IN THIS SCHEDULE AND ADJACENT SPLICES OR BARS SHALL BE IN ACCORDANCE WITH THE DETAILING PROVISIONS OF GENERAL STRUCTURAL NOTES.

		EXPANSION AN	ICHORS IN CONCRE	ETE SCHEDULE	
	ANCHOR SIZE	MINIMUM EDGE DISTANCE (Cac)	EMBEDMENT LENGTH (H nom)	MINIMUM CONCRETE THICKNESS (H min)	MINIMUM ANCHOR SPACING (S min)
	3/8"Ø	6.1/2"	2.7/8"	4.1/2"	3.3/4"
	1/2"Ø	10"	3.7/8"	6"	5"
	5/8"Ø	10"	5.1/8"	8"	6"
	3/4"Ø	16"	5.3/4"	10"	7"
EXPANSION ANCHOR	 NOTES: THIS SCHEDULE SHANCHORS AT OTHE EDGE DISTANCE, COR DETAILS TAKE F ANCHORS LOCATE REQUIRED MINIMULTO INSTALLATION. SEE GENERAL STRUSING EXPANSION 	IALL BE USED ONLY WI R LOCATIONS MUST B ac, AND EMBEDMENT L PRECEDENCE OVER V/ D WHERE THE THICKN M CONCRETE THICKNE	HERE SPECIFICALLY RE E APPROVED BY THE E _ENGTHS, H nom, AND A ALUES IN THIS SCHEDU ESS OF THE EXISTING (ESS MUST BE APPROVE LIST OF APPROVED AN	EFERENCED ON THE DE NGINEER PRIOR TO INS NCHOR SPACING SPEC LE. CONCRETE MEMBER DE D BY THE STRUCTURA	RAWINGS. STALLATION. CIFIED ON PLANS OES NOT MEET THE L ENGINEER PRIOR

EXPANSION ANCHORS IN CONCRETE SCHEDULE

4

		SCREW ANC	HORS IN CONCRET	E SCHEDULE	
	ANCHOR SIZE	MINIMUM EDGE DISTANCE (Cac)	EMBEDMENT LENGTH (H nom)	MINIMUM CONCRETE THICKNESS (H min)	MINIMUM ANCHOR SPACING (S min)
	3/8"Ø	3.3/4"	3.1/4"	5"	3"
	1/2"Ø	4.1/2"	4"	6.1/4"	3.1/2"
	5/8"Ø	6.3/8"	5.1/2"	8.1/2"	3.3/4"
	3/4"Ø	7.5/16"	6.1/4"	10"	4.1/2"
	NOTES:				
	1. THIS SCHEDULE SH OTHER LOCATIONS	HALL BE USED ONLY W S WITH APPROVAL OF	/HERE SPECIFICALLY RI THE STRUCTURAL ENG	EFERENCED ON THE D NEER.	RAWINGS AND AT
	2. EDGE DISTANCE, C DETAILS TAKE PRE	ac, AND EMBEDMENT CEDENCE OVER VALU	LENGTHS, H nom, AND A JES IN THIS SCHEDULE.	ANCHOR SPACING SPE	CIFIED ON PLANS OR
Sarin	3. ANCHORS LOCATE REQUIRED MINIMUI TO INSTALLATION	D WHERE THE THICKN M CONCRETE THICKN	IESS OF THE EXISTING (ESS MUST BE APPROVE	CONCRETE MEMBER D D BY THE STRUCTURA	OES NOT MEET THE L ENGINEER PRIOR
	4. SPECIAL INSPECTION EVALUATION REPO STRUCTURAL NOTE	ON IS REQUIRED DURI RT FOR THE ANCHOR ES.	NG INSTALLATION OF A AND THE QUALITY ASS	LL SCREW ANCHORS F URANCE SECTION OF 1	ER THE CODE THE GENERAL
EXISTING CONCRETE	5. SEE GENERAL STR USING SCREW AND	UCTURAL NOTES FOR CHORS.	LIST OF APPROVED AN	CHORS AND OTHER RE	EQUIREMENTS FOR
	6. SCREW ANCHORS	SHALL ONLY BE USED	IN INTERIOR DRY LOCA	TIONS	

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MHTN ARCHITECTS MHTN Architects, Inc. 280 South 400 West, Suite 250 Salt Lake City, Utah 84101 Telephone (801) 595-6700 Telefax (801) 595-6717 www.mhtn.com

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 ENING 611	CONTROL JOIN A1/SB611 OCCURS 	r Per
SCHEDU LINTEL R 1-#5 AT 8 2-#5 AT 1	LED MASONRY EINFORCING	
MAXIMUM SPACI AND HORIZO HEIGHT-WIDTH	NG OF VERTICAL ONTAL BARS SPACING	FLOOR

8" O.C.

16" O.C.

24" O.C.

32" O.C.

40" O.C.

48" O.C.

(H or L)<4'-0" WALL REINFORCING LEGEND 4'-0"≤(H or L)<6'-0" 6'-0"≤(H or L)<8'-0" SCHEDULED WALL REINF 8'-0"≤(H or L)<10'-0" 10'-0"≤(H or L)<12'-0" SCHEDULED OPENING REINF 12'-0"≤(H or L)

		MA	SONRY WALL SO	CHEDULE							
			F	REINFORCING							
MARK	THICK	MATERIAL	VERTICAL	HORIZONTAL	TYPE	REMARK					
MW-1	8"	CMU	#5 @ 32" O.C.	#5 @ 48" O.C.	TYPE 1						
MW-2	8"	CMU	#5 @ 16" O.C.	#5 @ 16" O.C.	TYPE 1						
MW-3	6"	CMU	#5 @ 32" O.C.	#5 @ 48" O.C.	TYPE 1						
NOTES											
1. SEE	1. SEE PLANS, DETAILS AND GENERAL STRUCTURAL NOTES FOR ADDITIONAL REINFORCING										
REC	QUIREMENTS.										
2. GRC	OUT SOLID ALL	CELLS BELOW	GRADE, CELLS CON	TAINING EMBEDS (HSA	A'S, DBA'S, AN	ICHOR BOLTS					

ETC.), AND CELLS CONTAINING REINFORCING. CONSOLIDATE GROUT AS PER THE GENERAL STRUCTURAL NOTES. HORIZONTAL WALL REINFORCING SHALL CONTINUE THROUGH MASONRY LINTELS. WHERE BOTH

HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE, THE LARGER BARS ARE TO REPLACE THE SMALLER BARS.

MASONRY COLUMN SCHEDULE DIMENSIONS MARK WIDTH DEPTH VERTICAL MC-1 2-#4 8" MC-2 12-#5 4' - 0" 8" MC-3 1' - 4" 4-#5 8" MC-4 2' - 0" 6-#5 8" NOTES HORIZONTAL WALL REINFORCING BARS SHALL BE CONTINUOUS THROUGH MASONRY COLUMNS. AT WALL ENDS OR OPENINGS TERMINATE HORIZONTAL WALL REINFORCING WITH A 90° OR 180° HOOK. FOR TYPE 2 & 2A COLUMNS, HORIZONTAL WALL REINFORCING SHALL BE LOCATED TO THE INSIDE OF VERTICAL COLUMN BARS.. UNLESS NOTED OTHERWISE, VERTICAL COLUMN REINFORCING AND TIES SHALL EXTEND THE FULL STORY HEIGHT OF THE WALL. MASONRY COLUMN VERTICAL BARS OR DOWELS IN CONCRETE FOUNDATION WALLS SHALL HAVE TIES TO MATCH MASONRY COLUMN TIES. VERTICAL REINFORCING IN TYPE 1 & 2 COLUMNS SHALL BE DISTRIBUTED EQUALLY IN EACH CELL. PLACE VERTICAL COLUMN BARS IN EACH END CELL FOR TYPE 1A & 2A COLUMNS. REMAINING

TYPE 1A AND TWO BARS PER CELL FOR TYPE 2A. ALL CELLS IN COLUMNS SHALL BE GROUTED SOLID.

		MASON	RY REIN	IFORCIN	IG BAR	LAP SPL	ICE SCI	HEDULE		
			f'm =	= 2000 psi				f'm =	= 2500 psi	
BAR	6" CMU	8" (CMU	10" (CMU	12" (CMU	6" BRICK	8" BRICK	
SIZE	CLASS	CL/	ASS	CLA	ASS	CLA	ASS	CLASS	CLA	ASS
	А	A	В	A	В	A	В	А	А	В
#3	12"	12"	12"	12"	12"	12"	12"	12"	12"	12"
#4	18"	13"	21"	12"	20"	12"	20"	16"	12"	20"
#5	28"	20"	35"	16"	32"	13"	32"	25"	18"	33"
#6	**	38"	54"	29"	54"	24"	54"	**	34"	54"
#7	-	52"	**	40"	**	33"	63"	-	47"	-
#8	-	**	-	61"	**	50"	**	-	**	-
#9	-	-	-	79"	-	64"	-	-	-	I

1. CLASS A SPLICES MAY BE USED WHEN ONLY ONE BAR IS CONTINUOUS IN THE MASONRY CELL OR COURSE

2. CLASS B SPLICES SHALL BE USED WHEN TWO BARS ARE CONTINUOUS IN THE MASONRY CELL OR COURSE

. ** INDICATES THAT A LAP SPLICE IS NOT ALLOWED AND MECHANICAL BAR COUPLERS ARE REQUIRED FOR THE BAR SPLICES. SPLICES SHALL BE OFFSET 2'-0" TO AVOID CONGESTION.

4. WHERE VERTICAL BARS HAVE A REQUIRED LAP SPLICE GREATER THAN THE HEIGHT OF THE GROUT POUR. THE BAR SPLICE SHALL BE MADE WITH A MECHANICAL BAR COUPLER. WHERE THE HEIGHT OF THE GROUT POUR EXCEEDS 60 INCHES, HIGH LIFT GROUTING PROCEDURES SHALL BE FOLLOWED.

5. WHERE MECHANICAL BAR COUPLERS ARE USED, THE CONNECTOR SHALL DEVELOP 125% OF THE SPECIFIED YIELD STRENGTH OF THE BAR IN TENSION AND COMPRESSION.

		MA	SONRY LINTEL SO	CHEDULE		ML-1
MARK			REINFO	MAXIMUM	REMARKS	
ML-1	8"	8"	1- #5 CONT.		3'-4"	
ML-2	16"	8"	1- #6 CONT. TOP & BOTTOM		5'-4"	
ML-3	32"	8"	1- #7 CONT. TOP & BOTTOM	#3 @ 8" O.C.		
ML-4	32"	8"	1- #8 CONT. EA. CELL	#3 @ 8" O.C.	17'-0"	HORIZ BARS TO HAVE HOOKED ENDS
ML-5	24"	8"	1- #6 CONT. TOP & BOTTOM		15'-0"	

NOTES:

. MASONRY LINTELS ML-1 THROUGH ML-4 SHALL BE USED OVER OPENINGS IN MASONRY WALLS WHEN A SPECIFIC MASONRY LINTEL IS NOT OTHERWISE SPECIFIED. THE MASONRY LINTEL TO BE USED SHALL BE DETERMINED BY THE MAXIMUM SPAN AS SPECIFIED IN THIS SCHEDULE. WHEN A SPECIFIC MASONRY LINTEL IS CALLED OUT ON THE PLAN, THE MAXIMUM SPAN AS NOTED IN THIS SCHEDULE SHALL NOT APPLY

2. MASONRY LINTELS ML-1 THROUGH ML-4 SHALL NOT BE LOCATED BELOW ANY FLOOR, OR ROOF BEAM, OR GIRDER, OR ANY OTHER CONCENTRATED LOAD UNLESS SHOWN SPECIFICALLY ON THE PLAN SHEET. JOISTS SHALL NOT BEAR ON ANY LINTEL LESS THAN 16" DEEP.

3. FOR MASONRY LINTELS NOT SHOWN ON THE DRAWINGS THAT CARRY ANY FLOOR, OR ROOF BEAM, OR GIRDER, OR ANY OTHER CONCENTRATED LOAD, OR THAT SPAN GREATER THAN 10'-0" CONSULT THE STRUCTURAL ENGINEER.

4. EXTEND ALL HORIZONTAL REINFORCING 48 BAR DIAMETERS BEYOND THE EDGE OF THE OPENING. IF HORIZONTAL REINFORCING CANNOT BE EXTENDED 48 BAR DIAMETERS BEYOND THE EDGE OF THE OPENING, PROVIDE 90 DEGREE STANDARD HOOK.

5. GROUT MASONRY LINTELS MONOLITHICALLY WITH THE SUPPORT WALL OR COLUMN AT EACH END.

6. SPLICE TOP BARS AT MIDSPAN OF LINTEL ONLY.

7. SPLICE BOTTOM BARS OVER SUPPORTS ONLY.

8. FOR WALL ABOVE LINTEL, DOWEL VERTICAL REINFORCING INTO FULL DEPTH OF THE LINTEL OR 48 BAR DIAMETERS, WHICHEVER IS LESS.

9. HORIZONTAL WALL REINFORCING SHALL CONTINUE THROUGH MASONRY LINTELS. WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING WOULD OCCUR IN THE SAME COURSE, THE LARGER BARS ARE TO REPLACE THE SMALLER BARS.

F	REINFORCING		
	TIES	TYPE	REMARKS
	NONE	TYPE 2	

REINFORCING SHALL BE SPACED EQUALLY THROUGHOUT THE COLUMN WITH ONE BAR PER CELL FOR

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DECK PROFILE AND DECK ATTACHMENT

DECK PER DETAIL D1/SF511. FOR ROUND OPENINGS LESS THAN 12 INCHES IN DIAMETER SEE DETAIL D2/SF511.

3. VERIFY SIZE WEIGHT, LOCATION AND CONFIGURATION OF ALL ROOF TOP EQUIPMENT WITH ARCHITECT AND MECHANICAL ENGINEER. PROVIDE STEEL FRAMES FOR SUPPORT OF ROOF

MECHANICAL UNITS.

5. SEE DETAIL D4/SF511 FOR TYPICAL ROOF DRAIN OPENING

2. SEE A2/SB611 FOR TYPICAL REINFORCING

3. SEE B1/SB611 FOR REQUIRED ADDITIONAL DUCTILITY REINFORCING IN LOAD BEARING

WALL LOCATIONS.

NON-BEARING MASONRY WALL BRACING BELOW UNDERSIDE OF STEEL DECK. SEE C4/SF512 FOR TYPICAL BRACING DETAIL BELOW

6. ALL EXTERIOR STRUCTURAL STEEL SHALL

1. OPEN WEB STEEL JOISTS AND JOIST GIRDERS SHALL BE DESIGNED BY THE MANUFACTURER TO SUPPORT THE MECHANICAL AND LATERAL LOADS SHOWN ON THE ROOF FRAMING PLANS IN ADDITION SHOWN ARE ASD UNO

DOWNWARD AND - INDICATES UPWARD LOADS. LOADS SHOWN ARE UNFACTORED, UNO.

3. T/C X.XXK INDICATES ADDITIONAL TOP CHORD AXIAL FORCE ON STEEL JOIST OR GIRDER. THIS AND INCLUDES APPLICABLE OVERSTRENGTH FACTORS PER THE GOVERNING BUILDING CODE. BE DESIGNED AS COLLECTOR ELEMENTS.

4. ALL LOADS SUPPORTED BY OPEN WEB STEEL GIRDER SHALL BE REINFORCED PER DETAIL D1/SF512.

5. HORIZONTAL CROSS BRIDGING SHALL BE SIZED AND SUPPLIED BY THE JOIST MANUFACTURER. CONNECT TO WALLS AS INDICATED ON DETAILS

OR MASONRY WALLS SHALL BE DESIGNED FOR A SEISMIC AXIAL LOAD TO BE TRANSFERRED THROUGH THE JOIST BEARING SEAT. MINIMUM UNFACTORED LOAD SHALL BE 4.0 K AT K JOISTS, 6.0 K AT LH & DLH JOISTS, UNO.

SEATS.

8. OPEN WEB STEEL JOISTS AT ROOF AREAS SHALL BE DESIGNED FOR THE FOLLOWING WIND ASD NET UPLIFT LOADS: 25PSF WITHIN 18FT OF ROOF EDGES, 18PSF AT ALL OTHER AREAS.

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NON-COMPOSITE FRAMING PLAN NOTES

1. SEE STEEL DECK SCHEDULE ON SHEET SF602 FOR DECK PROFILE AND DECK ATTACHMENT REQUIREMENTS.

2. PROVIDE FRAMING AT OPENINGS THROUGH STEEL DECK PER DETAIL D1/SF511. FOR ROUND OPENINGS LESS THAN 12 INCHES IN DIAMETER SEE DETAIL D2/SF511.

3. VERIFY SIZE WEIGHT, LOCATION AND CONFIGURATION OF ALL ROOF TOP EQUIPMENT WITH ARCHITECT AND MECHANICAL ENGINEER. PROVIDE STEEL FRAMES FOR SUPPORT OF ROOF TOP EQUIPMENT PER DETAIL D3/SF511. COORDINATE OPENINGS WITH MECHANICAL & ELECTRICAL.

4. SEE DETAIL C3/SF512 FOR SUPPORT OF HANGING MECHANICAL UNITS.

5. SEE DETAIL D4/SF511 FOR TYPICAL ROOF DRAIN OPENING

MASONRY WALL NOTES

1. PROVIDE ADDITIONAL HORIZONTAL AND VERTICAL REINFORCING AT WALL CORNERS, EDGES OF OPENINGS, WALL ENDS, AND WALL INTERSECTIONS PER D1/SB611

2. SEE A2/SB611 FOR TYPICAL REINFORCING AROUND MISCELLANEOUS OR RECESSED MASONRY WALL OPENINGS.

3. SEE B1/SB611 FOR REQUIRED ADDITIONAL DUCTILITY REINFORCING IN LOAD BEARING MASONRY WALLS.

4. SEE ARCHITECTURAL FOR TOP OF NON-BEARING WALL LOCATIONS.

5. SEE DETAILS C2/SF512, A2/SF512 FOR TYPICAL NON-BEARING MASONRY WALL BRACING BELOW UNDERSIDE OF STEEL DECK. SEE C4/SF512 FOR TYPICAL BRACING DETAIL BELOW UNDERSIDE OF STEEL BEAM.

6. ALL EXTERIOR STRUCTURAL STEEL SHALL GALVANZIED (G60)

FLOOR FRAMING PLAN NOTES

1. SEE STEEL DECK SCHEDULE ON SHEET SF602 FOR DECK PROFILE, DECK ATTACHMENT, CONCRETE FILL AND SLAB REINFORCEMENT REQUIREMENTS.

2. PROVIDE STEEL DECK FILLER PER DETAIL WHERE REQUIRED FOR HEADED STUD PLACEMENT.

3. DO NOT PLACE CONDUIT IN CONCRETE OVER STEEL DECK.

4. SEE ARCHITECTURAL DRAWINGS FOR SLAB AREAS TO RECEIVE FLOOR TILE.

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NON-COMPOSITE FRAMING PLAN NOTES

1. SEE STEEL DECK SCHEDULE ON SHEET SF602 FOR DECK PROFILE AND DECK ATTACHMENT REQUIREMENTS.

2. PROVIDE FRAMING AT OPENINGS THROUGH STEEL DECK PER DETAIL D1/SF511. FOR ROUND OPENINGS LESS THAN 12 INCHES IN DIAMETER SEE DETAIL D2/SF511.

3. VERIFY SIZE WEIGHT, LOCATION AND CONFIGURATION OF ALL ROOF TOP EQUIPMENT WITH ARCHITECT AND MECHANICAL ENGINEER. PROVIDE STEEL FRAMES FOR SUPPORT OF ROOF TOP EQUIPMENT PER DETAIL D3/SF511. COORDINATE OPENINGS WITH MECHANICAL & ELECTRICAL.

4. SEE DETAIL C3/SF512 FOR SUPPORT OF HANGING MECHANICAL UNITS.

5. SEE DETAIL D4/SF511 FOR TYPICAL ROOF DRAIN OPENING

MASONRY WALL NOTES

1. PROVIDE ADDITIONAL HORIZONTAL AND VERTICAL REINFORCING AT WALL CORNERS, EDGES OF OPENINGS, WALL ENDS, AND WALL INTERSECTIONS PER D1/SB611

2. SEE A2/SB611 FOR TYPICAL REINFORCING AROUND MISCELLANEOUS OR RECESSED MASONRY WALL OPENINGS.

3. SEE B1/SB611 FOR REQUIRED ADDITIONAL DUCTILITY REINFORCING IN LOAD BEARING MASONRY WALLS.

4. SEE ARCHITECTURAL FOR TOP OF NON-BEARING WALL LOCATIONS.

5. SEE DETAILS C2/SF512, A2/SF512 FOR TYPICAL NON-BEARING MASONRY WALL BRACING BELOW UNDERSIDE OF STEEL DECK. SEE C4/SF512 FOR TYPICAL BRACING DETAIL BELOW UNDERSIDE OF STEEL BEAM.

6. ALL EXTERIOR STRUCTURAL STEEL SHALL GALVANZIED (G60)

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ROOF	F FRAMING PLAN - DUGOUT 2 (BUILDING D)/	0	1
SCALE:	1/8" = 1'-0"	$\overline{\ }$	\square	

NON-COMPOSITE FRAMING PLAN NOTES

1. SEE STEEL DECK SCHEDULE ON SHEET SF602 FOR DECK PROFILE AND DECK ATTACHMENT REQUIREMENTS.

2. PROVIDE FRAMING AT OPENINGS THROUGH STEEL DECK PER DETAIL D1/SF511. FOR ROUND OPENINGS LESS THAN 12 INCHES IN DIAMETER SEE DETAIL D2/SF511.

3. VERIFY SIZE WEIGHT, LOCATION AND CONFIGURATION OF ALL ROOF TOP EQUIPMENT WITH ARCHITECT AND MECHANICAL ENGINEER. PROVIDE STEEL FRAMES FOR SUPPORT OF ROOF TOP EQUIPMENT PER DETAIL D3/SF511. COORDINATE OPENINGS WITH MECHANICAL & ELECTRICAL.

4. SEE DETAIL C3/SF512 FOR SUPPORT OF HANGING MECHANICAL UNITS.

5. SEE DETAIL D4/SF511 FOR TYPICAL ROOF DRAIN OPENING

MASONRY WALL NOTES

1. PROVIDE ADDITIONAL HORIZONTAL AND VERTICAL REINFORCING AT WALL CORNERS, EDGES OF OPENINGS, WALL ENDS, AND WALL INTERSECTIONS PER D1/SB611

2. SEE A2/SB611 FOR TYPICAL REINFORCING AROUND MISCELLANEOUS OR RECESSED MASONRY WALL OPENINGS.

3. SEE B1/SB611 FOR REQUIRED ADDITIONAL DUCTILITY REINFORCING IN LOAD BEARING MASONRY WALLS.

4. SEE ARCHITECTURAL FOR TOP OF NON-BEARING WALL LOCATIONS.

5. SEE DETAILS C2/SF512, A2/SF512 FOR TYPICAL NON-BEARING MASONRY WALL BRACING BELOW UNDERSIDE OF STEEL DECK. SEE C4/SF512 FOR TYPICAL BRACING DETAIL BELOW UNDERSIDE OF STEEL BEAM.

6. ALL EXTERIOR STRUCTURAL STEEL SHALL GALVANZIED (G60)

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NON-COMPOSITE FRAMING PLAN NOTES

1. SEE STEEL DECK SCHEDULE ON SHEET SF602 FOR DECK PROFILE AND DECK ATTACHMENT REQUIREMENTS.

2. PROVIDE FRAMING AT OPENINGS THROUGH STEEL DECK PER DETAIL D1/SF511. FOR ROUND OPENINGS LESS THAN 12 INCHES IN DIAMETER SEE DETAIL D2/SF511.

3. VERIFY SIZE WEIGHT, LOCATION AND CONFIGURATION OF ALL ROOF TOP EQUIPMENT WITH ARCHITECT AND MECHANICAL ENGINEER. PROVIDE STEEL FRAMES FOR SUPPORT OF ROOF TOP EQUIPMENT PER DETAIL D3/SF511. COORDINATE

4. SEE DETAIL C3/SF512 FOR SUPPORT OF HANGING MECHANICAL UNITS.

OPENINGS WITH MECHANICAL & ELECTRICAL.

5. SEE DETAIL D4/SF511 FOR TYPICAL ROOF DRAIN OPENING

MASONRY WALL NOTES

1. PROVIDE ADDITIONAL HORIZONTAL AND VERTICAL REINFORCING AT WALL CORNERS, EDGES OF OPENINGS, WALL ENDS, AND WALL INTERSECTIONS PER D1/SB611

2. SEE A2/SB611 FOR TYPICAL REINFORCING AROUND MISCELLANEOUS OR RECESSED MASONRY WALL OPENINGS.

3. SEE B1/SB611 FOR REQUIRED ADDITIONAL DUCTILITY REINFORCING IN LOAD BEARING MASONRY WALLS.

4. SEE ARCHITECTURAL FOR TOP OF NON-BEARING WALL LOCATIONS.

5. SEE DETAILS C2/SF512, A2/SF512 FOR TYPICAL NON-BEARING MASONRY WALL BRACING BELOW UNDERSIDE OF STEEL DECK. SEE C4/SF512 FOR TYPICAL BRACING DETAIL BELOW UNDERSIDE OF STEEL BEAM.

6. ALL EXTERIOR STRUCTURAL STEEL SHALL GALVANZIED (G60)

OPEN WEB JOIST FRAMING PLAN NOTES

1. OPEN WEB STEEL JOISTS AND JOIST GIRDERS SHALL BE DESIGNED BY THE MANUFACTURER TO SUPPORT THE MECHANICAL AND LATERAL LOADS SHOWN ON THE ROOF FRAMING PLANS IN ADDITION TO THE UNIFORM AND POINT LOADS SHOWN. LOADS SHOWN ARE ASD UNO

2. ±#.##k - INDICATES POINT LOAD ON STEEL JOIST IN ADDITION TO UNIFORM LOADING SHOWN. + INDICATES DOWNWARD AND - INDICATES UPWARD LOADS. LOADS SHOWN ARE UNFACTORED, UNO.

3. T/C X.XXK INDICATES ADDITIONAL TOP CHORD AXIAL FORCE ON STEEL JOIST OR GIRDER. THIS FORCE IS A FACTORED SEISMIC LOAD THAT SHALL BE CONSIDERED IN BOTH TENSION AND COMPRESSION AND INCLUDES APPLICABLE OVERSTRENGTH FACTORS PER THE GOVERNING BUILDING CODE. STEEL JOISTS AND GIRDERS WITH T/C FORCE SHALL BE DESIGNED AS COLLECTOR ELEMENTS.

4. ALL LOADS SUPPORTED BY OPEN WEB STEEL JOISTS AND GIRDERS SHALL BE LOCATED WITHIN 6" OF JOIST OR GIRDER PANEL POINT OR THE JOIST OR GIRDER SHALL BE REINFORCED PER DETAIL D1/SF512.

5. HORIZONTAL CROSS BRIDGING SHALL BE SIZED AND SUPPLIED BY THE JOIST MANUFACTURER. CONNECT TO WALLS AS INDICATED ON DETAILS

6. OPEN WEB STEEL JOISTS BEARING AT CONCRETE OR MASONRY WALLS SHALL BE DESIGNED FOR A SEISMIC AXIAL LOAD TO BE TRANSFERRED THROUGH THE JOIST BEARING SEAT. MINIMUM UNFACTORED LOAD SHALL BE 4.0 K AT K JOISTS, 6.0 K AT LH & DLH JOISTS, UNO.

7. ALL OPEN WEB STEEL JOISTS WITH A SLOPE OF 3/8" PER FOOT OR LARGER SHALL HAVE SLOPED BEARING SEATS.

8. OPEN WEB STEEL JOISTS AT ROOF AREAS SHALL BE DESIGNED FOR THE FOLLOWING WIND ASD NET UPLIFT LOADS: 25PSF WITHIN 18FT OF ROOF EDGES, 18PSF AT ALL OTHER AREAS.

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BRICK LEDGER ANGLE SCHEDULE				
d	ANGLE SIZE	NOTES		
UP TO 5"	L4x4x3/8			
≤ 6" L5X5x3/8				
≤ 7" L6x6x3/8				
≤ 8" 7x7x3/8 BENT PLATE				
≤ 9" L8x8x1/2				
≤ 10" 9x9x1/2 BENT PLATE				

MHTN ARCHITECTS MHTN Architects, Inc. 280 South 400 West, Suite 250 Salt Lake City, Utah 84101 Telephone (801) 595-6700 Telefax (801) 595-6717 www.mhtn.com

515 East 100 South, Suite 1200 Salt Lake City, Utah 84102 801 486 3883 www.reaveley.com

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515 East 100 South, Suite 1200 Salt Lake City, Utah 84102 801 486 3883 www.reaveley.com

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515 East 100 South, Suite 1200 Salt Lake City, Utah 84102 801 486 3883 www.reaveley.com

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	STEEL DECK							
MARN	PROFILE	MIN I (in⁴/ft)	MIN S (in³/ft)					
SD-1	TYPE B 1.1/2" DEEP x 20 GA	0.219	0.230	GALVA				
SD-2	TYPE W2 3.1/2" DEEP x 20 GA	0.907	0.510	PHOS P				
 STEEL SUBM FIBER ALL DI MANUFAI DECK TO STEEL NOTED C DISTRIBL DECK DO NO SEE T PROVI 10.SEE P 	DECK SHALL COMPL IT CURRENT CODE EV REINFORCEMENT, W ECK SHALL BE 3-SPAN CTURER FOR THE SPA ALLOW FOR UN-SHO DECK WITHOUT CON THERWISE. LIGHTWE JTE THE LOAD OVER I SHALL HAVE 2" MINIM TEMBED CONDUITS YPICAL DETAILS FOR IDE GALVANIZED STEL LANS AND DETAILS FO	Y WITH LATEST REQU ALUATION REPORT (HEN REQUIRED IN SC CONTINUOUS MININ AN CONDITION, SPAN RED DECK OR PROVI ICRETE FILL SHALL N EIGHT SUSPENDED AN MULTIPLE DECK FLUT IUM BEARING ON ALL OR PIPES IN CONCRE REINFORCEMENT RE EL DECK ABOVE & BE OR LOCATIONS WHEF	JIREMENTS OF THE S ICC OR IAPMO) WITH CHEDULE, SHALL BE M IUM WHERE POSSIBL LENGTH, AND DECK DE SHORING. OT BE USED TO SUP COUSTICAL CEILINGS ES. SUPPORTING MEMB ETE FILL OVER STEEL EQUIRED AT OPENING LOW MECHANICAL R RE ADDITIONAL SLAB	STEEL DI LOAD A MACROS .E. IN AF GAUGE. PORT LC S WITH A ERS (ME DECKS SS THRO COMS. REINFO				
	SLAB REINFORCEME WHERE REQUIRED	NT	1" COVER U.N.O.	COI THI				
				STE				
	CONCRETE SLAB OVER STEEL DECK PROFILE							

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	STEEL DECK SCHEDULE						
			CONCRETE FILL		STEEL DECK	MIN. ALLOWABLE	NOTES
ft)	FINISH	THICKNESS (t)	TYPE	REINFORCEMENT	ATTACHMENT	SHEAR CAPACITY	NOTES
	GALVANIZED (G60)	-	-	-	SDA-1	1304 PLF @ 6'-0"	-
	PHOSPHATIZED/ PAINTED	5.1/2"	NORMAL WEIGHT	6x6-W2.0xW2.0 WWF OR FIBER	SDA-2	1800 PLF @ 10'-0"	

F THE STEEL DECK INSTITUTE (SDI).

) WITH LOAD AND LATERAL SHEAR CAPACITIES WITH SHOP DRAWINGS.

L BE MACROSYNTHETIC FIBER REINFORCEMENT PER THE CONCRETE MATERIALS SECTION OF THE GENERAL STRUCTURAL NOTES. OSSIBLE. IN AREAS WHERE 3-SPAN CONDITIONS ARE NOT POSSIBLE THE CONTRACTOR SHALL VERIFY UN-SHORED DECK IS PERMITTED BY THE DECK

DECK GAUGE. WHERE DECK DOES NOT MEET THE REQUIREMENTS FOR UN-SHORED DECK, THE CONTRACTOR SHALL EITHER PROVIDE HEAVIER GAUGE

TO SUPPORT LOADS FROM PLUMBING, HVAC DUCTS, LIGHT FIXTURES, ARCHITECTURAL ELEMENTS OR EQUIPMENT OF ANY KIND, UNLESS SPECIFICALLY EILINGS WITH A TOTAL WEIGHT PER WIRE NOT EXCEEDING 50# MAY BE HUNG FROM THE STEEL ROOF DECK. THE HANGERS SHOULD BE STAGGERED TO

MEMBERS (MEMBERS PERPENDICULAR TO DECK) UNO. DECKS SHALL HAVE 1.1/2" MINIMUM BEARING AT PARALLEL MEMBERS. R STEEL DECKS WITHOUT APPROVAL OF STRUCTURAL ENGINEER.

PENINGS THROUGH STEEL DECK. OPENING REINFORCING SHALL BE INSTALLED PRIOR TO SAW CUTTING OPENINGS. NICAL ROOMS.

L SLAB REINFORCEMENT IS REQUIRED.

STEEL DECK ATTACHMENT SCHEDULE						
WELDED		MECHANICAL				
	SUPPORTS	PARALLEL	SIDE LAP	SUPPORTS	PARALLEL	SIDE LAP
SDA-1	PW @ 36/7	PW @ 12" O.C.	1.1/2" TSW @ 18" O.C.	PAF @ 36/7	PAF @ 12" O.C.	PSC @ 12" O.C.
SDA-2	PW @ 36/4	PW @ 12" O.C.	1.1/2" TSW @ 18" O.C.	-	-	-
NOTES: 1. PW = PUDDLE WELD - 1/2" EFFECTIVE DIAMETER ARC SPOT WELD AT INTERIOR FLUTES, 1" X 3/8" EFFECTIVE ARC SEAM WELD AT SUPPORTS ADJACENT TO SIDELAP. 2. TSW = TOP SEAM WELD - 1.1/2" LONG TOP SEAM WELDS BETWEEN ADJACENT PIECES OF DECKING. CRIMP SIDE SEAMS BEFORE WELDING INTERLOCKING SEAMS. 3. BP = BUTTON PUNCH - 3/16" BUTTON PUNCH BETWEEN ADJACENT PIECES OF DECK. CRIMP SEAMS BEFORE BUTTON PUNCHING INTERLOCKING SEAMS. 4. PAF = POWDER ACTUATED FASTENER - HILTI X-ENP-19 L15 AT SUPPORTS 3/16" THROUGH 3/8" THICK HILTI X-ENP-19 L15 AT SUPPORTS 1/4" THICK AND GREATER PNEUTEK SDK61075 AT SUPPORTS 0.113" THROUGH 0.155" THICK HILTI X-ENP.19 L15 AT SUPPORTS 1/4" THICK AND GREATER 5. SDS = SELF DRILLING SCREW. WHERE SIDELAPS HAVE SCREWED CONNECTION, THE DECK PROVIDED SHALL HAVE A SCREWABLE SIDE SEAM, UNO. 6. PSC = PROPRIETARY SIDELAP CONNECTION - VERCO SIDELAP CONNECTION 2 FOR VERCO PUNCHLOK II SYSTEM, ASC DELTA GRIP FOR ASC DECK SHEET WITH 4 PUDDLE WELDS AT EACH SUPPORT. 8. HEADED STUD ANCHORS WELDED THROUGH DECK WITH 1" MINIMUM COVER FROM EDGE OF DECK TO STUD CENTERLINE MAY BE SUBSTITUTED ONE FOR ONE FOR PW. ALIGN AND SECURE DECK IN POSITION BEFORE INSTALLING STUDS. 9. SEE PLANS AND SFRS. OLITA AND SECURE DECK IN POSITION BEFORE INSTALLING STUDS. 9. SEE PLANS AND SFRS SHEETS FOR ADDITIONAL FASTENERS REQUIRED AT MEMBERS DENOTED AS SFRS. OMIT ATTACHMENTS WHERE DENOTED AS PROTECTED ZONES IN SFRS. 10. ALL WELDED SURFACES SHALL BE DRY BEFORE WELDING OR INSTALLING FASTENERS OR STUDS. 11. ALIGN AND SECURE DECK IN POSITION BEFORE WELDING DECK OR STUDS TO SUPPORTS. 11. ALIGN AND SECURE DECK IN POSITION BEFORE WELDING DECK OR STUDS TO SUPPORTS. 11. ALIGN AND SECURE DECK IN POSITION BEFORE WELDING OR INSTALLING FASTENERS OR STUDS. 12. ALTERNATE MEANS OF DECK ATTACHMENT ARE PERMITTED WITH APPROVAL OF THE ENGINEER. THE CONTRACTOR SHALL SUBMIT THE PROPOSED TATCHMENT SYSTEM AND THE CODE EVALUATION REPORT DEMORDINGTRATING THE SYSTEM HAS THE STRENGTH TO MEET THE SPECIFIED DECK SHEAR. IF THE ALTERNATE METHOD						
E	3 DECK 36/4 🧷		∖●/──_/── ∕● W DE	СК 36/4 🔎	•	
E	3 DECK 36/7 🥒					

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STEEL ANGLE LINTEL SCHEDULE							
BEARING EQUALS 1" PER FOOT OF OPENING OR 6" MINIMUM TYPICAL							
	OPENING SIZE	ANGLE SIZE	NOTES: LINTELS ARE DESIGNED TO SUPPORT				
	0'-0" - 7'-0"	L3.1/2x3.1/2x1/4	UNIFORM LOADS CONSISTING ONLY OF				
	7'-0" - 9'-0"	L4x3.1/2x1/4	ISOSCELES TRIANGLE AREA ABOVE				
	9'-0" - 10'-0"	L5x3.1/2x1/4	OPENING.				
			ALL STEEL LINTELS ARE TO HAVE LONG				
			LINTEL SCHEDULE FOR 4" VENEER				
			ALL ANGLE LINTELS SHALL BE GALVANIZED.				

МНТ ARCHITECTS MHTN Architects, Inc. 280 South 400 West, Suite 250 Salt Lake City, Utah 84101 Telephone (801) 595-6700 Telefax (801) 595-6717 www.mhtn.com

515 East 100 South, Suite 1200 Salt Lake City, Utah 84102 801 486 3883 www.reaveley.com

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MHTN PROJECT NO. 2017559

Original drawing is 30 x 42. Do not scale contents of this drawing. REVISIONS CONTRACTOR TO VERIFY DRAWINGS IN FIELD USE REFLECT LAST REVISION DATE. NO.

BID PACKAGE #1 MAY 12, 2025

